

Water quality II

Today's lecture

- Biochemical oxygen demand (BOD)
 - Concept
 - Measurement
 - Modeling
- DO dynamics in river
 - DO sag curve
 - Modeling DO in the river
 - Solution: Streeter-Phelps equation

Oxygen demand

- Biochemical oxygen demand (BOD)
 - The oxygen demand is measured by a bioassay
 - The water sample is inoculated with bacteria that degrade organic matter in water
 - The difference in DO in the water sample at the beginning and end of the test is used to calculate BOD



Oxygen demand

ThOD \geq COD $>$ BOD

- Some organic compounds may not be oxidized even with a strong oxidizing agent (ThOD \geq COD)
- Some carbon is used for bacterial growth; some organic compounds are not biodegradable; some organic matter is converted to non-biodegradable materials (COD $>$ BOD)

BOD Measurement

Step 1. Take the wastewater sample and dilute if needed. Fill the test bottle (usually 300 mL) with the (diluted) sample and a suspension of microorganisms (seed) if needed. Seal the bottle to prevent air intrusion/water evaporation.

$$\text{Dilution factor} = P = \frac{\text{volume of wastewater sample}}{\text{volume of wastewater} + \text{dilution water}}$$

The expected BOD of the diluted sample should be 2-6 mg/L.

* saturation DO concentration at 20°C: 9.17 mg/L

BOD Measurement

Step 2. Prepare blank samples (control) containing only the dilution water and the seed.

Step 3. Incubate the samples and blanks at 20°C in the dark. Usually the incubation time is 5 days.

Step 4. Measure the DO after incubation.

BOD Measurement

The BOD of the wastewater sample can be calculated as:

$$BOD_t = \frac{DO_{b,t} - DO_{s,t}}{P}$$

$DO_{b,t}$ = DO concentration in blank after t days of incubation

$DO_{s,t}$ = DO concentration in sample after t days of incubation

If the BOD of the seed is significant, the following equation should be used instead:

$$BOD_t = \frac{(DO_{s,i} - DO_{s,t}) - (DO_{b,i} - DO_{b,t})f}{P}$$

$DO_{s,i}$ = the initial DO of the sample

$DO_{b,i}$ = the initial DO of the blank

f = (volume of seed in sample) /

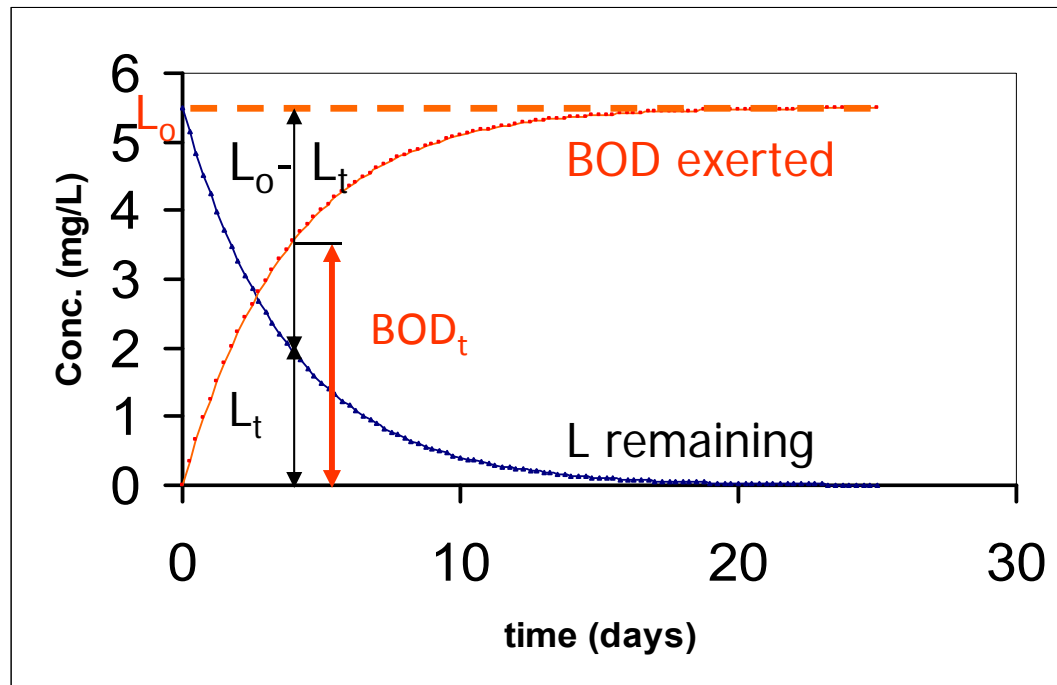
(volume of seed in blank)

BOD Measurement

Q: The BOD of a wastewater sample was initially estimated to be 180 mg/L. What volume of the sample should be added to a 300-mL bottle?

Applying the calculated dilution factor, the DO values for the blank and diluted sample after 5 days of incubation were 8.7 and 4.2 mg/L, respectively. What is the BOD_5 of the sample? Assume that the sample and the blank were not seeded.

Modeling BOD



L = oxygen demand of remaining biodegradable organic chemicals (mg/L)

- L_t decreases with time and BOD_t increases with time
- $L_0 = L_t + BOD_t$
- $L_0 (= BOD_\infty)$: ultimate BOD

Modeling BOD

The degradation of organic compounds by microorganisms is modeled as a first-order reaction:

$$\frac{dL}{dt} = -kL \quad k = \text{first-order reaction constant (day}^{-1}\text{)}$$

Integration of the equation gives:

$$L_t = L_0 e^{-kt}$$

As $BOD_t = L_0 - L_t$,

$$BOD_t = L_0(1 - e^{-kt})$$

Modeling BOD

The magnitude of the BOD rate constant, k depends on:

1. Nature of waste: whether the waste is easily biodegradable or not
2. Ability of organisms to use waste: the microorganisms in the test bottle may not be ready to degrade the waste! (recall the “lag phase”)
3. Temperature

$$k_T = k_{20} \theta^{T-20}$$

k_T = BOD rate constant at temperature T (day⁻¹)

k_{20} = BOD rate constant at 20°C (day⁻¹)

θ = temperature coefficient

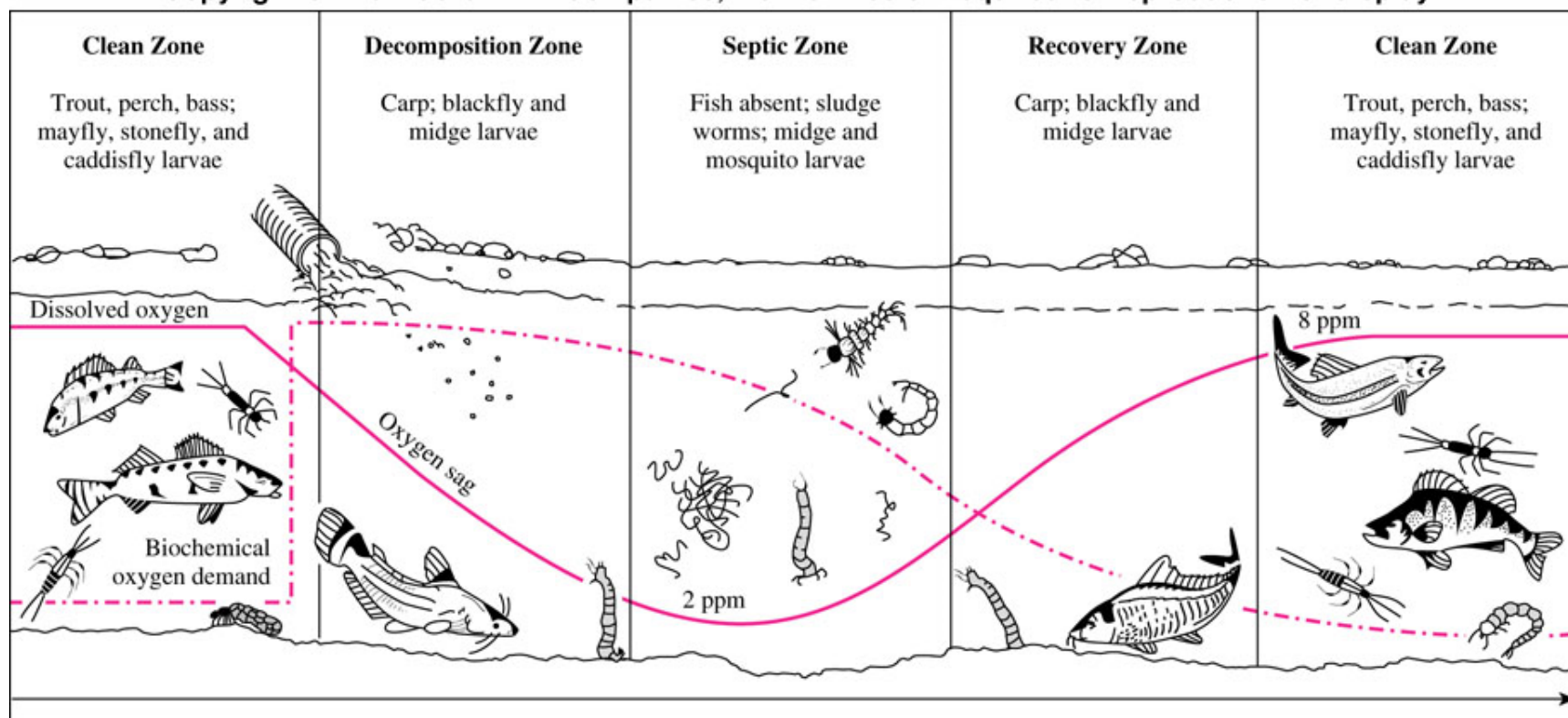
(use 1.135 for 4-20°C and 1.056 for 20-30°C)

Modeling BOD

Q: The BOD_5 of a wastewater is 120 mg/L and the BOD rate constant is 0.115 day^{-1} at 20°C . What is the ultimate BOD? If the wastewater is incubated at 15°C with a supply of oxygen, how much oxygen will be used by microorganisms in three days?

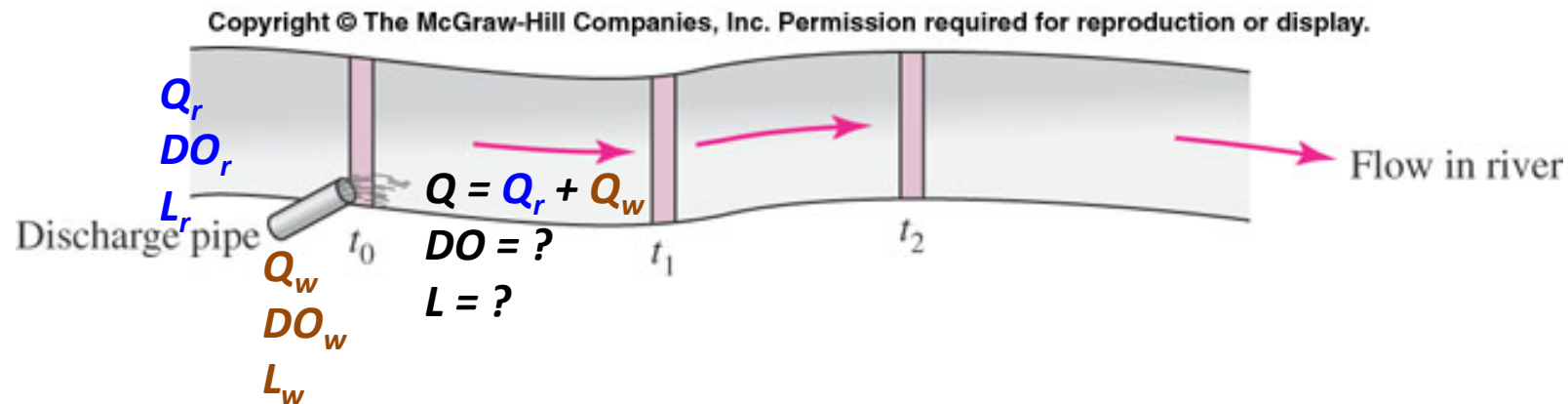
DO sag curve

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- Factors causing DO depletion: BOD in water (upstream + waste)
- Factors causing DO increase: reaeration from the atmosphere (+ photosynthesis – neglected)

Modeling the DO along a river

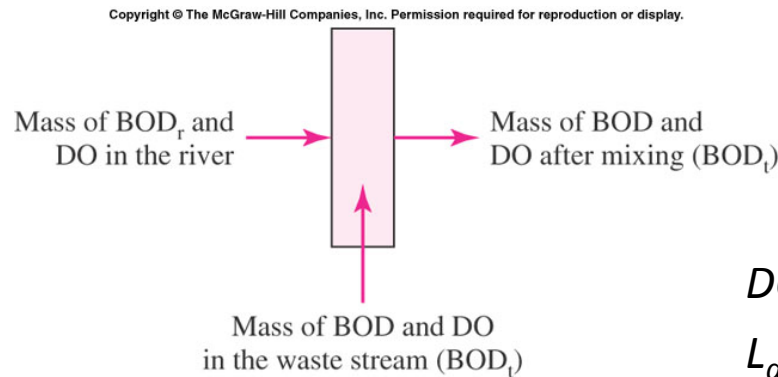


We will model the DO of a river receiving waste at time t_0 . The river will be modeled as a PFR.

* The solution for this problem is known as “Streeter-Phelps equation”, a well-known equation derived by Streeter and Phelps in 1925.

Modeling the DO along a river

The DO and ultimate BOD at t_0 are calculated by a mass balance approach:



$$(Q_w + Q_r)DO_a = Q_wDO_w + Q_rDO_r$$

$$(Q_w + Q_r)L_a = Q_wL_w + Q_rL_r$$

DO_a = DO concentration right after mixing (mg/L)

L_a = ultimate BOD right after mixing (mg/L)



$$DO_a = \frac{Q_wDO_w + Q_rDO_r}{Q_w + Q_r}$$

$$L_a = \frac{Q_wL_w + Q_rL_r}{Q_w + Q_r}$$

Modeling the DO along a river

The temperature after mixing is calculated in the same way:

$$T_a = \frac{Q_w T_w + Q_r T_r}{Q_w + Q_r} \quad T_a = \text{temperature after mixing (}^\circ\text{C or K)}$$

Oxygen deficit

- Oxygen deficit (D): the difference between the saturation DO value and the actual DO concentration

$$D = DO_s - DO$$

Therefore, the oxygen deficit right after mixing is calculated as:

$$D_a = DO_s - \frac{Q_w DO_w + Q_r DO_r}{Q_w + Q_r}$$

D_a = oxygen deficit right after mixing
(mg/L)

Modeling the DO along a river

- Rate of reaeration

- Should depend on the stream velocity and depth

- The reaeration coefficient, k_r [day⁻¹]

$$k_r = \frac{3.9u^{1/2}}{h^{2/3}} \quad \begin{array}{l} u = \text{average stream velocity (m/s)} \\ h = \text{average stream depth (m)} \end{array}$$

- Rate of reaeration should also depend on oxygen deficit

$$\text{Rate of reaeration} = \left. \frac{d(DO)}{dt} \right|_{\text{reaeration}} = - \left. \frac{dD}{dt} \right|_{\text{reaeration}} = k_r D$$

- Effect of temperature on k_r : faster mass transfer at higher temp.

$$k_{r,T} = k_{r,20} \theta^{T-20} \quad \begin{array}{l} k_{r,T} = \text{reaeration coeff. at temperature } T \text{ (day}^{-1}\text{)} \\ k_{r,20} = \text{reaeration coeff. at } 20^\circ\text{C, obtained from} \\ k_{r,20} = 3.9u^{1/2}/h^{2/3} \text{ (day}^{-1}\text{)} \\ \theta = \text{temperature coefficient (use 1.024)} \end{array}$$

Modeling the DO along a river

- Rate of deoxygenation
 - Rate of oxygen consumption by microorganisms
 - Assume that the first-order deoxygenation rate constant is equal to the BOD rate constant, k
 - The assumption is valid for deep, slow-moving streams
 - The rate of deoxygenation

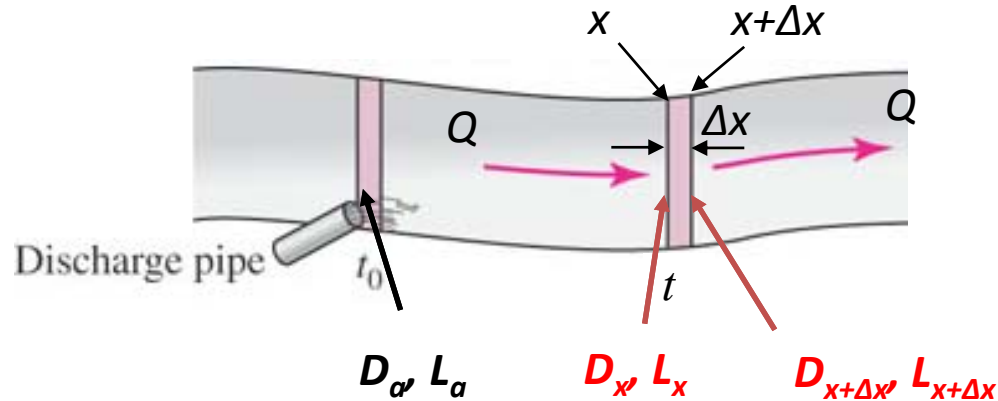
$$\begin{aligned} \text{Rate of deoxygenation} &= - \left. \frac{d(DO)}{dt} \right|_{\text{deoxygenation}} = \left. \frac{dD}{dt} \right|_{\text{deoxygenation}} \\ &= k_d L \end{aligned}$$

k_d = first-order deoxygenation rate constant [T⁻¹]

- Effect of temperature on k_d : use the equation for k !

$$k_T = k_{20} \theta^{T-20} \quad \theta = 1.135 \text{ for } 4\text{-}20^\circ\text{C and } 1.056 \text{ for } 20\text{-}30^\circ\text{C}$$

Modeling the DO along a river



Steady-state D ($=DO_s - DO$) balance for a thin plate at time t :

$$0 = QD_x - QD_{x+\Delta x} + k_d L_x \cdot \Delta V - k_r D_x \cdot \Delta V \quad \Delta V = \text{volume of the CV} = A \cdot \Delta x$$

(A = cross-sectional area)

With rearrangements and $\Delta x \rightarrow 0$, we obtain:

$$\frac{dD}{dt} = k_d L - k_r D$$

Modeling the DO along a river

Governing equation: $\frac{dD}{dt} = k_d L - k_r D$

+ *Boundary conditions:*

at $t=0$, $D=D_a$ and $L=L_a$

Solution:

$$D_t = \frac{k_d L_a}{k_r - k_d} (e^{-k_d t} - e^{-k_r t}) + D_a (e^{-k_r t})$$

D_t = oxygen deficit in a river after flowing downstream from the mixing point for time t

(Note $DO_t = DO_s - D_t$)

Critical point

- Critical point: the point where the DO is the lowest on the DO sag curve

$$t_c = \frac{1}{k_r - k_d} \ln \left[\frac{k_r}{k_d} \left(1 - D_a \frac{k_r - k_d}{k_d L_a} \right) \right]$$

t_c = the time to the critical point [T]

- The critical deficit, D_c

$$D_c = \frac{k_d L_a}{k_r - k_d} (e^{-k_d t_c} - e^{-k_r t_c}) + D_a (e^{-k_r t_c})$$

Modeling BOD

Q: A city disposes of $1.05 \text{ m}^3/\text{s}$ of treated sewage having ultimate BOD of 28.0 mg/L and DO of 1.8 mg/L into a river. At the upstream from the outfall, the river flowrate is $7.08 \text{ m}^3/\text{s}$, the velocity is 0.37 m/s , and the ultimate BOD and DO of the river are 3.6 and 7.6 mg/L , respectively. At the river temperature, the saturation value of DO is 8.5 mg/L , the deoxygenation coefficient, k_d , 0.61 day^{-1} , and the reaeration coefficient, k_r , 0.76 day^{-1} .

- 1) Calculate the ultimate BOD and DO just downstream from the outfall. Assume complete mixing.
- 2) Calculate the DO 16 km downstream from the outfall.
- 3) Calculate the critical time, distance, and the minimum DO.

Reading assignment

Textbook Ch 9 p. 392-418