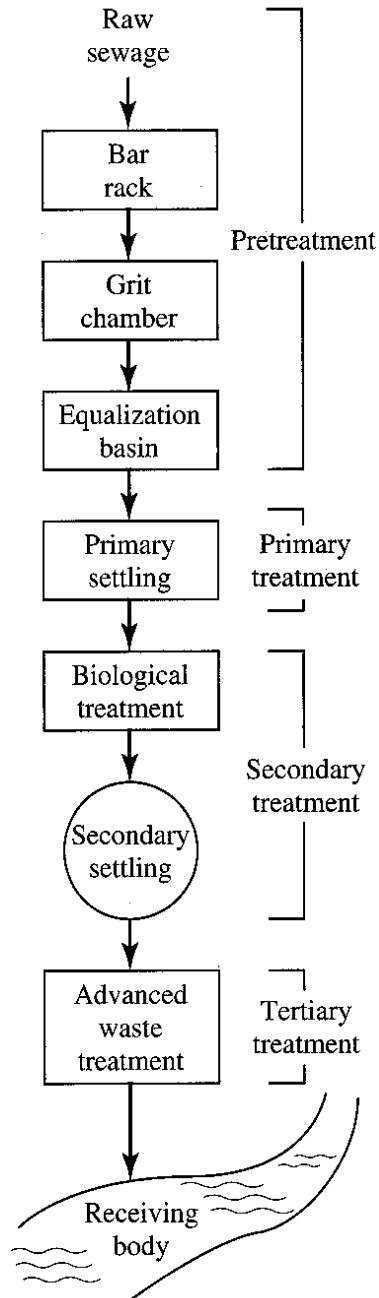


# Wastewater treatment processes

# Municipal wastewater treatment systems

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- Pretreatment: removes materials that can cause operational problems, equalization optional
- Primary treatment: remove ~60% of SS and ~35% of BOD
- Secondary treatment – remove ~85% of BOD and SS
- Advanced (tertiary) treatment – more BOD and SS, N, P, others

# Bar racks (screens)

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- Purpose: to remove large objects that would damage or foul pumps, valves, and other mechanical equipment



<http://www.infobarscreens.com>

# Grit chamber

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- Grits: inert dense materials such as sand, broken glass, silt, and pebbles
- Purpose: to remove grits that can abrade pumps and other mechanical devices



# Flow equalization

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- The flow rate and strength of wastewater varies from hour to hour
- High flow rate and strength in the morning, low at night; high flow rate and low strength during storm events
- Flow equalization is to achieve nearly constant wastewater flow rate and strength → better performance of wastewater treatment and reduce the size and cost

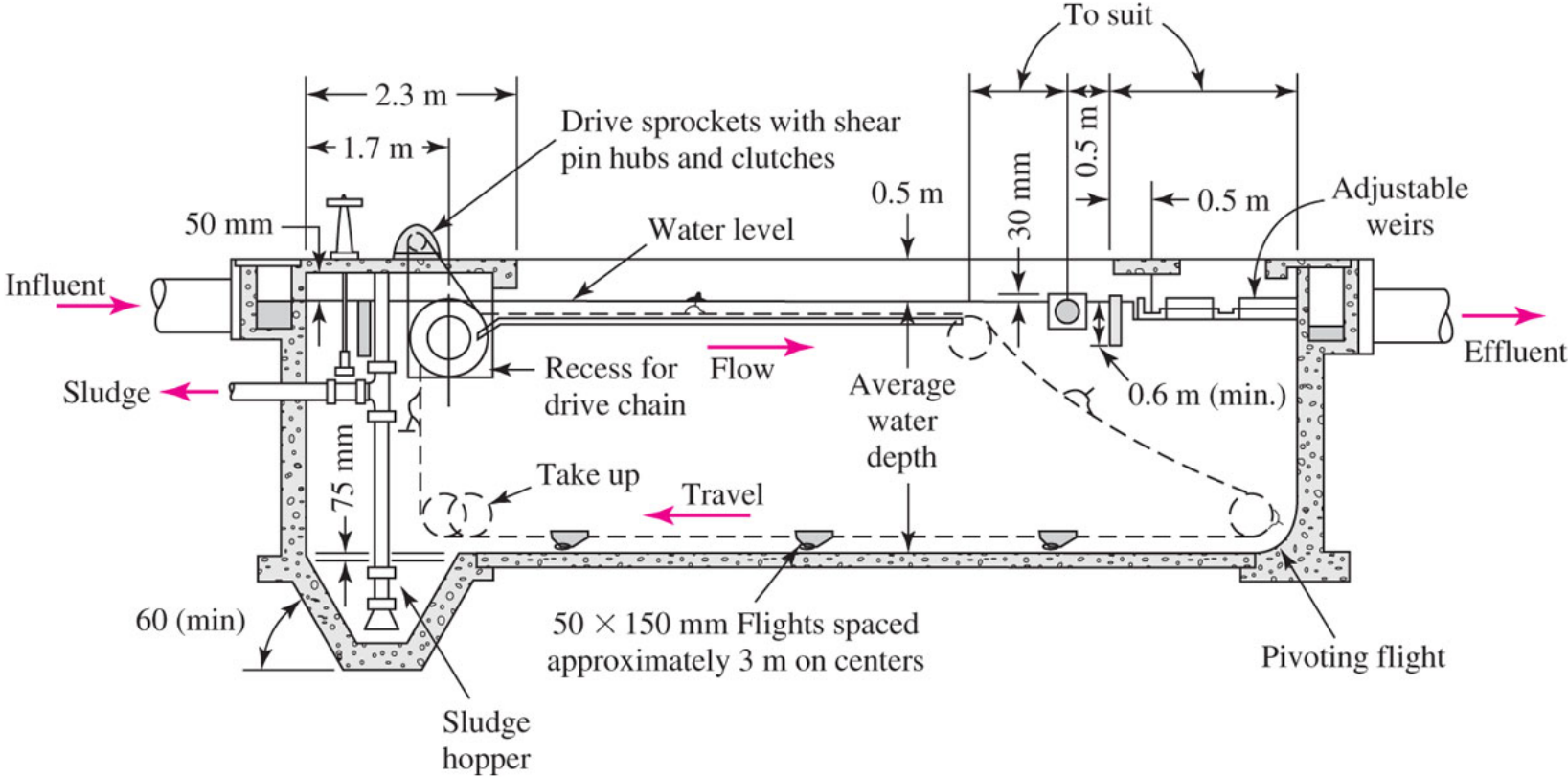
# Primary sedimentation basins

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- Removal of suspended solids by settling
- This removes some BOD as well!
- Removes ~60% of SS and ~35% of BOD
- Sludge settled at the bottom and collected by mechanical devices
- Floating materials such as oil and grease are also removed

# Primary sedimentation basins

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# Primary sedimentation basins

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- Design parameters
  - Retention time: ~2 hr
  - Overflow rate,  $v_o$

$$v_o = \frac{Q}{A_c}$$

$Q$  = water flow rate (m<sup>3</sup>/s)  
 $A_c$  = surface area of the sedimentation basin (m<sup>2</sup>)

- Weir loading,  $WL$

$$WL = \frac{Q}{L_{weir}}$$

$L_{weir}$  = weir length (m)

- \* Large, dense particles: better settling properties
  - higher  $v_o$  and  $WL$  allowed



# Primary sedimentation basins

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- Rectangular or circular



<http://www.mlive.com>



<http://www.lgam.info>

# Primary sedimentation basins

---

**Q:** Calculate the retention time, overflow rate, and weir loading of the primary sedimentation basin with following design parameters.

$$\text{Flow} = 0.150 \text{ m}^3/\text{s}$$

$$\text{Length} = 40.0 \text{ m}$$

$$\text{Width} = 10.0 \text{ m}$$

$$\text{Water depth} = 2.0 \text{ m}$$

$$\text{Weir length} = 75.0 \text{ m}$$

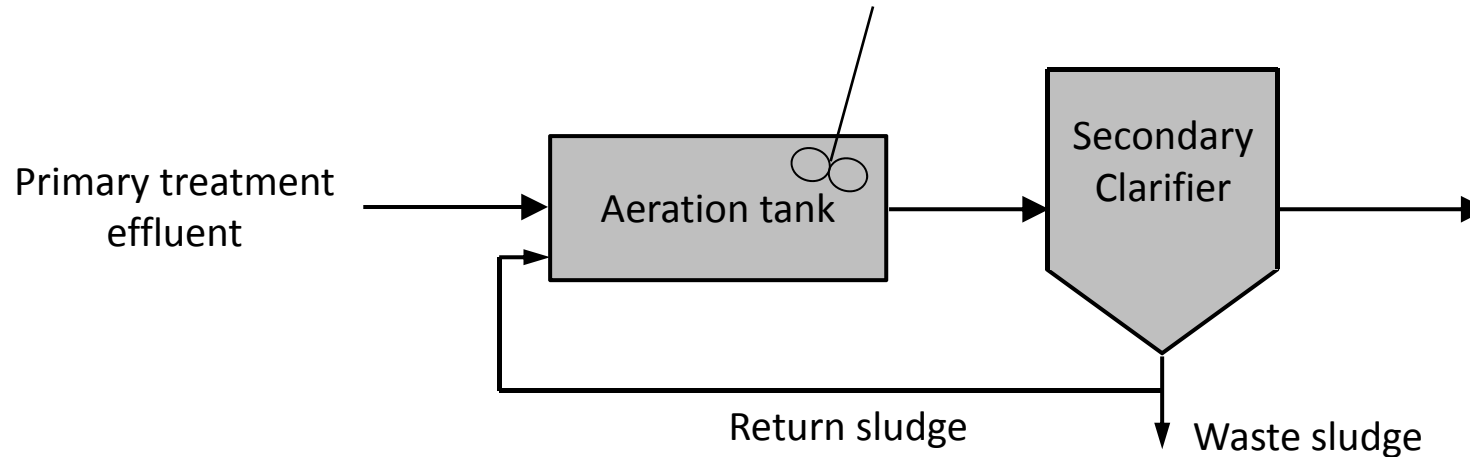
# Secondary treatment

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- Goal: provide BOD removal beyond what is achieved in primary treatment
  - Removal of soluble BOD
  - Additional removal of SS
- How: by providing favorable conditions for microbial activities
  - Availability of high density of microorganisms
  - Good contact between organisms and wastes
  - Favorable temperature, pH, nutrients, carbon source (food)
  - Oxygen (or other electron acceptors)
  - No or little toxic chemicals present

# Activated sludge process

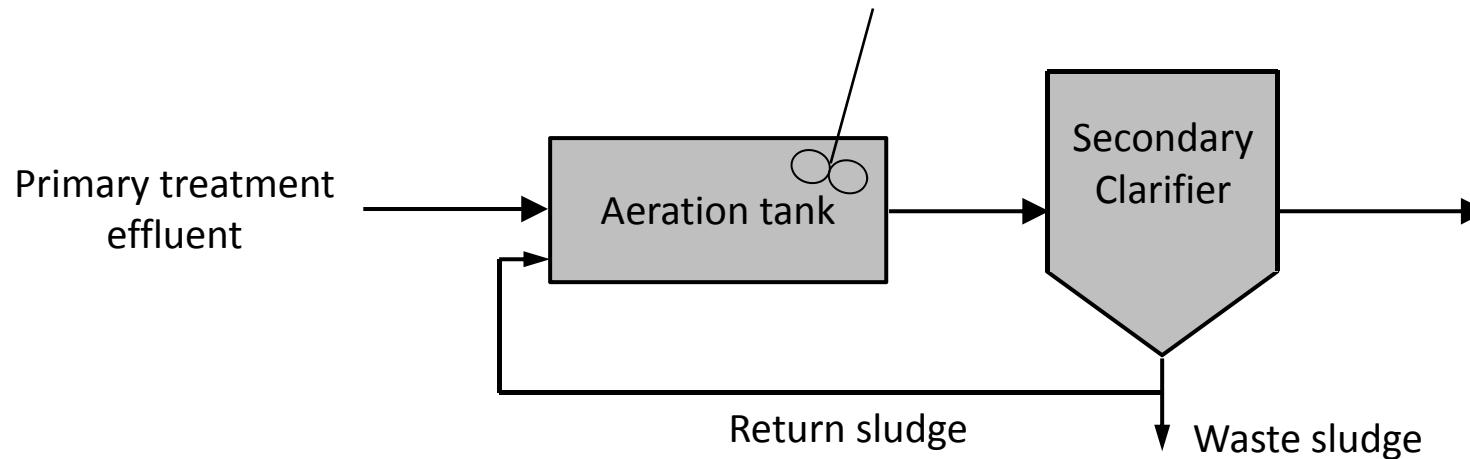
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- A biological wastewater treatment technique using suspended microorganisms (dispersed growth)
- Aeration tank: a mixture of wastewater and microorganisms is agitated and aerated
- Wastewater BOD is removed by active microorganisms

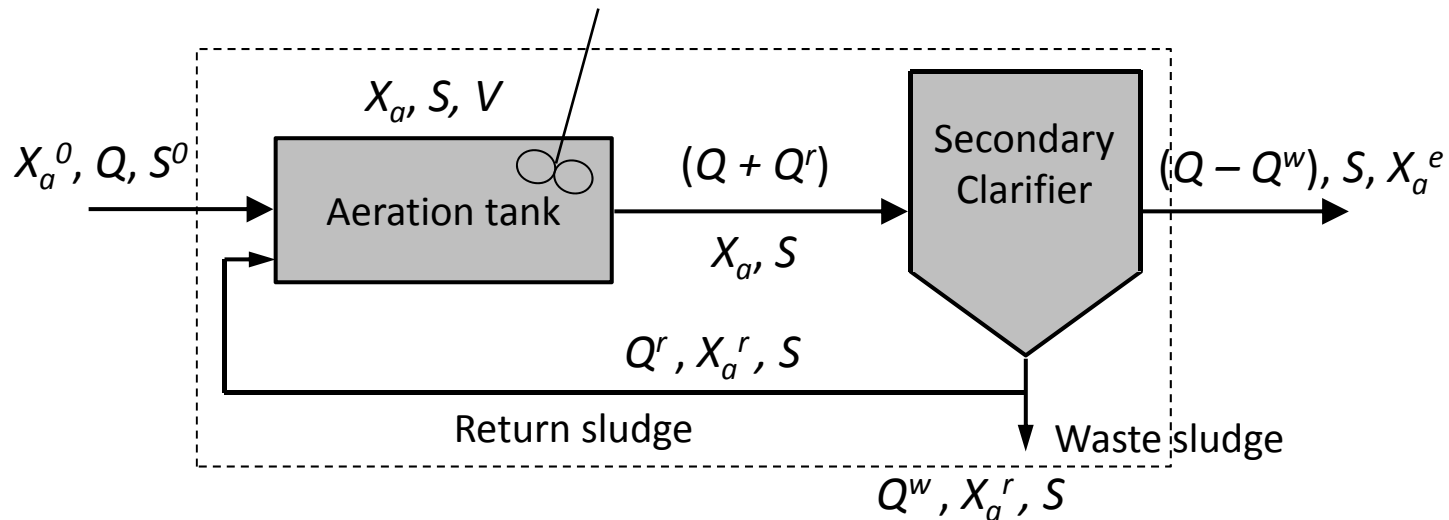
# Activated sludge process

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- Secondary clarifier: the microorganisms (also called biosolids or sludge) are separated from water by gravity
- Most of the settled sludge is returned to the aeration tank (Why? - We need a high population of microorganisms)
- A fraction of the settled sludge is wasted (Why? – microorganisms grow!)

# Analyzing activated sludge process



Assumption:

- i) Steady-state
- ii) The aeration tank is a CSTR
- iii) All reactions occur in the aeration tank

# Analyzing activated sludge process

---

- Mass balance for substrate:

$$0 = Q^0 S^0 - (Q^e S + Q^w S) + r_{ut} V$$

- Mass balance for microorganisms:

$$0 = 0 - (Q^e X_a^e + Q^w X_a^w) + [Y(-r_{ut})V - bX_a V]$$

This can be rearranged to:

$$\frac{Q^e X_a^e + Q^w X_a^w}{X_a V} = \frac{Y(-r_{ut})}{X_a} - b$$

# Analyzing activated sludge process


---

- Solids retention time (SRT),  $\theta_x$

$$\theta_x = \frac{X_a V}{Q^e X_a^e + Q^w X_a^w}$$

Therefore,

$$\frac{1}{\theta_x} = \frac{Y(-r_{ut})}{X_a} - b = Y \frac{\hat{q}S}{K + S} - b$$


$$S = K \frac{1 + b\theta_x}{\theta_x(Y\hat{q} - b) - 1}$$

We've seen this!!



# Analyzing activated sludge process

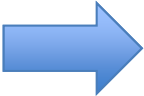
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- Mass balance for substrate:

$$0 = Q^0 S^0 - (Q^e S + Q^w S) + r_{ut} V$$

This can be rearranged to ( $Q^0 = Q^e + Q^w$ ):

$$-r_{ut} = \frac{Q^0(S^0 - S)}{V} = \frac{(S^0 - S)}{\theta}$$


$$X_a = \frac{\theta_x Y(S^0 - S)}{\theta (1 + b\theta_x)}$$

We've seen something similar to this!!

# Other important operation parameters

---

- Food-to-microorganism ratio (F/M)

$$F/M = \frac{Q^0 S^0}{VX}$$

$X$  = total suspended solids (MLSS) in aeration tank (mg/L)

- Volumetric organic loading rate (Volumetric OLR): the amount of BOD or COD applied to the aeration tank volume per day

$$\text{Volumetric OLR} = \frac{Q^0 S^0}{V}$$

# Other important operation parameters

---

- Sludge production,  $P_{X,VSS}$

$$\begin{aligned} P_{X,VSS} &= Y_{obs}(Q)(S^0 - S) + QX_i^0 \\ &= QY(S^0 - S) \frac{1 + (1 - f_d)b\theta_x}{1 + b\theta_x} + QX_i^0 \end{aligned}$$

$P_{X,VSS}$  = daily net sludge production (g VSS/d)

$Y_{obs}$  = observed yield (g VSS/g substrate)

- Oxygen requirements,  $R_o$

$$R_o = Q(S^0 - S) - 1.42P_{X,bio}$$

$P_{X,bio}$  = daily biomass production (g VSS/d);

$$= P_{X,VSS} - QX_i^0$$

# Biological nutrient removal

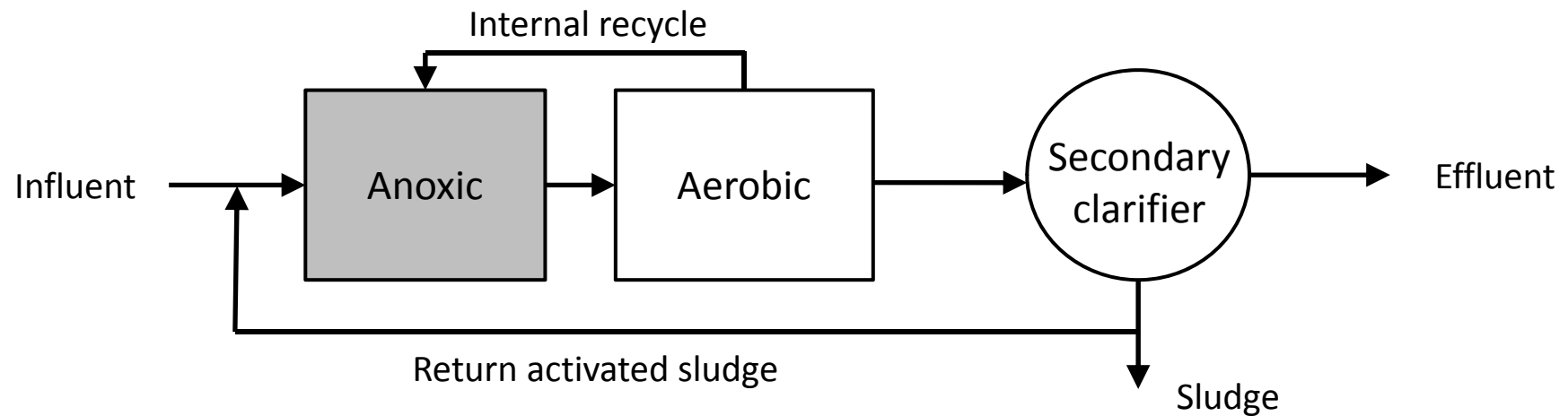
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- The secondary treatment process can be modified to improve nutrient removal by microorganisms
- Nitrogen removal
  - Nitrification ( $\text{NH}_4^+ \rightarrow \text{NO}_3^-$ ): needs high DO & low BOD
  - Denitrification ( $\text{NO}_3^- \rightarrow \text{N}_2$ ): needs low DO & some BOD
- Phosphorus removal
  - If microorganisms are exposed to alternating periods of anaerobic & aerobic conditions, they tend to accumulate excessive phosphorus at aerobic conditions

# Biological nutrient removal

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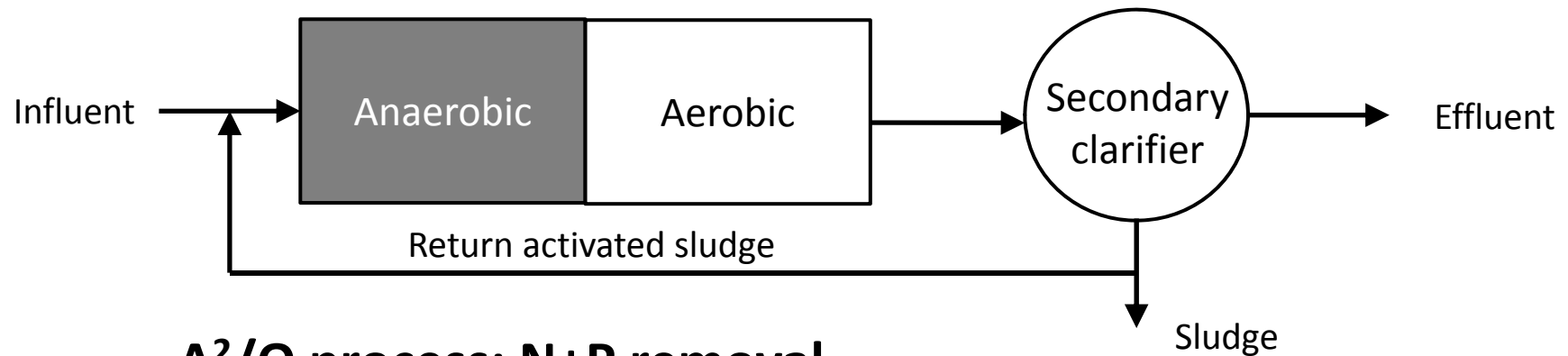
- Examples of modified secondary treatment processes for biological nutrient removal
  - **Modified Ludzack-Ettinger (MLE) process: N removal**



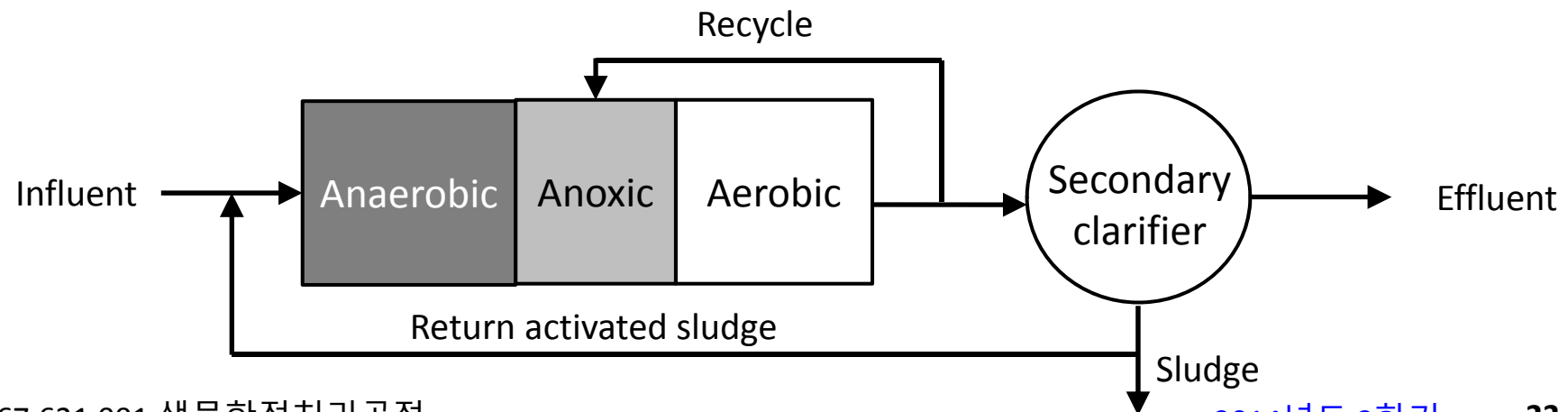
# Biological nutrient removal

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## – A/O process: P removal



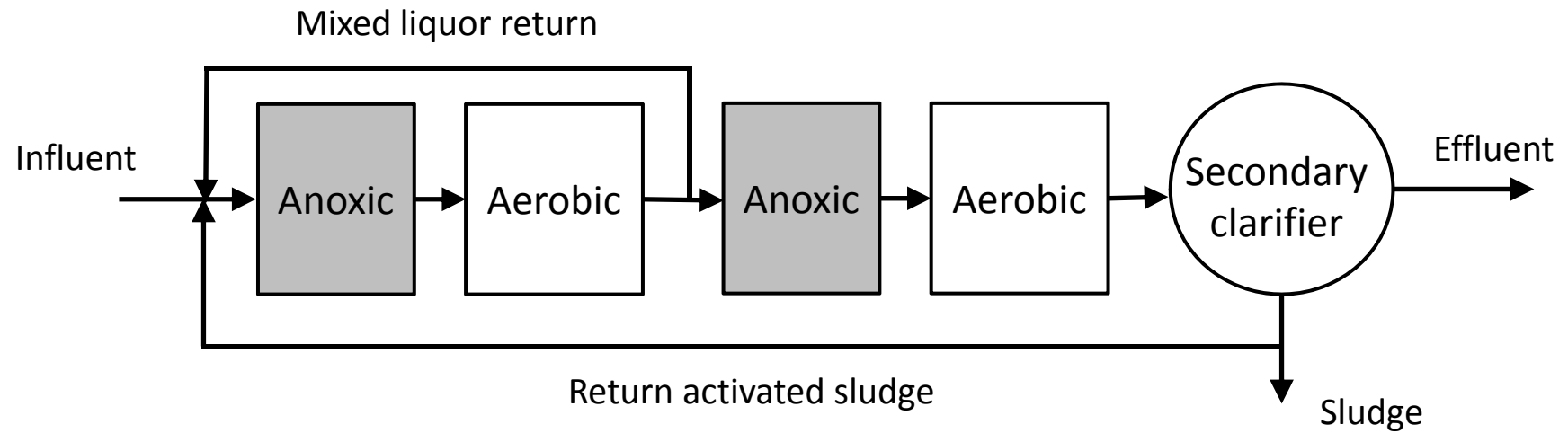
## – A<sup>2</sup>/O process: N+P removal



# Biological nutrient removal

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## – Bardenpho process: N+P removal



# Tertiary (advanced) treatment

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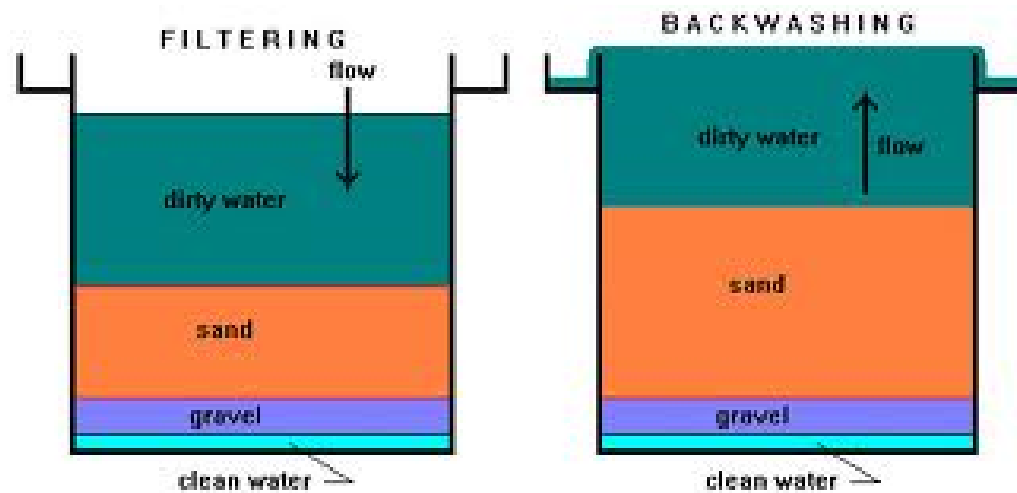
- Goal: to improve the quality of the secondary treatment effluent
- Many of the Korean wastewater treatment plants now have advanced treatment process
- Further BOD and SS removal, nutrient removal, TDS removal, or the removal of refractory organic compounds
- Different processes can be used depending on the major target



# Tertiary (advanced) treatment

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- Available tertiary treatment technologies
  - Granular filtration
    - Additional removal of SS
    - Sand is most frequently used

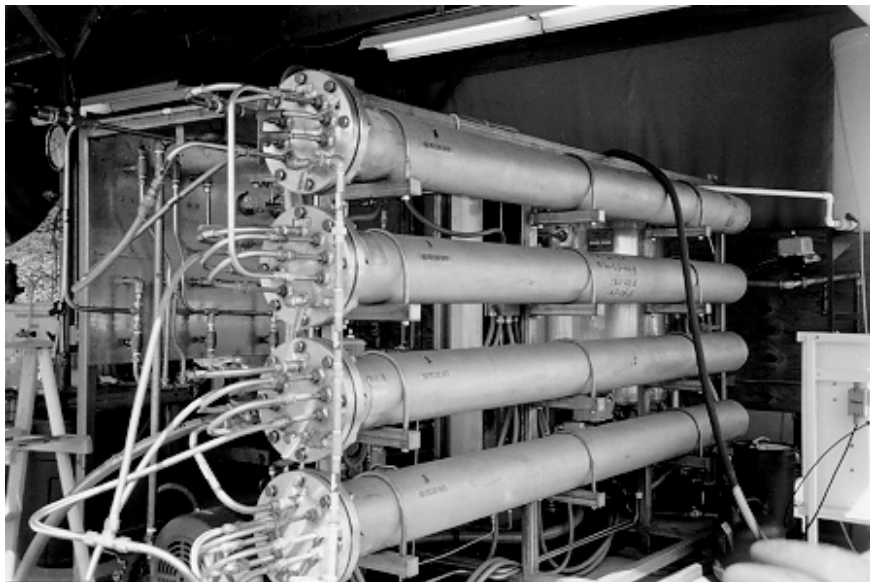


<http://www.rpi.edu>

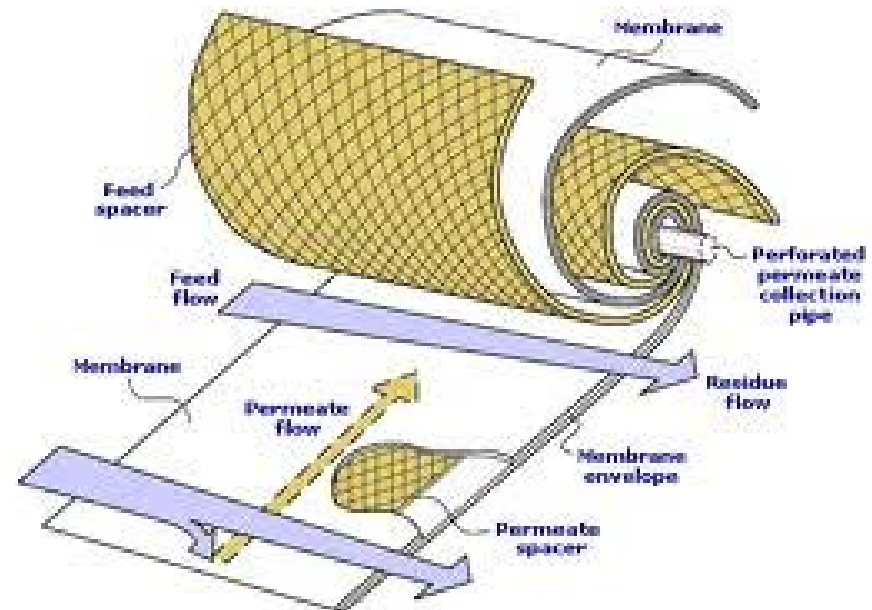
# Tertiary (advanced) treatment

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- Available tertiary treatment technologies
  - Membrane filtration: additional removal of SS



<http://www.clu-in.org>



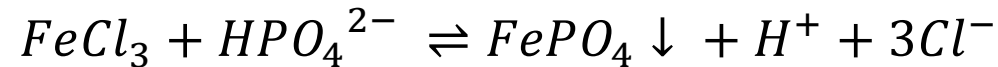
<http://www.onlinembr.info>

# Tertiary (advanced) treatment

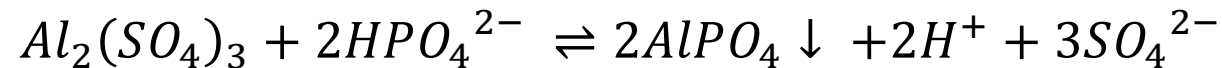
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- Available tertiary treatment technologies
  - Chemical phosphorus removal
    - Use chemicals (ferric chloride, alum, lime, ...) to precipitate P from secondary effluent

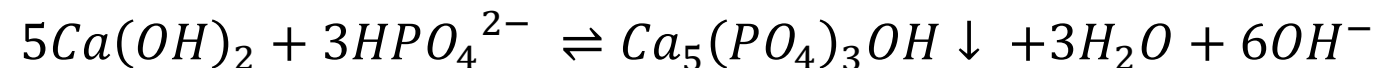
- Using ferric chloride:



- Using alum



- Using lime:



# Tertiary (advanced) treatment

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- Available tertiary treatment technologies
  - Activated carbon adsorption: removal of refractory organic compounds



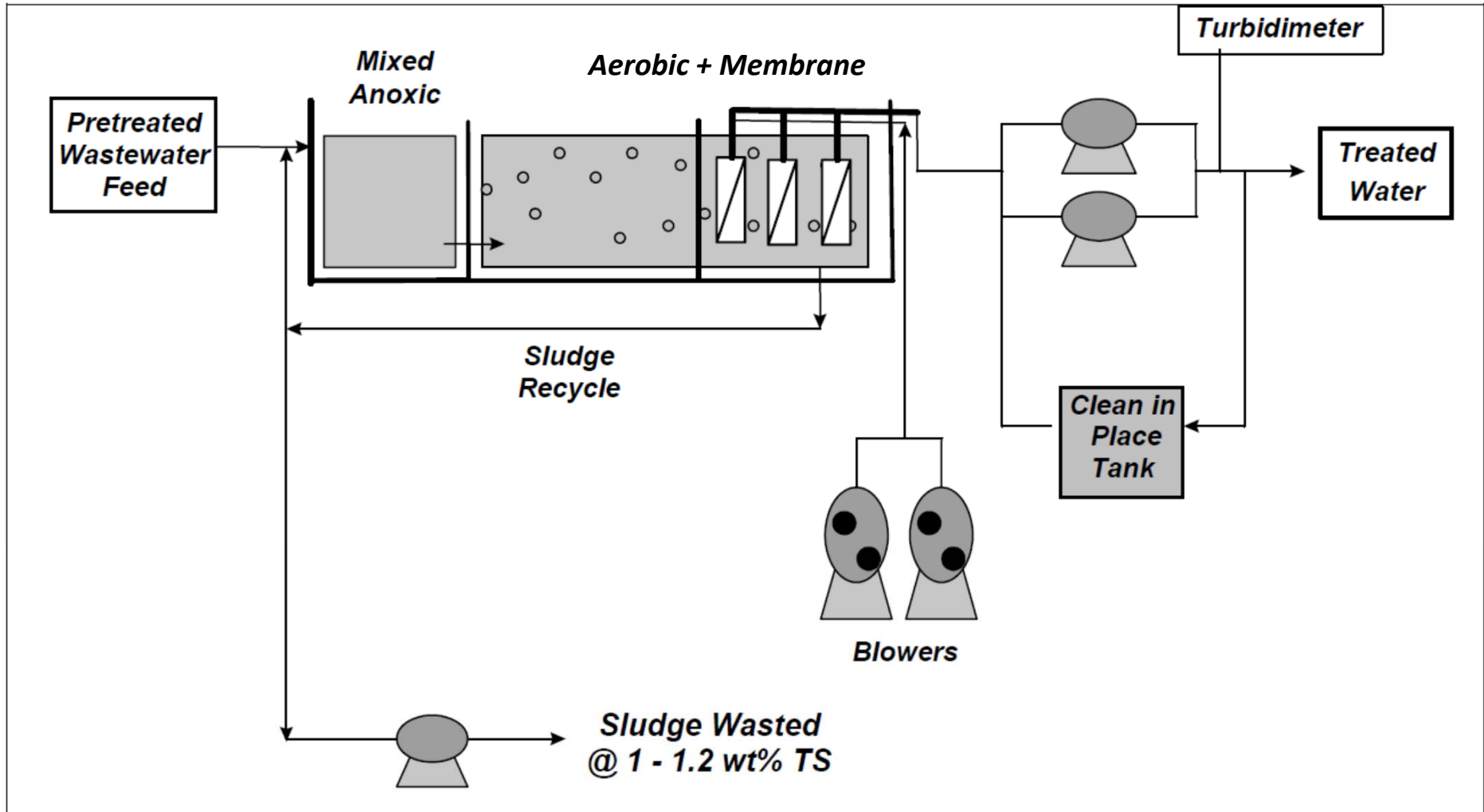
<http://www.chemvironcarbon.com>

# Membrane bioreactors

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- Concept
  - Utilizing a suspended-growth bioreactor and microfiltration as one unit process
  - Effect: secondary treatment (aeration tank + clarifier) + tertiary treatment (granular media filtration)
  - High removal efficiencies of BOD, SS, bacteria, and nutrients can be obtained

# Membrane bioreactors - example



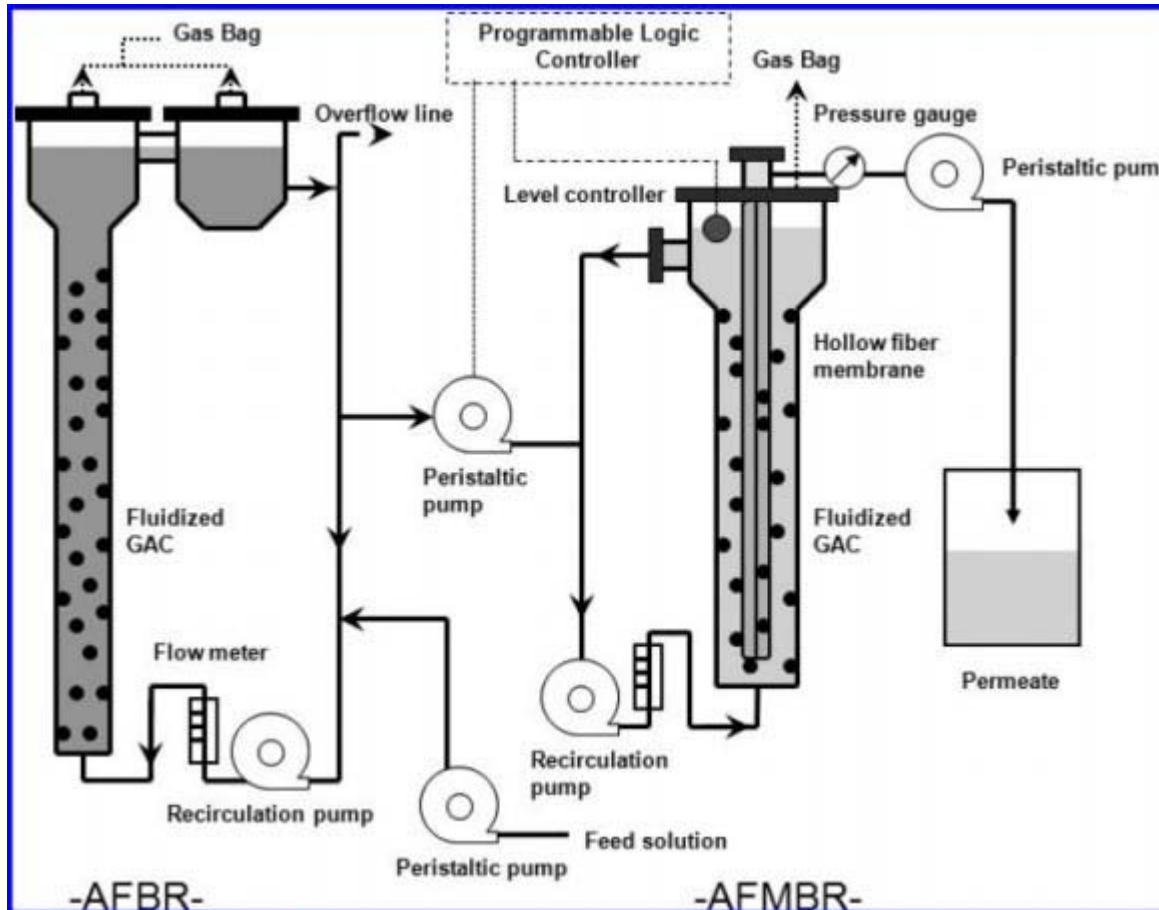
From USEPA

# Membrane bioreactors

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- Advantages
  - Better effluent quality (can be directly reused)
  - Smaller space requirements (can maintain higher biomass concentration → higher volumetric OLR, with appropriate F/M → lower HRT → smaller reactor volume)
- Disadvantages
  - Higher capital & operating costs
    - Operating costs for membrane cleaning, fouling control, and replacement
    - Energy costs for air scouring
    - Becoming more and more economically viable with the advances in membrane technologies

# Membrane bioreactors



## Anaerobic fluidized bed membrane bioreactor (AFMBR)

- GAC scours the membrane → fouling control
- Additional organic removal by GAC
- Methane (fuel) production

Kim et al. (2011)