



457.309.02 Hydraulics and Laboratory

.11 Turbulent flow in rough pipe(3)



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Today's objectives

- Can determine the head loss due to the shape change of pipes.
- Apply the knowledge to the more practical problems.



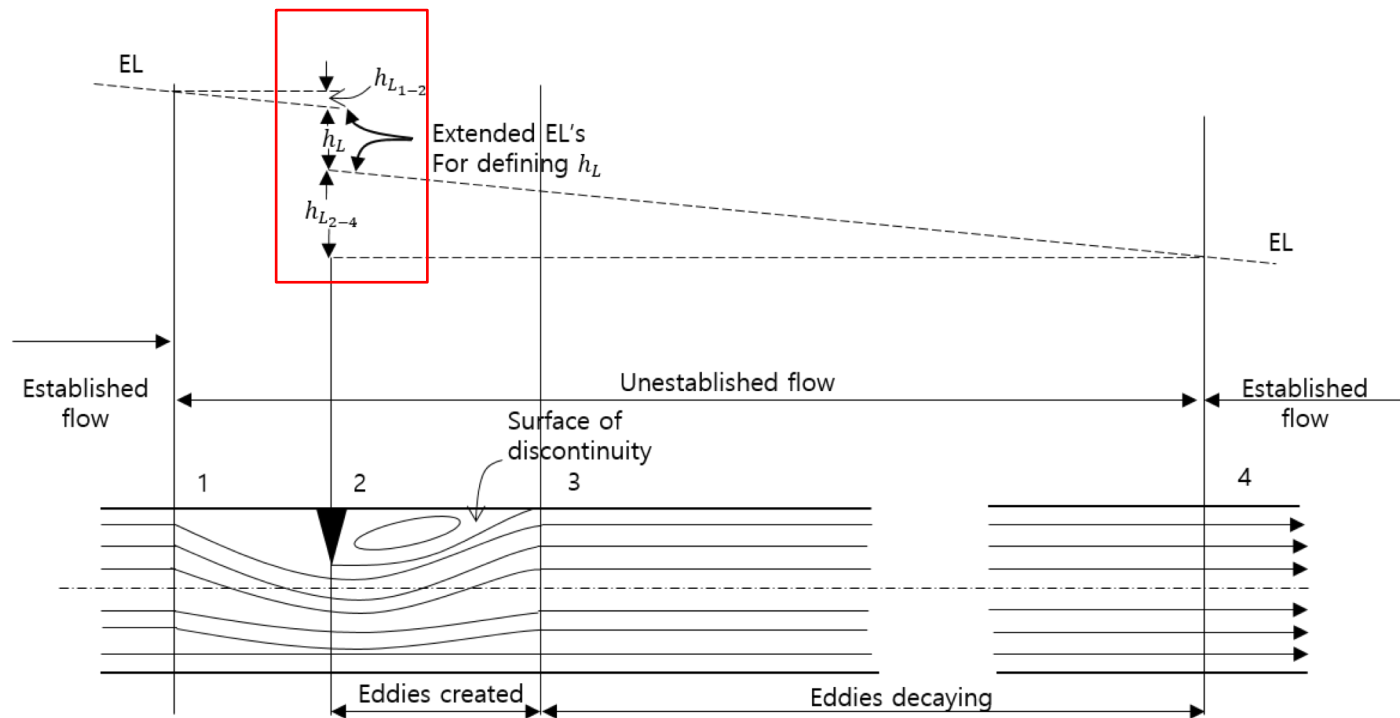
9. Local losses in pipelines

- Bends, elbow, valves, and fittings. Those have the change of cross-section and it causes the head loss.
- In the long pipes, such effects can be neglected but in the short ones, those are significant.
- For example, an abrupt obstruction placed in a pipeline creates dissipation of energy and causes local loss.
- This is generated by the velocity change mainly, and deceleration of velocity causes more energy loss (or head loss) due to the turbulence than acceleration.



9. Local losses in pipelines

- Head loss





9. Local losses in pipelines

- Earlier experiments with water (at high Reynolds number) indicated that local losses vary approximately with the square of velocity and led to the proposal of the basic equation.

$$h_L = K_L \frac{V^2}{2g_n} \quad K_L \text{ is the loss coefficient}$$

- Loss coefficient tends to increase with increasing roughness
- Increases with decreasing Reynolds number
- Constant at the real high Reynolds number
- Mainly determined by the geometry and the shape.

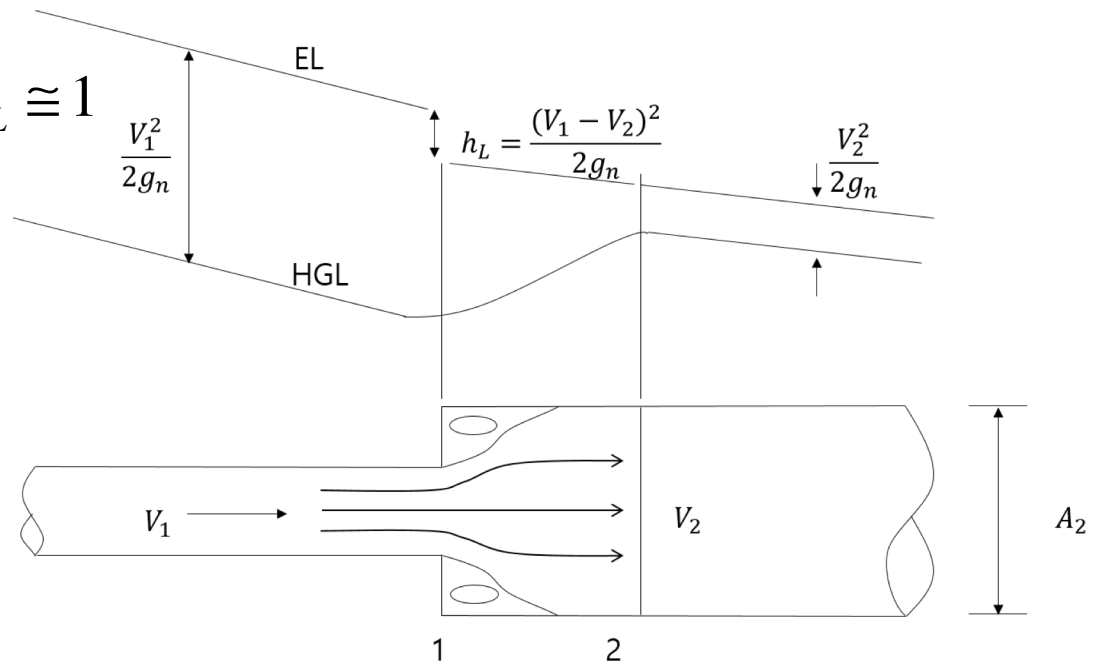


9. Local losses in pipelines

- When an **abrupt enlargement** of section occurs in a pipeline, a rapid deceleration takes place, accompanied by characteristic large-scale turbulence
- It will persist for a distance of 50 diameter or more down stream.

$$h_L = K_L \frac{(V_1 - V_2)^2}{2g_n}$$

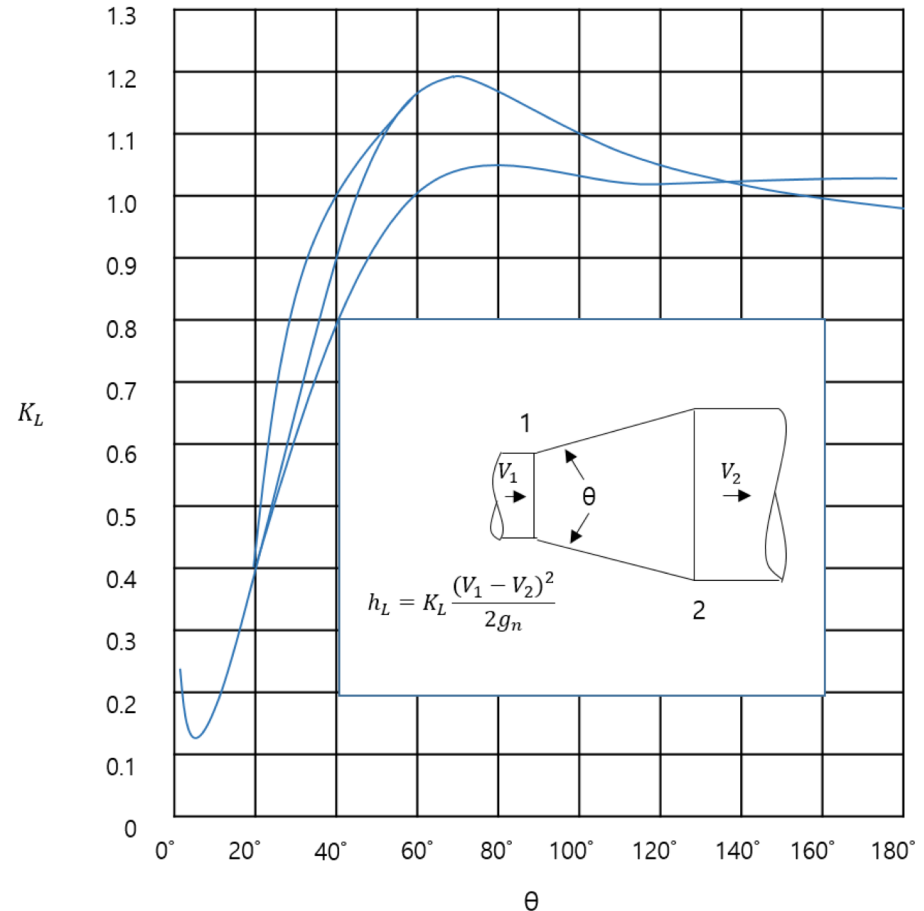
$$K_L \cong 1$$





9. Local losses in pipelines

- The loss of head due to **gradual enlargement** is, of course, dependent on the shape of the enlargement.
- Effects of wall friction and large-scale turbulence
- As angle becomes larger, big separation occurs (minimizing effects of wall friction and separation at 7°)





Example 1.

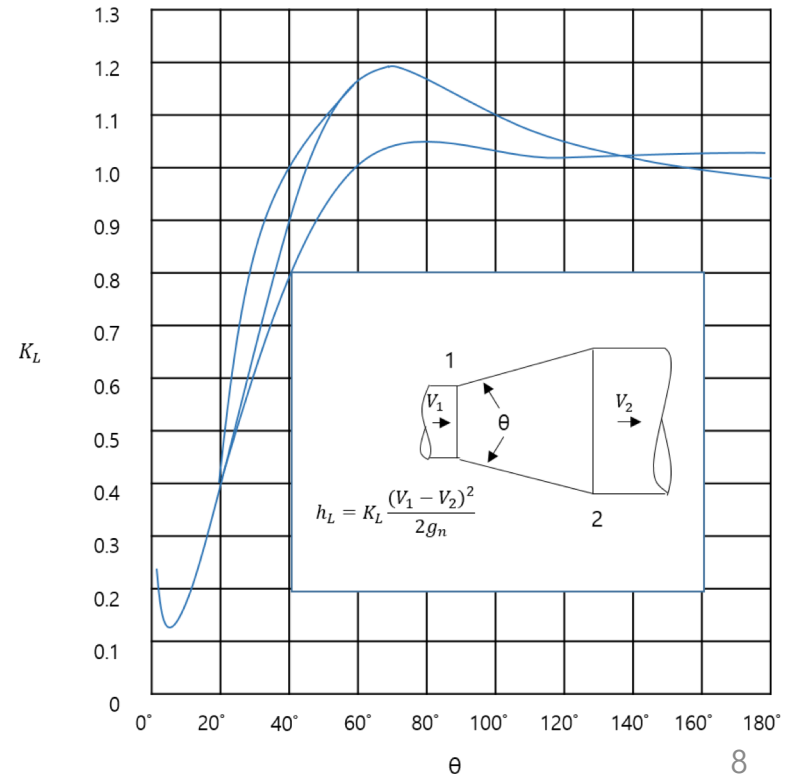
- A 300 mm horizontal water line enlarges to a 600 mm line through 2 0° conical enlargement. When 0.30 m³/s flow through this line, the pressure in the smaller pipe is 140 kPa. Calculate the pressure in the larger pipe, neglecting pipe friction.

- Velocities in each pipe

$$V_{300} = \frac{Q}{A_{300}} = \frac{0.30 \text{ m}^3 / \text{s}}{(\pi / 4)(0.300 \text{ m})^2} = 4.24 \text{ m} / \text{s}$$

$$V_{600} = \frac{Q}{A_{600}} = \frac{0.30 \text{ m}^3 / \text{s}}{(\pi / 4)(0.600 \text{ m})^2} = 1.06 \text{ m} / \text{s}$$

- $K_L = 0.43$ (see the figure)





Example 1

- To compute the pressure in the large pipe, , using Bernoulli's

$$z_{300} + \frac{p_{300}}{\gamma} + \frac{V_{300}^2}{2g_n} = z_{600} + \frac{p_{600}}{\gamma} + \frac{V_{600}^2}{2g_n} + h_L$$

$$h_L = K_L \frac{(V_{300} - V_{600})^2}{2g_n}$$

- Taking the datum as the pipe centerline eliminates z from the calculations leaving

$$\frac{140 \times 10^3 \text{ Pa}}{9,800 \text{ N/m}^3} + \frac{(4.24 \text{ m/s})^2}{2 \times 9.81} = \frac{p_{600}}{\gamma} + \frac{(1.06 \text{ m/s})^2}{2 \times 9.81} + 0.43 \frac{(4.24 - 1.06)^2}{2 \times 9.81}$$

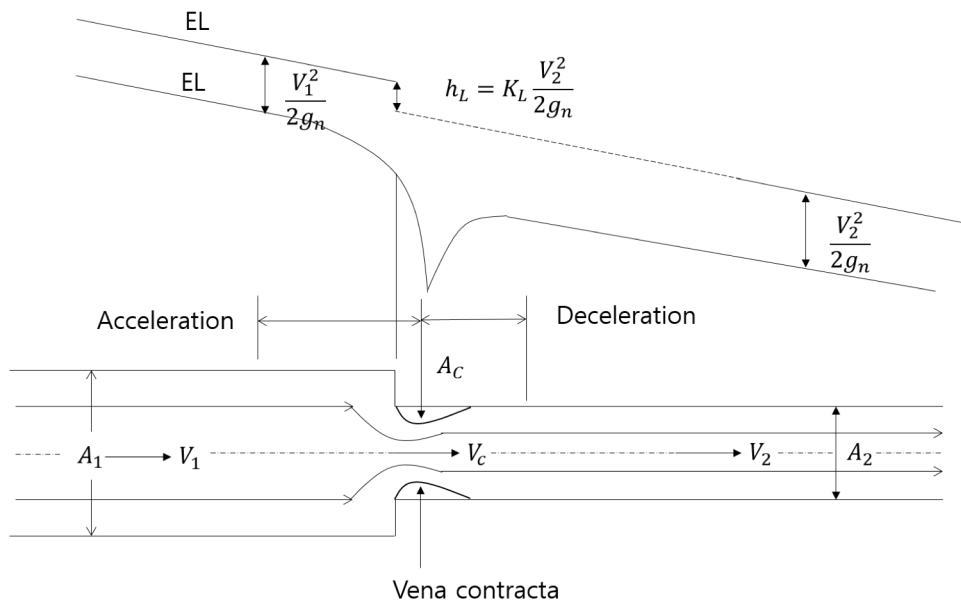
$$\frac{p_{600}}{\gamma} = 14.6 \text{ m}, \quad p_{600} = 14.6 \times 9,800 = 143 \text{ kPa}$$

- Pressure rose at 2 comparing 1. (where we can use it?)



Local Losses in pipelines

- Flow through an abrupt contraction (figure) and is featured by the formation of a vena contracta and subsequent deceleration and reexpansion.

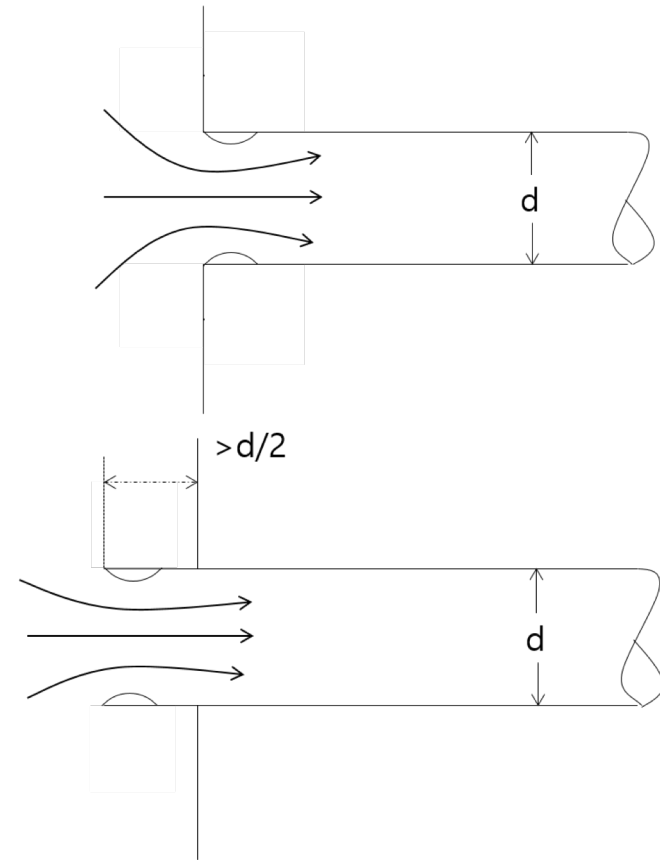
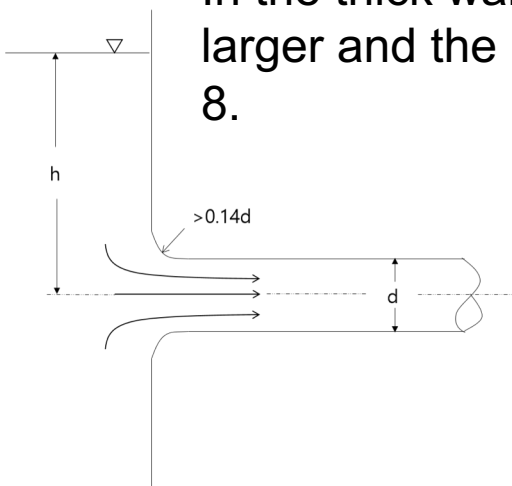


A_2/A_1	0	0.2	0.4	0.6	0.8	1.0
$C_c = A_c/A_2$	0.617	0.632	0.659	0.712	0.813	1.00
K_L	0.50	0.41	0.30	0.18	0.06	0



Local Losses in pipelines

- Square edge
 - Limiting case of the abrupt contraction ($A_2/A_1=0$, since $A_1=\infty$)
 - K_L is 0.5 in turbulent flow
- Re-entrant
 - In the thin wall of pipe, due to the vena contracta large deceleration loss.
 - In the thick wall, vena contracta becomes larger and the loss coefficient less than 0.8.

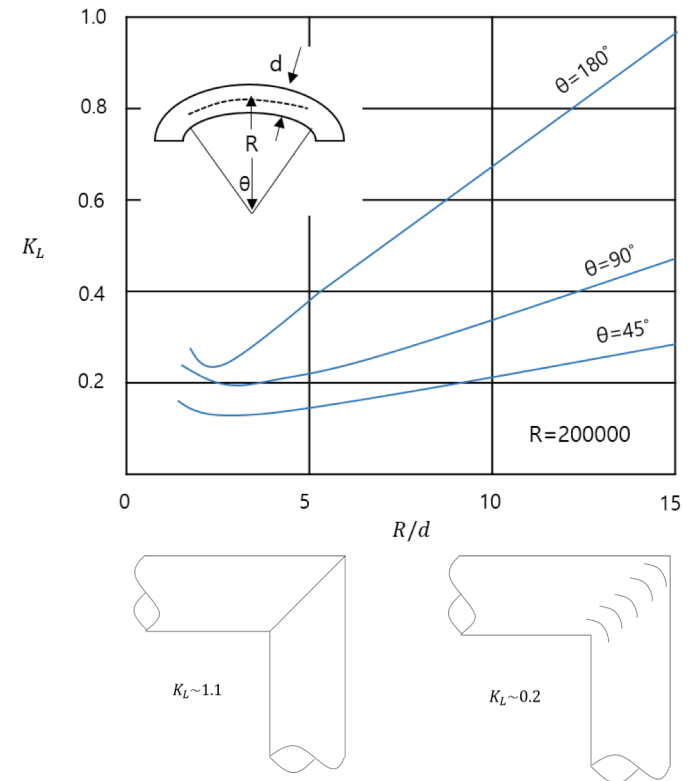


- Bell mouth entrance can prevent vena contracta ($R > 0.14d$)
- Gradual contraction can reduce loss to 0.02~0.04



Local Losses in pipelines

- Losses of head in smooth pipe bends are caused by the combined effects of separation, wall friction and the twin-eddy secondary flow.
- For bends of large radius of curvature, the last two effects will predominate,
- Whereas, for small radius of curvature, separation and the secondary flow will be the more significant.
- K_L minimum at a certain value of R/d .
- As a conclusion, if we can break up the large scale turbulence after separation, then we can reduce the head loss



Miter fit needs to special design for breaking stream line