Mechanics of Composite Materials

CHAPTER 4. Laminate Theory

SangJoon Shin
School of Mechanical and Aerospace Engineering
Seoul National University





Can now manipulate orthotropic plies in plane stress

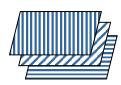
$$\underline{\sigma} = Q \underline{\varepsilon}, \quad \overline{\underline{\sigma}} = \overline{Q} \overline{\varepsilon}$$

where

$$\overline{Q} = f(Q, \theta)$$

Similarly, have $\underline{\varepsilon} = \underline{S} \underline{\sigma}$, etc.

But, composites are actually used as laminates



- Many plies (lamina) are arranged at many θ
- Carry load, provide stiffness, strength, etc.

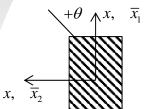
Note, other laminates → electronics - circuit boards

Capacitors, active materials (piezoelectrics), thermal barrier

- coats for engine combustors, etc.

Laminate Notation

Need keep track of ply orientation Use a compact notation



 θ : ply angle

Note: Usually 0° direction corresponds to principal loading direction

Laminates specified as
$$\begin{bmatrix} \pm 30 / 0_2 \end{bmatrix}_s$$
 \rightarrow $\begin{bmatrix} -30 \\ 0 \end{bmatrix}$ repeat symm. $\begin{bmatrix} \pm 30 / 0_3 \end{bmatrix}_{T}$ Bottom $\begin{bmatrix} 0 \\ -30 \\ 0 \end{bmatrix}$

$$\begin{bmatrix} 0 / 30 \end{bmatrix}_{2S} \rightarrow \begin{bmatrix} 0 \\ 30 \\ 0 \\ repeat group \end{bmatrix}$$

Typical Laminates may bear

Cross Ply -
$$[0_2/90_2]_T$$
 \rightarrow

Angle ply -
$$[\pm \theta]_s$$

Quasi – Isotropic – [0 /
$$\pm$$
45 / 90]_s , [0 / \pm 60]_s

❖ In-plane stress strain & stiffness

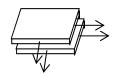
(Symmetric laminates - no bending)

Basic Assumptions:

- 1. Plies are all glued together
- 2. Plies are in plane-stress

$$\sigma_{33} = \sigma_{23} = \sigma_{13} = 0$$

Strain



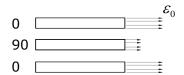
Because of gluing, ε_{ij} 's all same

$$\mathcal{E}_{\mathbf{z}}^{0} = \mathcal{E}_{\mathbf{z}} = \begin{cases} \mathcal{E}_{x}^{0} \\ \mathcal{E}_{y}^{0} \\ \gamma_{xy}^{0} \end{cases}$$

Stress



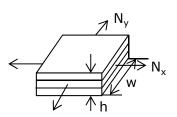
Is stress in 0° and 90° same? Not same.



Q_{ij} different

To find stresses, look at average force in plies.

Define N ← force/unit width of laminate



$$N_x = \frac{load \quad in \quad x}{dx}$$

Total load - P (lb)

$$N = \frac{P}{w} \quad (lb/in)$$

Average stress $(\sigma_x)_A = N_x/h$, h = laminate thickness

$$N_x = \int_{-\frac{h}{2}}^{\frac{h}{2}} \sigma_x dz$$

In discrete plies, $N_x = \sum_{k=1}^n \sigma_x^{(k)} t_k$ n = number of plies, k = 1, 2, 3, --- top to down

Similarly
$$N_y = \sum_{k=1}^n \sigma_y^{(k)} t_k$$
, $N_{xy} = \sum_{k=1}^n \sigma_{xy}^{(k)} t_k$

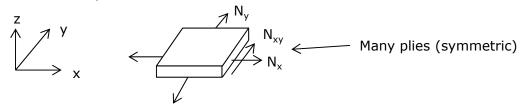
will then have

Similarly for N_y and N_{xy} So finally

$$\begin{cases}
N_{x} \\
N_{y} \\
N_{xy}
\end{cases} =
\begin{bmatrix}
A_{11} & A_{12} & A_{16} \\
A_{12} & A_{22} & A_{26} \\
A_{16} & A_{26} & A_{66}
\end{bmatrix}
\begin{cases}
\mathcal{E}_{x}^{0} \\
\mathcal{E}_{y}^{0} \\
\gamma_{xy}^{0}
\end{cases}$$

$$= \mathbf{N} \qquad = \mathbf{A} \qquad = \mathcal{E}^{0}$$

Given a symmetric laminate



Have formed a relation $N = A \varepsilon^0$

Where,
$$N = \begin{cases} N_x \\ N_x \\ N_{xy} \end{cases}$$
 = Force (lbs/in)

$$\tilde{\varepsilon}^{0} = \begin{cases} \varepsilon_{x}^{0} \\ \varepsilon_{x}^{0} \\ \gamma_{xy} \end{cases} = \text{Midplane strains (in/in)}$$
(laminate axes)

$$\vec{A} = \begin{cases}
A_{11} & A_{12} & A_{16} \\
A_{21} & A_{22} & A_{26} \\
A_{16} & A_{26} & A_{66}
\end{cases}$$
= Extensional stiffness (lb/in)

and
$$A_{ij} = \sum_{k=1}^{N} \overline{Q}_{ij}^{(k)} t_k$$

Could also write as equivalent moduli

$$\frac{\overline{\sigma}}{\sigma} = \underline{E}^{eq} \quad \underline{\mathcal{E}}^{0} \qquad \xrightarrow{h} \qquad \longrightarrow N_{x}$$
Averaged stress = N/h
$$So \quad \frac{1}{h} \underbrace{N} = \underbrace{\frac{1}{h} \underbrace{A}_{x} \underbrace{\mathcal{E}}^{0} \qquad \qquad }_{\underline{E}^{eq}} \longleftarrow \text{Lbs/in}^{2} : \text{modulus}$$

$$\underline{E}^{eq} = \begin{bmatrix} E_{11}^{\ \ eq} & E_{12}^{\ \ eq} & E_{16}^{\ \ eq} \\ E_{12}^{\ \ eq} & E_{22}^{\ \ eq} & E_{26}^{\ \ eq} \\ E_{16}^{\ \ eq} & E_{26}^{\ \ eq} & E_{66}^{\ \ eq} \end{bmatrix} \longleftarrow \text{ Like } \underline{Q} \text{ matrix for the laminate }$$

These are not the Engineering constants for the laminate.

Also, have Inverse Relations

$$\underline{\varepsilon}^0 = \underline{a} \quad \underline{N} \quad \text{where} \quad \underline{a} = A^{-1}$$

This only applies for symmetric laminates (no bending)

Deal later with unsymm. laminates

❖ Properties of A matrix

$$\underline{A} = \sum_{k=1}^{N} \overline{Q}^{(k)} t_{k}$$

$$\overline{Q}_{11} = c^{4}Q_{11} + s^{4}Q_{22} + \cdots
\overline{Q}_{11} = c^{2}s^{2}(Q_{11} + \cdots
\overline{Q}_{11} = \vdots$$

See Handout, also Jones, p.51

- \cdot Remark on A
 - 1. Thickness (area) weight stiffness $ar{Q}_{ij}$
 - 2. Independent of stacking order
 - 3. Balanced laminates

"a -
$$\theta$$
 for every $+\theta$ "
$$\overline{Q}_{11}, \quad \overline{Q}_{12}, \quad \overline{Q}_{22}, \quad \overline{Q}_{66} \quad \text{not sensitive to sign}$$

$$\underbrace{\left(\begin{array}{cccc} \mathbf{c}^4, \, \mathbf{s}^4, \, \mathbf{c}^2, \, \mathbf{s}^2, \, \dots \end{array}\right)}_{Q_{16}, \quad \overline{Q}_{26}} \quad \text{are affected } \left(\mathbf{c}^3\mathbf{s}, \, \mathbf{cs}^3, \, \dots \right) \quad A_{16} = A_{21} = 0$$

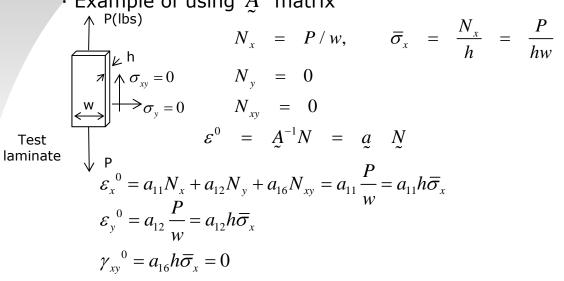
: Balanced laminates are orthotropic.

4. Quasi - isotropic laminate

$$[0 / \pm 60]_s$$
, $[0 / \pm 45 / 90]_s$ \leftarrow primary
 $[0 / \pm 30 / \pm 60 / 90]_s$ \leftarrow Built up from $[0 / \pm 60]_s$
 $A_{22} = A_{11}$, $A_{66} = f(A_{11}, A_{22})$

· · · Quasi – isotropic have "isotropic" stiffness

 \cdot Example of using A matrix



Laminate Engineering Constants

Constants we get from mechanical tests on laminates (as for plies)

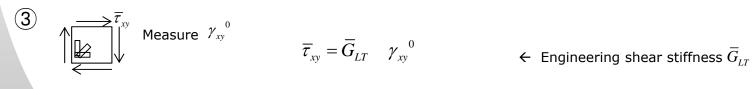
$$\bar{\sigma}_x = \frac{P}{wh},$$

$$\overline{\sigma}_{x} = \overline{E}_{L} \varepsilon_{x}^{0}$$

$$\overline{V}_{LT} = -\varepsilon_{y}^{0} / \varepsilon_{x}^{0}$$

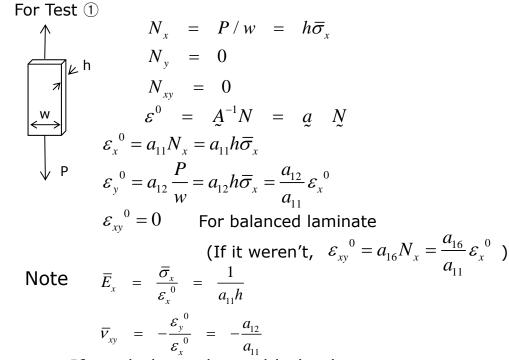
$$\bar{\sigma}_{x} = \frac{P}{wh},$$

$$\overline{\tau}_{y} = \overline{E}_{T} \quad \varepsilon_{y}^{0} \leftarrow \text{Engineering Transverse stiffness}$$

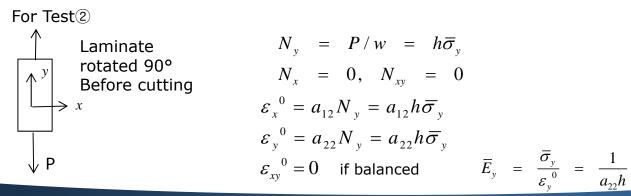


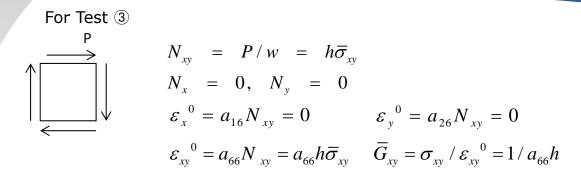
$$\overline{ au}_{xy} = \overline{G}_{LT} \quad \gamma_{xy}^{0}$$

Obtaining Laminate Engineering Constants



If not balanced, would also have $\eta_{xy,x}$ (Lekhnitski coefficient)





For balanced laminate,

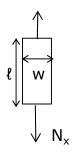
$$\underline{A} = \begin{bmatrix} A_{11} & A_{12} & 0 \\ A_{12} & A_{22} & 0 \\ 0 & 0 & A_{66} \end{bmatrix}, \quad \underline{a} = \begin{bmatrix} a_{11} & a_{12} & 0 \\ a_{12} & a_{22} & 0 \\ 0 & 0 & a_{66} \end{bmatrix}$$

$$a_{66} = \frac{1}{A_{66}}$$

$$\overline{G}_{xy} = \frac{1}{a_{66}h} = \frac{A_{66}}{h} = E_{66}^{eq}$$
But $a_{11} \neq \frac{1}{A_{11}}$, So $\overline{E}_{x} \neq E_{11}^{eq} \Rightarrow E_{quivalent}$

When does difference come up?

For w << ℓ case

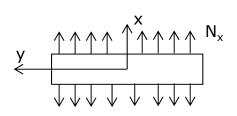


Unconstrained

$$N_{y} = N_{xy} = 0$$

$$\varepsilon_x^0 = \overline{\sigma}_x / \overline{E}_x \rightarrow E$$

For $w >> \ell$ case



Can't say $N_y = N_{xy} = 0$

$$\varepsilon_{y}^{0} = \varepsilon_{xy}^{0} = 0$$
 is a better approximation

Then would have

$$N = A \varepsilon^0$$

$$N_x = A_{11} \varepsilon^0 + 0 + 0$$

$$N_{y} = A_{12} \varepsilon^{0}$$

$$N_{xy} = A_{16} \varepsilon^0$$

$$N_x = \overline{\sigma}_x h = A_{11} \varepsilon_x^0$$

$$\frac{\overline{\sigma}_x}{\varepsilon_x^0} = \frac{A_{11}}{h} = E_{11}^{eq}$$

➤ Equivalent stiffness

Effect of Boundaries

Note also this effect in isotropic materials.

current stiffness there $\rightarrow E^{eq} = E/(1-v^2)$

Because plies are constrained by neighbors, usually more

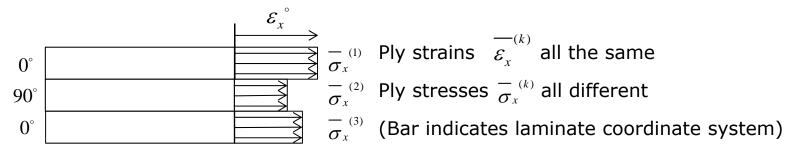
convenient to work with ${A\over z}$ (or E^{eq}) rather then ${\overline E}_{{\scriptscriptstyle X}}$, ${\overline E}_{{\scriptscriptstyle Y}}$, etc.

Ply Stresses

- Given a laminate description, can find laminate engineering constants for input to plate and shell problems.
- Given loads P_x , P_y , P_{xy} , can find N_x , N_y , N_{xy} and then one gets laminate strains from

$$\varepsilon^{\circ} = \underline{a} \cdot \underline{N}$$

- Average laminate stresses $\sigma = N / h$
- Now, want to look at individual stress in kth ply. (to predict failure)



- How do we calculate $\, \, {f \sigma}^{^{(k)}} \,$

- Note: no bar want stress in ply coordinate system
- Two paths for getting $\, \, {f \sigma}^{^{(k)}} \,$

Path #1

Know

$$\frac{\mathcal{E}}{\mathcal{E}} = \mathcal{E}^{\circ} \qquad \qquad \qquad \text{laminate strain}$$

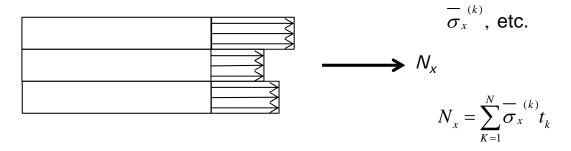
ply strain, ply k, laminate coord.

Also know

$$\overline{\underline{\sigma}}^{(k)} = \overline{Q}^{(k)} \overline{\varepsilon}^{(k)} \iff \text{ply strain, laminate coord.}$$

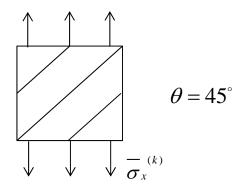
ply stress, ply k, laminate coord.

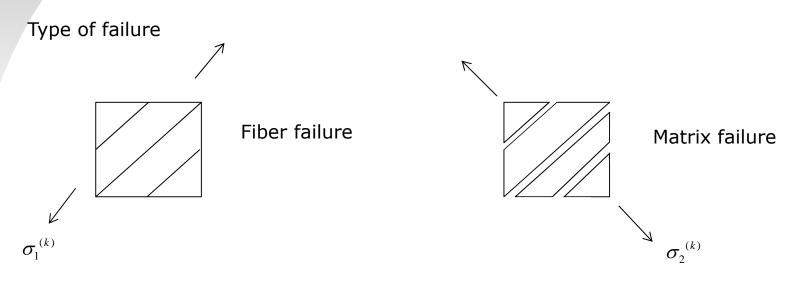
Therefore, can calculate stresses in ply



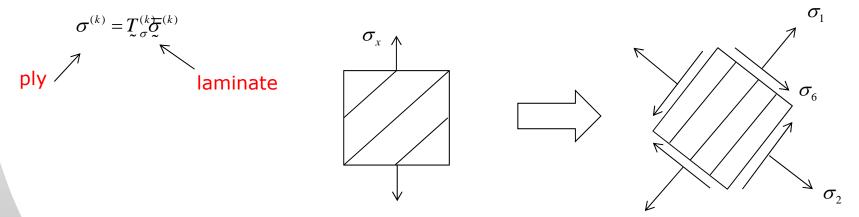
(can check out N_x)

Unfortunately, $\sigma_x^{(k)}$ isn't very useful.





Given $\overset{-}{\sigma}^{\scriptscriptstyle(k)}$, get $\overset{-}{\sigma}^{\scriptscriptstyle(k)}$ by transformation



Also, get ply strain in ply coordinates.

$$\varepsilon^{(k)} = \widetilde{S}^{(k)} \widetilde{\sigma}^{(k)}$$

So, system we have

$$P \longrightarrow N \xrightarrow{\tilde{Q}} \tilde{\mathcal{E}}^{\circ} \xrightarrow{\text{same}} \tilde{\mathcal{E}}^{(k)} \xrightarrow{\overline{Q}^{k}} \tilde{\mathcal{Q}}^{(k)} \xrightarrow{T_{\sigma}} \tilde{\mathcal{Q}}^{k} \xrightarrow{\tilde{S}} \tilde{\mathcal{E}}^{(k)}$$

Path #2

Given $\stackrel{-(k)}{\varepsilon}$, go directly to $\stackrel{-(k)}{\varepsilon}$ by transformation

$$\mathcal{E}^{(k)} = \mathcal{T}_{\varepsilon}^{(k)} \mathcal{E}^{(k)}$$
Ply coords. laminate coordinates

Then from $\varepsilon^{(k)}$, get $\sigma^{(k)}$ from ply stress – strain equations.

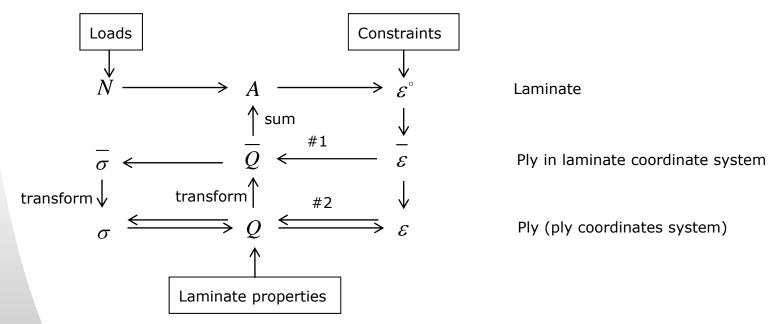
$$\boldsymbol{\sigma}^{(k)} = \boldsymbol{Q}^{(k)} \boldsymbol{\varepsilon}^{(k)}$$

So, have another system

$$P \longrightarrow \underset{\sim}{N} \xrightarrow{\overset{a}{\sim}} \underset{\sim}{\mathcal{E}} \xrightarrow{\text{same}} \underset{\varepsilon}{\overset{-(k)}{\varepsilon}} \xrightarrow{T_{\varepsilon}^{(k)}} \underset{\varepsilon}{\overset{Q^{(k)}}{\varepsilon}} \xrightarrow{Q^{(k)}}$$

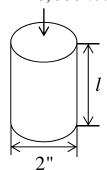
So, summarizing what we know so far, we have arrived at,

• In-Plane Classical Laminate Plate Theory (CLPT)



- Example of In-Plane CLPT

Use previous system to solve a practical problem. $20,000 \; lbs$



Tubular compression member

Assume *l* short (no buckling)

Material T300/934 Gr/Ep

Ply Engineering Properties

$$E_L = 20 \text{ Msi}, E_T = 1.4 \text{ Msi}, v_{LT} = 0.29, G_{LT} = 0.7 \text{ Msi}$$

Ply thicknesses: 0.005"(5 mils)

Lay up : $[0/\pm 45/90]_s$

Referring to ground scheme, already have Ply Eng'g Consts (obtained by micromechanics and test)

Step #1: Find
$$Q$$

$$v_{LT} = \frac{E_r}{E_L} v_{LT} = 0.020$$

$$Q_{11} = \frac{E_L}{1 - v_{LT}v_{TL}} = \frac{20}{1 - 0.29(0.02)} = 20.12 \ Msi$$
 contains much different from E_L

$$Q_{12} = \frac{v_{LT} E_T}{1 - v_{LT} v_{TL}} = 0.408 \ Msi$$

$$Q_{22} = \frac{E_T}{1 - v_{TT}v_{TT}} = 1.41 \, Msi \, \text{approximately } E_T$$

$$Q_{16} = 0, \ Q_{26} = 0,$$

$$Q_{66} = G_{LT} = 0.7 \ Msi$$

$$Q = \begin{bmatrix} 20.12 & 0.408 & 0 \\ 0.408 & 1.41 & 0 \\ 0 & 0 & 0.7 \end{bmatrix} (Msi)$$

Step #2: Compute \overline{Q} for each ply

$$0^{\circ}$$
 plies: Trivial $\longrightarrow \overline{Q}_{0} = Q$

 90° plies: Easy \longrightarrow Reverse 1, 2

$$\overline{Q}_{90} = \begin{bmatrix} 1.41 & 0.408 & 0 \\ 0.408 & 20.12 & 0 \\ 0 & 0 & 0.7 \end{bmatrix} (Msi)$$

 45° Plies: Harder \rightarrow Use transform formulas

$$\overline{Q}_{11} = c^4 Q_{11} + s^2 Q_{22} + 2c^2 s^2 (Q_{12} + 2Q_{66})$$

Note:
$$\begin{cases} \cos \theta = \sin \theta = 0.707 \\ \cos^2 \theta = \sin^2 \theta = 0.500 \end{cases}$$

$$\overline{Q}_{11} = 0.25(20.12) + 0.25(1.41) + 0.500(0.408 + 2[0.7])$$

= 6.29 Msi

Similarly

$$\overline{Q}_{12} = 4.89 \ Msi$$
 $\overline{Q}_{16} = , \overline{Q}_{26} = , \overline{Q}_{66} =$

$$\overline{Q}_{45} = \begin{bmatrix} 6.29 & 4.87 & 4.68 \\ 4.87 & 6.29 & 4.68 \\ 4.68 & 4.68 & 5.18 \end{bmatrix} (Msi)$$

 -45° Plies: Easy, same as +45 except $\,\overline{Q}_{\scriptscriptstyle 16}\,$ and $\,\overline{Q}_{\scriptscriptstyle 66}\,$ change signs

Note:
$$\sin(-\theta) = -\sin(\theta) \longrightarrow \text{Only } s, s^3$$

 $\cos(-\theta) = \cos(\theta)$

$$\overline{Q}_{-45} = \begin{bmatrix} 6.29 & 4.89 & 4.68 \\ 4.89 & 6.29 & -4.68 \\ 4.68 & -4.68 & 5.18 \end{bmatrix} (Msi)$$

Step #3: Assemble $\frac{A}{2}$ matrix

$$A = \sum_{k=1}^{N} Q^{(k)} t_k$$

Note: - Thickness all the same

- order doesn't matter here
- Symmetric $\overline{\underline{Q}}$ ightarrow symmetric $\underline{\underline{A}}$
- Also, $\overline{Q}_{11}^{-45^{\circ}} = \overline{Q}_{11}^{45^{\circ}}$ (etc. for 12, 22, 66) but, $\overline{Q}_{16}^{-45^{\circ}} = -\overline{Q}_{16}^{45^{\circ}}$ (and for 26),

so, 16 and 26 term cancel

So summing,

$$A_{11} = t(2Q_{11}^{\circ} + 2Q_{11}^{90} + 4Q_{11}^{45})$$

$$= 0.005(2(20.12 \times 10^{6}) + 2(1.41 \times 10^{6}) + 4(6.29 \times 10^{6}))$$

$$= 0.341 \times 10^{6} \ lb / in$$

$$A_{12} = t(2Q_{12}^{\circ} + 2Q_{12}^{90} + 4Q_{12}^{45})$$

$$= 0.005(2(0.408 \times 10^{6}) + 2(0.408 \times 10^{6}) + 4(4.89 \times 10^{6}))$$

$$= 0.106 \times 10^{6} \ lb / in$$

$$A_{22} = 0.341 \times 10^{6} \ lb / in$$

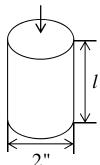
$$A_{66} = 0.118 \times 10^{6} \ lb / in$$

$$A_{16} = 0, A_{26} = 0$$

$$A = \begin{bmatrix} 0.341 & 0.106 & 0 \\ 0.106 & 0.341 & 0 \\ 0.106 & 0.341 & 0 \end{bmatrix} \times 10^{6} \ lb / in$$

Step #4: Establish Loading

20,000 *lbs*



Loading assume thin, load distributes evenly

$$N_x = \frac{P}{circumference} = \frac{P}{2\pi r} = \frac{20,000}{2\pi (1)} = -3.183 \ lb / in$$

Assume unrestrained, $N_x = 0$, $N_{xy} = 0$

Step #5: Calculate Laminate Strain

$$\varepsilon^{\circ} = aN$$
, $a = A^{-1}$

Can invert 3x3 matrix, on else,

$$\underline{a} = \begin{bmatrix} A_{11} & A_{12} \\ A_{12} & A_{22} \end{bmatrix}^{-1} & 0 \\ 0 & 0 & \frac{1}{A_{66}} \end{bmatrix} = \begin{bmatrix} \frac{A_{22}}{A_{11}A_{22} - A_{12}^{2}} & \frac{-A_{22}}{A_{11}A_{22} - A_{12}^{2}} & 0 \\ \frac{-A_{22}}{A_{11}A_{22} - A_{12}^{2}} & \frac{A_{22}}{A_{11}A_{22} - A_{12}^{2}} & 0 \\ 0 & 0 & \frac{1}{A_{66}} \end{bmatrix} = \begin{bmatrix} 3.25 & -1.01 & 0 \\ -1.01 & 3.25 & 0 \\ 0 & 0 & 8.47 \end{bmatrix} \times 10^{6} \text{ in / lb}$$

$$\varepsilon_x^{\circ} = a_{11}N_x = 3.25 \times 10^{-6} (-3183) = -0.0103$$

$$\varepsilon_y^{\circ} = a_{12}N_x = -1.01 \times 10^{-6} (-3183) = +0.0032$$

$$\varepsilon_{xy}^{\circ} = a_{16}N_x = 0$$

Step #6: Calculate Ply Strains in Laminate coordinates

All $\overset{-}{\varepsilon}^{\scriptscriptstyle(k)}$ equal to $\overset{-}{\varepsilon}^{\circ}$

Step #7: Calculate Ply Stresses in Laminate Coordinates System

$$\left\{ \begin{array}{l} \overline{\sigma}_{x} \\ \overline{\sigma}_{y} \\ \overline{\sigma}_{xy} \end{array} \right\} = \begin{bmatrix} \overline{Q}_{11} & \overline{Q}_{12} & \overline{Q}_{16} \\ \overline{Q}_{12} & \overline{Q}_{22} & \overline{Q}_{26} \\ \overline{Q}_{16} & \overline{Q}_{26} & \overline{Q}_{66} \end{bmatrix} \begin{bmatrix} \varepsilon_{x}^{\circ} \\ \varepsilon_{y}^{\circ} \\ \gamma_{xy}^{\circ} \end{bmatrix}$$

0° ply:
$$\overline{\sigma}_{x} = \overline{Q_{11}} \varepsilon_{x}^{\circ} + \overline{Q_{12}} \varepsilon_{y}^{\circ}$$

$$= 20.12 \times 10^{6} (-0.0103) + 0.408 \times 10^{6} (+0.0032)$$

$$= -206 (Ksi) \text{ (high)}$$

$$\overline{\sigma}_{y} = \overline{Q_{12}} \varepsilon_{x}^{\circ} + \overline{Q_{12}} \varepsilon_{y}^{\circ}, \qquad \overline{\sigma}_{xy} = 0$$

$$= 0.300 (Ksi) \text{ (low)}$$

+90° ply:
$$\overline{\sigma}_{x} = \overline{Q_{11}}^{90} \varepsilon_{x}^{\circ} + \overline{Q_{12}}^{90} \varepsilon_{y}^{\circ}$$

$$= -13 (Ksi)$$

$$\overline{\sigma}_{y} = 60 Ksi$$

$$\overline{\sigma}_{xy} = 0$$

 $+45^{\circ}$ ply:

$$\overline{\sigma}_{x} = \overline{Q_{11}} \varepsilon_{x}^{\circ} + \overline{Q_{12}} \varepsilon_{y}^{\circ} + \overline{Q_{16}} \gamma_{xy}^{\circ} = -49 (Ksi)$$

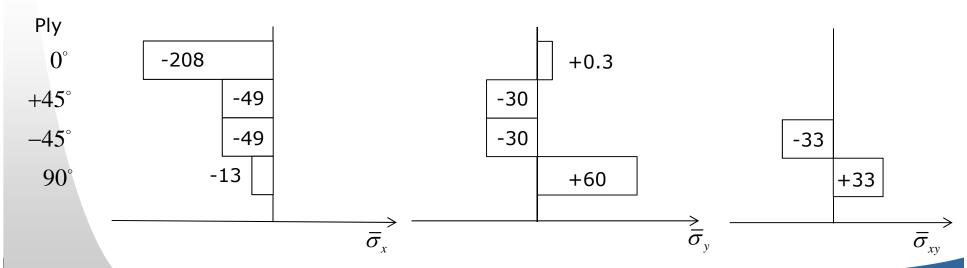
$$\overline{\sigma}_{y} = \overline{Q_{12}} \varepsilon_{x}^{\circ} + \overline{Q_{22}} \varepsilon_{y}^{\circ} + \overline{Q_{26}} \gamma_{xy}^{\circ} = -30 \ (Ksi)$$

$$\overline{\sigma}_{x} = \overline{Q_{16}} \varepsilon_{x}^{\circ} + \overline{Q_{26}} \varepsilon_{y}^{\circ} + \overline{Q_{66}} \gamma_{xy}^{\circ} = -33 \ (Ksi)$$

 -45° ply: same as $+45^{\circ}$, but

$$\overline{\sigma}_{xy}(-45^{\circ}) = -\overline{\sigma}_{xy}(+45^{\circ})$$

Plotting stresses



Step #8: Calculate Ply Stress \mathcal{I} in ply coordinates

$$\sigma = \begin{cases} \sigma_1 \\ \sigma_2 \\ \sigma_6 \end{cases} \quad \leftarrow \text{Jones Notation}$$

$$\sigma = T_{\sigma} \overline{\sigma} \qquad \Rightarrow \quad \sigma = \begin{cases} \sigma_{1} \\ \sigma_{2} \\ \sigma_{6} \end{cases} = \begin{bmatrix} c^{2} & s^{2} & 2sc \\ s^{2} & c^{2} & -2sc \\ -cs & cs & (c^{2} - s^{2}) \end{bmatrix} \begin{bmatrix} \overline{\sigma}_{x} \\ \overline{\sigma}_{y} \\ \overline{\sigma}_{xy} \end{bmatrix}$$

$$\begin{cases}
\sigma_{1} \\
\sigma_{2} \\
\sigma_{6}
\end{cases} = \begin{bmatrix}
1 & 0 & 0 \\
0 & 1 & 0 \\
0 & 0 & 1
\end{bmatrix} \begin{bmatrix}
-206 \\
30 \\
0
\end{bmatrix} = \begin{bmatrix}
-206 \\
30 \\
0
\end{bmatrix} Ksi$$

90° ply:
$$\begin{cases} \sigma_1 \\ \sigma_2 \\ \sigma_6 \end{cases} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{cases} -13 \\ 60 \\ 0 \end{cases} = \begin{cases} 60 \\ -13 \\ 0 \end{cases} Ksi$$

$$\begin{cases}
\sigma_1 \\
\sigma_2 \\
\sigma_6
\end{cases} = \begin{bmatrix}
0.5 & 0.5 & 0 \\
-0.5 & -0.5 & -1 \\
-0.5 & 0.5 & 0
\end{bmatrix} \begin{cases}
-49 \\
30 \\
33
\end{cases} = \begin{cases}
-73 \\
-6.5 \\
-9.5
\end{cases} Ksi$$

 -45° ply: Same signs as $+45^{\circ}$ by σ_{6} change sign

Summary of stress (Ksi)

ply, θ	$\sigma_{_{1}}$	$\sigma_{\scriptscriptstyle 2}$	$\sigma_{\scriptscriptstyle 6}$
0°	-206	0.3	0
+45°	-73	-6.5	9.5
-45°	-73	-6.5	-9.5
+90°	60	-13	0

Note

Compare to strength of unidirectional material

Compress ultimate (1-dir) = 160 Ksi

Compress ultimate (2-dir) = 25 Ksi

Shear ultimate = 10 Ksi

Fiber failure in 0° ply, reinforce strut

Look also at ply-axis strains

Step #9: Calculate Ply Strains \mathcal{E} in ply coordinates.

$$\mathcal{E} = \begin{cases} \mathcal{E}_{1} \\ \mathcal{E}_{2} \\ \mathcal{E}_{6} \end{cases} \quad \text{← Jones notation}$$

$$\mathcal{E} = \mathcal{S}\mathcal{T} \quad \Rightarrow \quad \begin{cases} \mathcal{E}_{1} \\ \mathcal{E}_{2} \\ \mathcal{E}_{6} \end{cases} = \begin{bmatrix} S_{11} & S_{12} & S_{16} \\ S_{12} & S_{22} & S_{26} \\ S_{16} & S_{26} & S_{66} \end{bmatrix} \begin{cases} \sigma_{1} \\ \sigma_{2} \\ \sigma_{6} \end{cases}$$

$$S_{11} = \frac{1}{E_{L}} = \frac{1}{20 \times 10^{6}} = 0.050 \times 10^{-6}$$

$$S_{12} = -\frac{v_{LT}}{E_{L}} = -\frac{0.29}{20 \times 10^{6}} = -0.0145 \times 10^{-6}$$

$$S_{22} = \frac{1}{E_{T}} = \frac{1}{1.4 \times 10^{6}} = 0.7143 \times 10^{-6}$$

$$S_{66} = \frac{1}{G_{LT}} = \frac{1}{0.7 \times 10^{6}} = 1.429 \times 10^{-6}$$

$$S_{16} = S_{26} = 0$$

$$+90^{\circ} \text{ ply: } \begin{cases} \varepsilon_{1} \\ \varepsilon_{2} \\ \varepsilon_{6} \end{cases} = \begin{bmatrix} 0.0500 & -0.0145 & 0 \\ -0.0145 & 0.7143 & 0 \\ 0 & 0 & 1.429 \end{bmatrix} \begin{bmatrix} 60 \\ -13 \\ 0 \end{bmatrix} = \begin{bmatrix} -0.0032 \\ -0.0103 \\ 0 \end{bmatrix}$$

$$\begin{cases}
\varepsilon_1 \\
\varepsilon_2 \\
\varepsilon_6
\end{cases} = \begin{bmatrix}
0.0500 & -0.0145 & 0 \\
-0.0145 & 0.7143 & 0 \\
0 & 0 & 1.429
\end{bmatrix} \begin{bmatrix}
-73 \\
-6.5 \\
9.5
\end{bmatrix} = \begin{bmatrix}
-0.0036 \\
-0.0036 \\
0.0136
\end{bmatrix}$$

-45° ply - same as +45° by $\varepsilon_{\rm 6}$ change sign

Summary of strains

ply, θ	\mathcal{E}_1	\mathcal{E}_2	\mathcal{E}_6
0°	-0.0103	0.0032	0
+45°	-0.0036	-0.0036	0.0136
-45°	-0.0036	-0.0036	-0.0136
+90°	-0.0032	-0.0103	0

Sometimes use a max. strain criteria instead of max. stress $(\varepsilon_1 = 7000 \ \mu \varepsilon)$

Also can do ply stress analysis by Path #2

Steps $1 \sim 6$ same as before

Step #7A Calculate ply strain \mathcal{E} in ply coords.

$$\widetilde{\varepsilon} = T_{\varepsilon} \overline{\varepsilon} \quad \Rightarrow \qquad
\begin{cases}
\varepsilon_{1} \\ \varepsilon_{2} \\ \varepsilon_{6}
\end{cases} =
\begin{bmatrix}
c^{2} & s^{2} & sc \\ s^{2} & c^{2} & -sc \\ -2cs & 2cs & (c^{2} - s^{2})
\end{bmatrix}
\begin{bmatrix}
\overline{\varepsilon}_{x} \\ \overline{\varepsilon}_{y} \\ \overline{\varepsilon}_{xy}
\end{bmatrix}$$

$$0^{\circ} \text{ ply:} \qquad \begin{cases} \mathcal{E}_{1} \\ \mathcal{E}_{2} \\ \mathcal{E}_{6} \end{cases} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{cases} -0.0103 \\ 0.0032 \\ 0 \end{cases} = \begin{cases} -0.0103 \\ 0.0032 \\ 0 \end{cases}$$

$$\begin{cases}
\varepsilon_1 \\
\varepsilon_2 \\
\varepsilon_6
\end{cases} = \begin{bmatrix}
1 & 0 & 0 \\
0 & 1 & 0 \\
0 & 0 & 1
\end{bmatrix} \begin{bmatrix}
-0.0103 \\
0.0032 \\
0
\end{bmatrix} = \begin{bmatrix}
0.0032 \\
-0.0103 \\
0
\end{bmatrix}$$

etc. 45° and -45° plus

Step #8A: Calculate ply stress σ in ply coords.

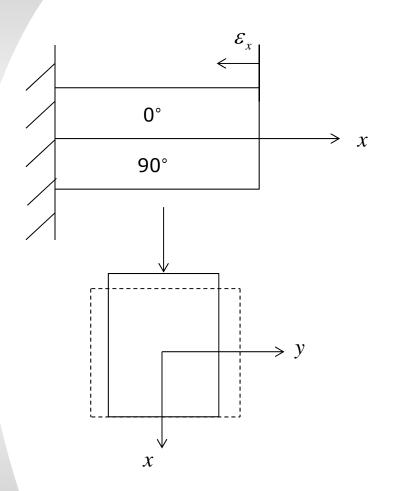
$$\begin{cases}
\sigma_{1} \\
\sigma_{2} \\
\sigma_{6}
\end{cases} =
\begin{bmatrix}
20.12 & 0.408 & 0 \\
0.408 & 1.41 & 0 \\
0 & 0 & 0.7
\end{bmatrix}
\begin{cases}
-0.0103 \\
-0.0032 \\
0
\end{cases} =
\begin{cases}
-206 \\
-3 \\
0
\end{cases} Ksi$$

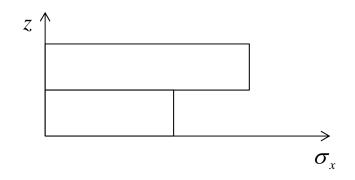
$$\times 10^{-6}$$

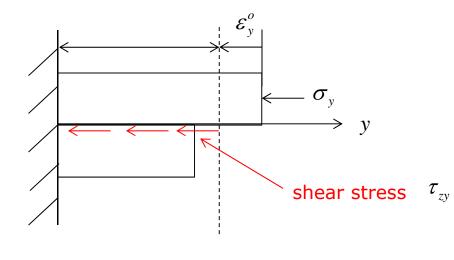
$$\begin{cases} \sigma_1 \\ \sigma_2 \\ \sigma_6 \end{cases} = \begin{bmatrix} same \\ -0.0103 \\ 0 \end{bmatrix} \begin{cases} -0.0032 \\ -0.0103 \\ 0 \end{cases} \quad Ksi$$

same +45° and -45°

Same results from Path #1 and Path #2 easier (but no σ)







$$\tau_{zy} dy dx = \sigma_{y} h(\varepsilon_{y}^{F} - \varepsilon_{y}^{\circ}) dy dx$$
$$\tau_{zy} = \sigma_{y} h(\varepsilon_{y}^{F} - \varepsilon_{y}^{\circ})$$