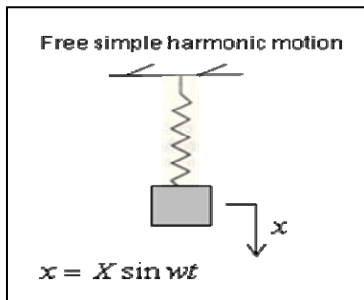


Wave propagation (overview)

Rayleigh`s method [Linear conservative system (no damping)]

- The equation of Energy for a vibrating system



- the original Eq. : $\ddot{x} + \frac{k}{m}x = 0$

- multiply by \dot{x} : $\ddot{x}\dot{x} + \frac{k}{m}x\dot{x} = 0$

- integration of the Eq. : $\frac{1}{2}(\dot{x})^2 + \frac{1}{2}\frac{k}{m}x^2 = c$ (c : integral constant)

$$\rightarrow \underbrace{\frac{1}{2}m(\dot{x})^2}_{\text{Instantaneous kinetic Energy}} + \underbrace{\frac{1}{2}kx^2}_{\text{Instantaneous potential Energy}} = cm$$

- When the displacement x is a maximum X

The velocity $\dot{x} = 0 \rightarrow$ all the dynamic energy in the system is potential

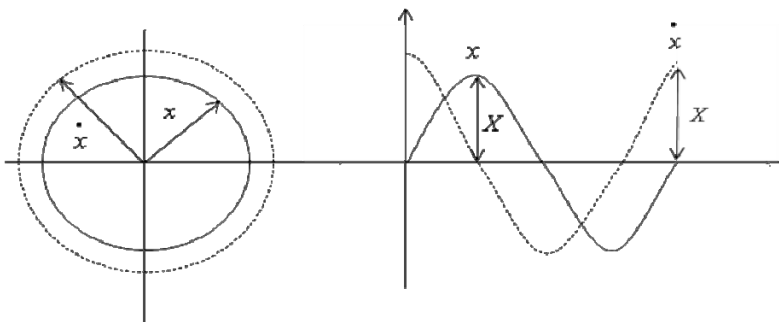
$$\rightarrow \frac{1}{2}kX^2 = cm = P$$

- When the displacement x is zero, the velocity \dot{x} will be a maximum \dot{X}

\rightarrow all the d.e. is kinetic

$$\rightarrow \frac{1}{2}k\dot{X}^2 = cm$$

- Thus, $\frac{1}{2}m(\dot{x})^2 + \frac{1}{2}kx^2 = \frac{1}{2}kX^2 = \frac{1}{2}m(\dot{X})^2 = P$



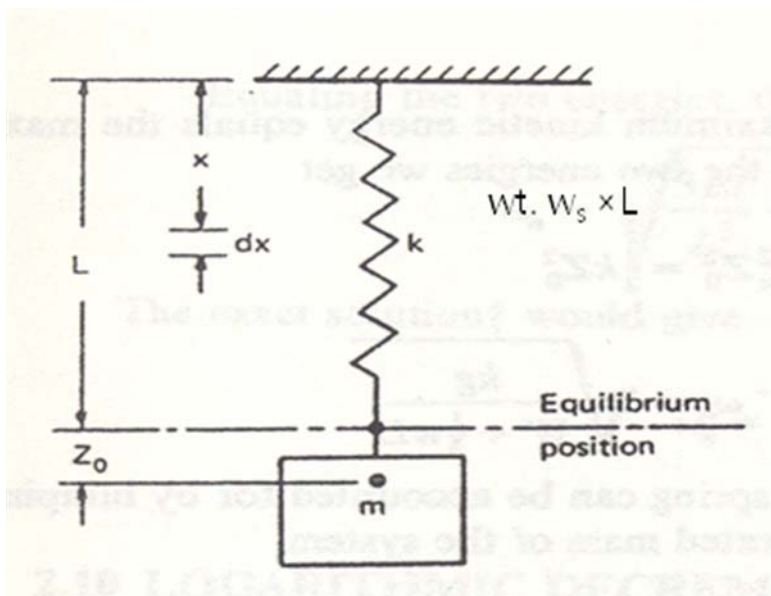
$$x = X \sin wt$$

$$\dot{x} = wX \cos wt (\dot{X} = wX)$$

- Rayleigh`s principles

- The fundamental natural frequency as calculated from the assumed shape of a dynamic deflection curve of a system will be equal to or higher than the system`s true natural frequency
(upper bound nature ↔ Southwell-Dunkerley Method)
- Small departures from the shape of the true dynamic – deflection curve will not be critical in the determination of the system`s w_n
(justifies the use of the static deflection curve)

Ex. 1. Natural frequency of the spring-mass system (spring w/ its weight)



- maximum KE $(\frac{1}{2}m(\dot{X})^2)$

Spring: the displacement of spring at distance x

- $z = \frac{x}{L} Z_0 \cos w_n t$ (← assumed the extension of the spring is linear)

- Velocity of element dx , $\dot{z} = -\frac{x}{L} w_n Z_0 \sin w_n t$

- Max. KE of the element (mass = $(\frac{W_s}{g})dx$)

$$d(KE)_{\max} = \frac{W_s}{2g} dx \left(\frac{x}{L} w_n Z_0 \right)^2 \leftarrow \left[\frac{1}{2} \cdot \frac{W_s}{g} dx \cdot \left(\frac{x}{L} w_n Z_0 \right)^2 \right]$$

- For the whole spring

$$\begin{aligned}(KE)_{\max} &= \frac{w_s}{2g} \left(\frac{w_n Z_0}{L} \right)^2 \int_0^L x^2 dx \\ &= \frac{1}{2} \frac{w_s L}{3g} w_n^2 Z_0^2\end{aligned}$$

- mass :

$$(KE)_{\max} = \frac{1}{2} m (\dot{Z})^2 = \frac{1}{2} \frac{W_m}{g} (w_n Z_0)^2$$

- The total KE = $\frac{1}{2} \left(\frac{W_m + \frac{1}{3} w_s L}{g} \right) w_n^2 Z_0^2$

- maximum PE $\left(\frac{1}{2} kX^2 \right)$

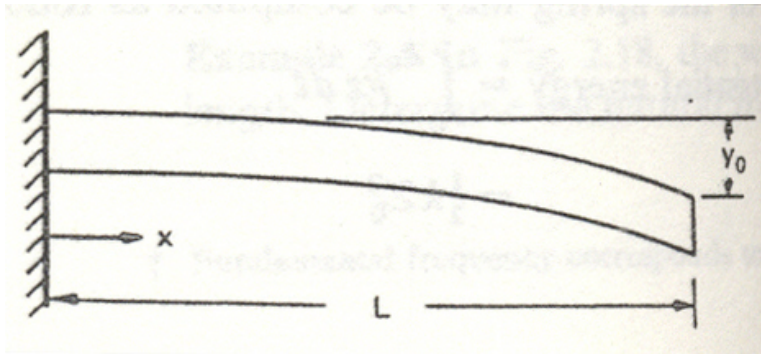
$$(PE)_{\max} = \frac{1}{2} kX^2 = \frac{1}{2} kZ_0^2$$

- $(KE)_{\max} = (PE)_{\max}$

$$\frac{1}{2} \left(\frac{W_m + \frac{1}{3} w_s L}{g} \right) w_n^2 Z_0^2 = \frac{1}{2} kZ_0^2$$

$$\therefore w_n = \sqrt{\frac{kg}{W_m + \frac{1}{3} w_s L}}$$

Ex. 2 Cantilever beam



$$z = y \cos w_n t$$

$$\dot{z} = -w_n y \sin w_n t$$

Assume the dynamic deflection curve of the beam as a static deflection curve of a weightless beam w/ the concentrated load P acting at its end.

- $(PE)_{\max}$

$$\rightarrow y_0 = \frac{PL^3}{3EI} \rightarrow (k_{eq}) \text{ at the free end is } \left(\frac{L^3}{3EI} \right)$$

$$\rightarrow (PE)_{\max} = \frac{1}{2} k X^2 = \frac{1}{2} k y_0^2 = \frac{3EI}{2L^3} y_0^2$$

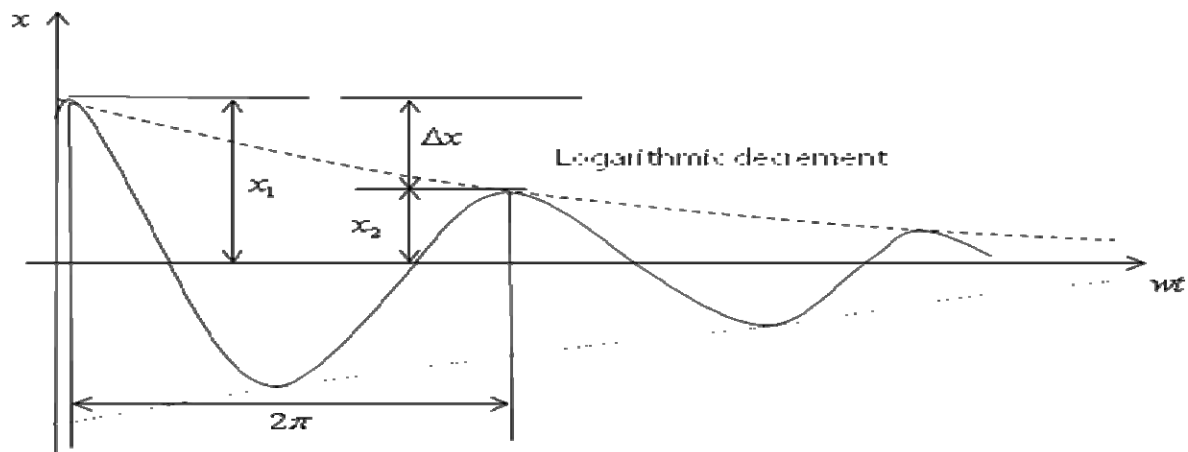
- $(KE)_{\max}$

$$\rightarrow \frac{1}{2} m (\dot{X})^2 = \frac{1}{2} \frac{W}{g} \int_0^L (w_n y)^2 dx$$

$$= \frac{w}{2g} \left(\frac{w_n y_0}{2} \right)^2 \int_0^L \left[3 \left(\frac{x}{L} \right)^2 - \left(\frac{x}{L} \right)^3 \right]^2 dx$$

$$= \frac{1}{2} \left(\frac{33wL}{140g} \right) w_n^2 y_0^2$$

$$- w_n = 3.56 \sqrt{\frac{gEI}{wL^4}} \quad (\text{cf. } (w_n)_{\text{exact}} = 3.515 \sqrt{\frac{gEI}{wL^4}})$$

Logarithmic Decrement

$$- \delta = \log_e \frac{x_1}{x_2}, \quad x_1 \text{ \& } x_2 : \text{ successive peak amplitudes}$$

$$\text{And } z = Z_0 \exp\left(\frac{-\xi w_{nd} t}{\sqrt{1-\xi^2}}\right) \sin(w_{nd} t + \phi)$$

If x_1 is the amplitude at $w_{nd} t_1$, then x_2 , at $(w_{nd} t_1 + 2\pi)$

$$\begin{aligned} \rightarrow \delta &= \log_e \frac{\exp\left(\frac{-\xi w_{nd} t_1}{\sqrt{1-\xi^2}}\right)}{\exp\left(\frac{-\xi (w_{nd} t_1 + 2\pi)}{\sqrt{1-\xi^2}}\right)} \\ &= \log_e \exp\left(\frac{2\pi\xi}{\sqrt{1-\xi^2}}\right) \\ &= \frac{2\pi\xi}{\sqrt{1-\xi^2}} \\ &\approx 2\pi\xi \text{ (if } \xi \ll 1) \end{aligned}$$

※ quite often $\xi \leq 0.1$ in practice

- In this case ($\xi \ll 1$)

$$\frac{x_0}{x_1} = \frac{x_1}{x_2} = \dots = \frac{x_{n-1}}{x_n} = e^\delta = e^{2\pi\xi}$$

$$\& \frac{x_0}{x_n} = \frac{x_0}{x_1} \cdot \frac{x_1}{x_2} \cdot \dots \cdot \frac{x_{n-1}}{x_n} = (e^\delta)^n$$

$$\rightarrow \delta = \frac{1}{n} \log_e \frac{x_0}{x_n}$$

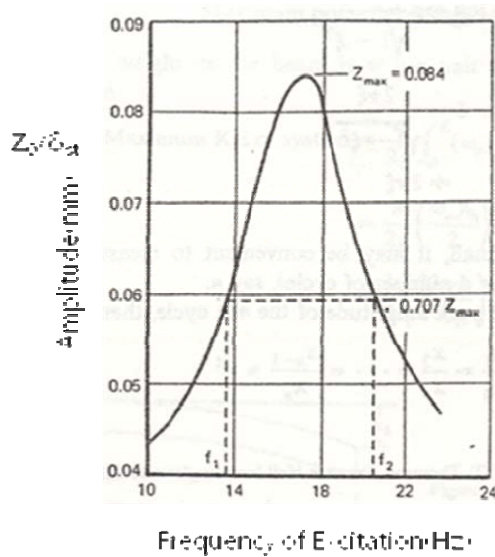
Determination of viscous Damping

- In a free vibration test,

$$\xi = \frac{\delta}{2\pi} = \frac{1}{2\pi} \log_e \frac{x_1}{x_2}$$

or $\xi = \frac{\delta}{2\pi} = \frac{1}{2n\pi} \log_e \frac{x_0}{x_n}$

- In a forced vibration test, [varying frequencies obtain a resonance curve]



$$\frac{Z_0}{\delta_{st}} = \frac{1}{\sqrt{(1-r^2)^2 + (2\xi r)^2}} = N \text{ (magnification factor)}$$

If $r = \frac{\omega}{\omega_n} = 1$ (at resonance)

$$\frac{Z_0}{\delta_{st}} = \frac{1}{2\xi} (N_{max}), (\because \xi \ll 1) \text{ n practice}$$

(Fig 2.20)

- When the magnification factor of motion is $\frac{1}{\sqrt{2}} (\frac{1}{2\xi})$, the frequency ratio, r is,

$$\frac{1/\sqrt{2}}{2\xi} = \frac{1}{\sqrt{(1-r^2)^2 + 4\xi^2 r^2}}$$

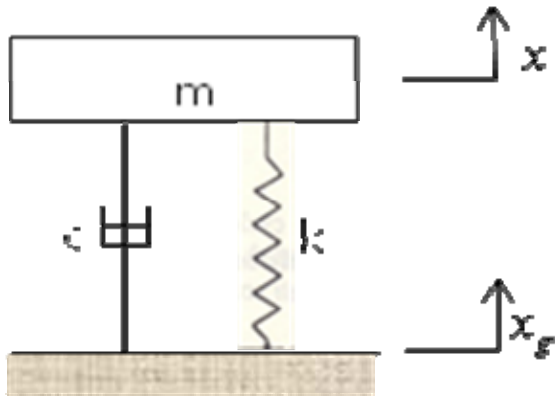
$$\rightarrow r_2^2 - r_1^2 = 4\xi \sqrt{1-\xi^2} \cong 4\xi \text{ (if } \xi \ll 1)$$

And $r_2^2 - r_1^2 = \frac{f_2^2 - f_1^2}{f_n^2} = (\frac{f_2 + f_1}{f_n})(\frac{f_2 - f_1}{f_n})$

$$\cong 2 \cdot (\frac{f_2 - f_1}{f_n}) \text{ - refer to Fig 2.20}$$

$$\rightarrow 4\xi = 2 \cdot (\frac{f_2 - f_1}{f_n}) \rightarrow \xi = \frac{1}{2} \cdot (\frac{f_2 - f_1}{f_n})$$

✖ called 'bandwidth' method

Ground Acceleration

$$\begin{aligned}
 - \sum F &= ma \\
 -c(\dot{x} - \dot{x}_g) - k(x - x_g) &= m\ddot{x} \\
 \rightarrow m\ddot{x} + c(\dot{x} - \dot{x}_g) + k(x - x_g) &= 0 \quad \dots \textcircled{1}
 \end{aligned}$$

- let $x_d = x - x_{g1}$

Then, $\dot{x}_d = \dot{x} - \dot{x}_g$

$$\ddot{x}_d = \ddot{x} - \ddot{x}_g \rightarrow \ddot{x}_d = \ddot{x}_d - \ddot{x}_g \quad \dots \textcircled{2}$$

- Sub. Eq. ② into ①

$$m(\ddot{x}_d + \ddot{x}_g) + c\dot{x}_d + kx_d = 0$$

$$\rightarrow \boxed{m\ddot{x}_d + c\dot{x}_d + kx_d = -m\ddot{x}_g} \quad \leftarrow \text{Govern Eq.} \quad \dots \textcircled{3}$$

- If $x_g = x_g(t) = X_g \sin wt$

$$\rightarrow \ddot{x}_g(t) = -w^2 X_g \sin wt$$

$$\rightarrow m\ddot{x}_d + c\dot{x}_d + kx_d = mw^2 X_g \sin wt$$

- Solution :

$$x_d = X_d \sin(wt - \phi)$$

$$X_d = \frac{mw^2 X_g}{\sqrt{(k - mw^2)^2 + (cw)^2}}, \quad \phi = \tan^{-1} \frac{cw}{k - mw^2}$$

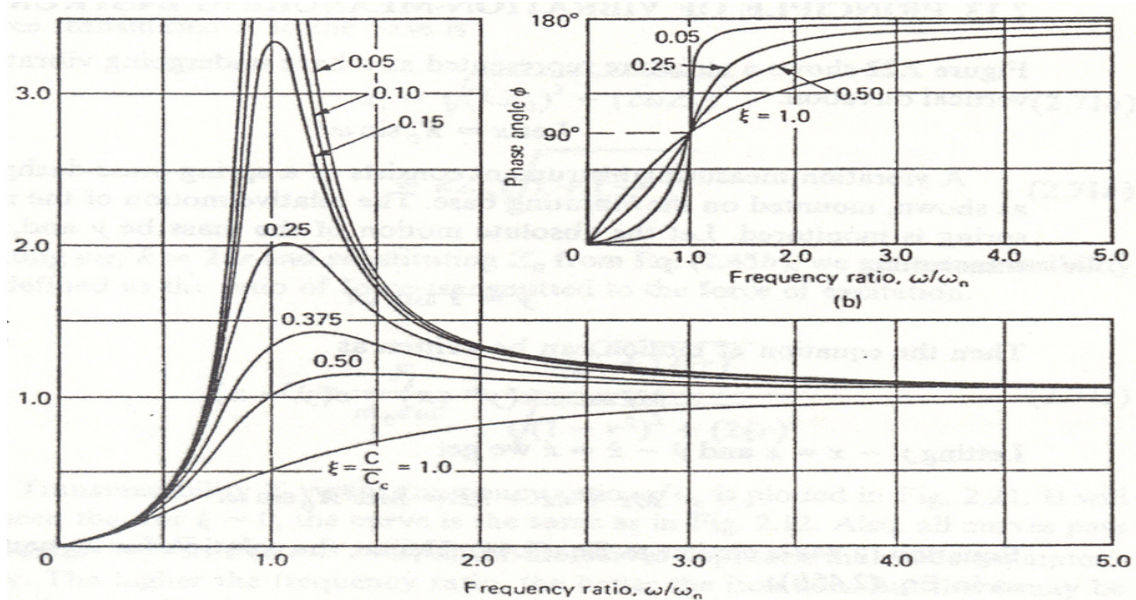
$$X_d / X_g = \frac{(w/w_n)^2}{\sqrt{(1 - r^2)^2 + (2\xi r)^2}} \quad \dots \textcircled{4}, \quad \phi = \tan^{-1} \frac{2\xi r}{1 - r^2}$$

The Governing Eq. of the system of vibration measuring instrument is same with Eq. ③

- Displacement pickup

→ For large value of $w/w_n (= r)$

$X_d / X_g = 1$ for all values of damping (refer to Fig. 2.23)



→ i.e. the resulting relative motion $X_d = X_g (X_{base})$

Picked up by the instrument

- Acceleration pickup

→ Rearrange Eq. ④

$$\frac{X_d}{w^2 X_b} = \frac{1}{w_n^2 \sqrt{(1 - (w/w_n)^2)^2 + [2\xi(w/w_n)]^2}} = \frac{1}{w_n^2 \sqrt{D}}$$

For $\xi = 0.69 \rightarrow \sqrt{D} = 1$ when w/w_n is small

w/w_n	0.0	0.1	0.2	0.3	0.4
\sqrt{D}	1.000	0.9995	0.9989	1.0000	1.0053

→ i.e. $X_d \propto w^2 X_b$ (max. acceleration of the vibrating base)

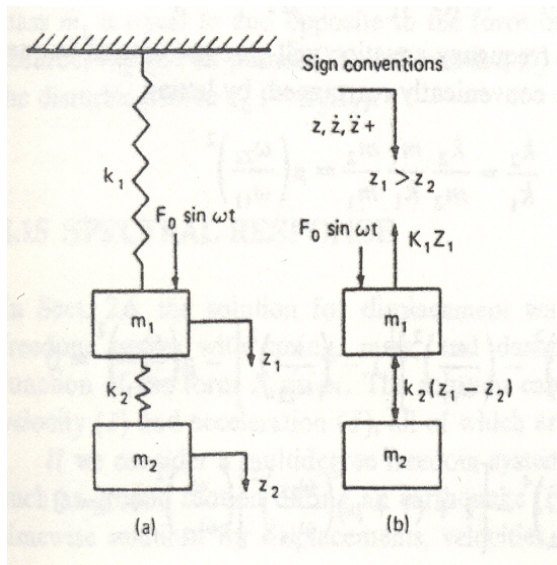
$$X_d = \frac{1}{w_n^2} \cdot w^2 X_b$$

$$x = X_b \sin wt$$

$$\dot{x} = w X_b \cos wt$$

$$\ddot{x} = -w^2 X_b \sin wt \rightarrow (\ddot{x})_{\max} = |w^2 X_b|$$

System w/ Two-degrees of Freedom



- mass 1

$$m_1 \ddot{z}_1 = F_0 \sin \omega t - k_1 z_1 - k_2(z_1 - z_2) \dots \textcircled{1}$$

- mass 2

$$m_2 \ddot{z}_2 = k_2(z_1 - z_2) \dots \textcircled{1}'$$

- assume (∴ vibrating frequency = exciting frequency)

$$z_1 = Z_1 \sin \omega t$$

$$z_2 = Z_2 \sin \omega t$$

- Sub. These into Eq. ①, ①'

$$(-m_1 \omega^2 + k_1 + k_2)Z_1 - k_2 Z_2 = F_0 \dots \textcircled{2}$$

$$(-m_2 \omega^2 + k_2)Z_2 - k_2 Z_1 = 0 \dots \textcircled{2}'$$

- Let the natural circular frequencies of the systems 1 & 2 be

$$\omega_{11} = \sqrt{\frac{k_1}{m_1}}, \quad \omega_{22} = \sqrt{\frac{k_2}{m_2}}$$

& $Z_0 = \frac{F_0}{k_1}$ (i.e. the static deflection of the mass m_1 due to F_0)

- Dividing Eq. ② & ②' by k_1 & k_2 respectively,

$$\left[1 + \frac{k_1}{k_2} - \left(\frac{\omega}{\omega_{11}}\right)^2\right]Z_1 - \frac{k_2}{k_1}Z_2 = Z_0 \dots \textcircled{3}$$

$$-Z_1 + \left[1 - \left(\frac{\omega}{\omega_{22}}\right)^2\right]Z_2 = 0 \dots \textcircled{3}'$$

- From Eq. ③', we get

$$Z_2 = \frac{Z_1}{1 - (w/w_{22})^2}$$

- Sub. Into Eq. ③

$$\left[1 + \frac{k_2}{k_1} - \left(\frac{w}{w_{11}}\right)^2\right]Z_1 - \frac{k_2}{k_1} \frac{Z_1}{\left[1 - \left(\frac{w}{w_{22}}\right)^2\right]} = Z_0$$

- Therefore,

$$\frac{Z_1}{Z_0} = \frac{1 - \left(\frac{w}{w_{22}}\right)^2}{\left[1 - \left(\frac{w}{w_{22}}\right)^2\right]\left[1 + \frac{k_2}{k_1} - \left(\frac{w}{w_{11}}\right)^2\right] - \frac{k_2}{k_1}} \dots \text{④}$$

Similarly, we get

$$\frac{Z_2}{Z_0} = \frac{1}{\left[1 - \left(\frac{w}{w_{22}}\right)^2\right]\left[1 + \frac{k_2}{k_1} - \left(\frac{w}{w_{11}}\right)^2\right] - \frac{k_2}{k_1}} \dots \text{④}'$$

- From Eq. ④

$$\text{If } w_{22} = w \rightarrow Z_1 = 0$$

$$\text{then } \rightarrow Z_2 = -Z_0 \frac{k_1}{k_2} = -\frac{F_0}{k_2}$$

✘ the system 2 is used as a vibration absorber for the main system 1

- the natural frequencies of the system

(consider the free vibration \rightarrow R.H.S of ③ = 0)

From Eq. ③ & ③'

$$\frac{Z_1}{Z_2} = \frac{k_2/k_1}{[1 + k_2/k_1 - (w/w_{11})^2]} = 1 - \left(\frac{w}{w_{22}}\right)^2$$

$$\text{or} \quad \left[1 + \frac{k_2}{k_1} - \left(\frac{w}{w_{11}}\right)^2\right] \left[1 - \left(\frac{w}{w_{22}}\right)^2\right] - \frac{k_1}{k_2} = 0 \quad [\text{Frequency equation}]$$

$$\text{or, since } \frac{k_2}{k_1} = \frac{k_2}{m_2} \frac{m_1}{k_1} \frac{m_2}{m_1} = \mu \left(\frac{w_{22}}{w_{11}}\right)^2 \text{ where, } \mu = \frac{m_2}{m_1}$$

$$\left[\frac{w_{22}}{w_{11}}\right]^2 \left(\frac{w}{w_{22}}\right)^4 - \left[1 + (1 + \mu) \left(\frac{w_{22}}{w_{11}}\right)^2\right] \left(\frac{w}{w_{22}}\right)^2 + 1 = 0$$

Spectral Response

- Response quantities : the quantities that are examined as a result of a forcing function (may or may not be a mathematical function)

Ex. Displacement, velocity, acceleration, stress, strain.....

- maximum response of a multidegree freedom system

$$X = \sqrt{(A_{11}X_1)^2 + (A_{22}X_2)^2 + \dots + (A_{mm}X_m)^2}$$

where, A_{11}, A_{22}, \dots : the mode participation factor ($A_{11} > A_{22} > \dots$)

X_1 : the maximum response of the 1st mode

Mode participation factor depends on

- 1) type of system
- 2) boundary condition
- 3) type of response quantity

- e.g., Longitudinal vibration of Prismatic Bar

$$u = \frac{-4Pl}{\pi^2 AE} \sum_{n=1,3,5,\dots}^{\infty} \frac{1}{n^2} \cos \frac{n\pi x}{l} \cos w_n t$$

$$\sigma = \frac{4P}{\pi A} \sum_{n=1,3,5,\dots}^{\infty} \frac{1}{n} \sin \frac{n\pi x}{l} \cos w_n t$$

- Response spectrum (Fig.2.25) : max. response of SDF system vs. natural period (of frequency) under different damping $\leftarrow (N = f(\omega_n, c))$

