#### 445.204

## Introduction to Mechanics of Materials (재료역학개론)

# Chapter 7: Generalized Hooke's law (Ch. 6 in Shames)

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- Hooke's law in 3D
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- Plane stress, plane strain

The stress  $\sigma_x$  in the x-direction produces 3 strains 1) Longitudinal strain (extension) along the **x-axis**,

$$\varepsilon_x = \frac{\sigma_x}{E}$$

2) Transverse strains (contraction) along the **y** and **z** -axes, due to the Poisson's effect,

$$\varepsilon_y = \varepsilon_z = -v\varepsilon_x = -\frac{v\sigma_x}{E}$$

The total strain produced along a particular direction can be determined by the principle of superposition

That is, the resultant strain along the x-axis, comes from the strain contribution due to the application of  $\sigma_x$ ,  $\sigma_y$  and  $\sigma_z$ 

 $\sigma_x$  causes:  $\frac{\sigma_x}{E}$  in the x-direction

$$\sigma_y$$
 causes:  $-\frac{\nu\sigma_y}{E}$  in the x-direction

$$\sigma_z$$
 causes:  $v\sigma_z$  in the x-direction

 $\boldsymbol{E}$ 

Therefore, applying the principle of superposition in the xaxis results in

$$\varepsilon_x = \frac{1}{E} \left[ \sigma_x - v(\sigma_y + \sigma_z) \right]$$

In general, by superposition of the components of strain in the x, y, and z directions, the strains can be written as

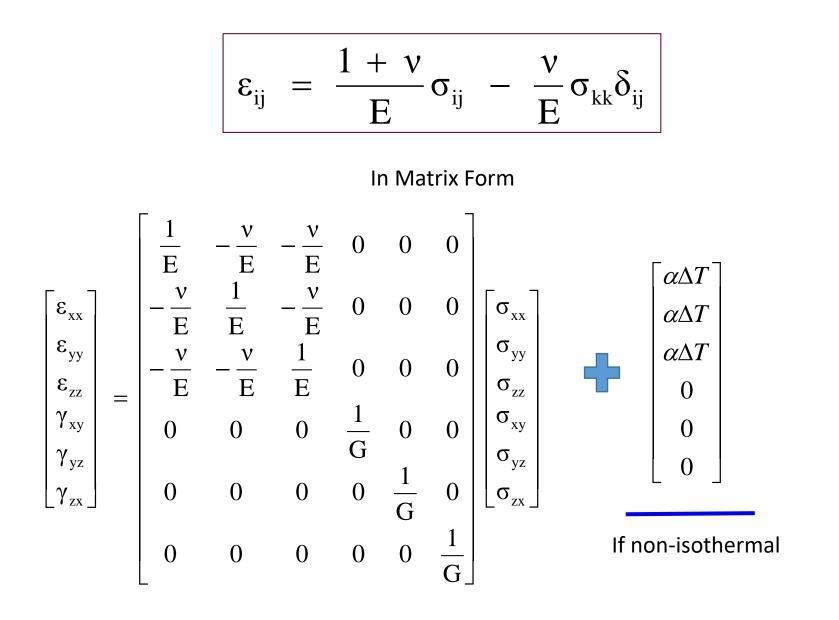
$$\varepsilon_{x} = \frac{1}{E} \left[ \sigma_{x} - v(\sigma_{y} + \sigma_{z}) \right]$$
$$\varepsilon_{y} = \frac{1}{E} \left[ \sigma_{y} - v(\sigma_{z} + \sigma_{x}) \right]$$
$$\varepsilon_{z} = \frac{1}{E} \left[ \sigma_{z} - v(\sigma_{x} + \sigma_{y}) \right]$$

Shearing stresses acting on the unit cube produce shearing Strains; i.e., no Poisson's effect

$$\tau_{xy} = G\gamma_{xy}$$
$$\tau_{yz} = G\gamma_{yz}$$
$$\tau_{xz} = G\gamma_{xz}$$

$$\gamma_{ij} = 2\epsilon_{ij}$$

Why?



• Stress tensor (state) in x-y coordinate system

$$\begin{bmatrix} \tau_{xx} & \tau_{xy} \\ \tau_{xy} & \tau_{yy} \end{bmatrix} = \begin{bmatrix} -S & 0 \\ 0 & S \end{bmatrix}$$

#### Force equilibrium in the x' direction

$$\tau_{y'x'}(2a)(t)\sqrt{2} - 2\left[S(2a)(t)\frac{1}{\sqrt{2}}\right] = 0 \rightarrow \tau_{y'x'} = S$$

• Stress tensor (state) in x'-y' coordinate system

$$\begin{bmatrix} \tau_{x'x'} & \tau_{x'y'} \\ \tau_{x'y'} & \tau_{y'y'} \end{bmatrix} = \begin{bmatrix} 0 & S \\ S & 0 \end{bmatrix}$$

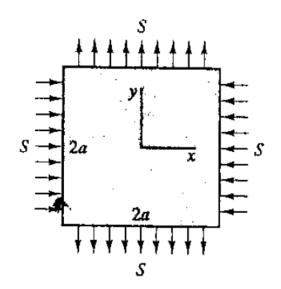


Figure 6.3. Square plate under stress.

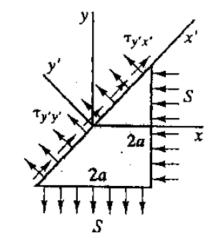
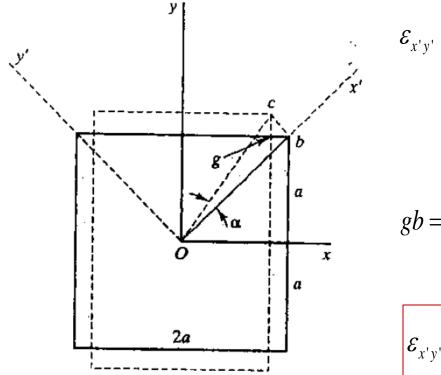


Figure 6.4. Half of square plate.

#### E vs. G vs. v



$$cb = \frac{gb}{\cos 45^{\circ}} = \sqrt{2}gb$$
$$Ob = \frac{a}{\cos 45^{\circ}} = \sqrt{2}a$$
$$= \frac{1}{2}\varepsilon_{xx}(2a) + \frac{1}{2}v\varepsilon_{yy}(2a) = \frac{S}{E}a + v\frac{S}{E}a = \frac{Sa}{E}(1+v)$$

$$\varepsilon_{x'y'} = \frac{S}{E}(1+v)$$

Figure 6.5. Deformation of square plate.

$$\tau_{x'y'} = G(2\varepsilon_{x'y'}) \leftrightarrow S = G(2)(\frac{S}{E}(1+v))$$

$$G = \frac{E}{2(1+v)}$$

## Strain energy

• Strain energy increment in the uniaxial tension for infinitesimal volume dv  $\tau_{xx} d\varepsilon_{xx} dv$ 

 $\tau_{xy}d\gamma_{xy} = \tau_{xy}\left(2d\varepsilon_{xy}\right) = \tau_{xy}d\varepsilon_{xy} + \tau_{yx}d\varepsilon_{yx}$ 

• More generally for normal stresses

$$\left(\tau_{xx}d\varepsilon_{xx}+\tau_{yy}d\varepsilon_{yy}+\tau_{zz}d\varepsilon_{zz}\right)dv$$

• More generally for normal and shear stresses

$$\left(\tau_{xx}d\varepsilon_{xx} + \tau_{yy}d\varepsilon_{yy} + \tau_{zz}d\varepsilon_{zz} + \tau_{xy}d\gamma_{xy} + \tau_{yz}d\gamma_{yz} + \tau_{zx}d\gamma_{zx}\right)dv$$

• Energy per unit volume

$$du = \tau_{xx} d\varepsilon_{xx} + \tau_{yy} d\varepsilon_{yy} + \tau_{zz} d\varepsilon_{zz} + \tau_{xy} d\gamma_{xy} + \tau_{yz} d\gamma_{yz} + \tau_{zx} d\gamma_{zx} = \tau_{ij} d\varepsilon_{ij}$$

• Then,

$$u = \int_0^{\varepsilon_{ij}} \tau_{ij} d\varepsilon_{ij} \qquad \qquad U = \int_V \int_0^{\varepsilon_{ij}} \tau_{ij} d\varepsilon_{ij} dv$$

## Strain energy for linear isotropic elastic solids

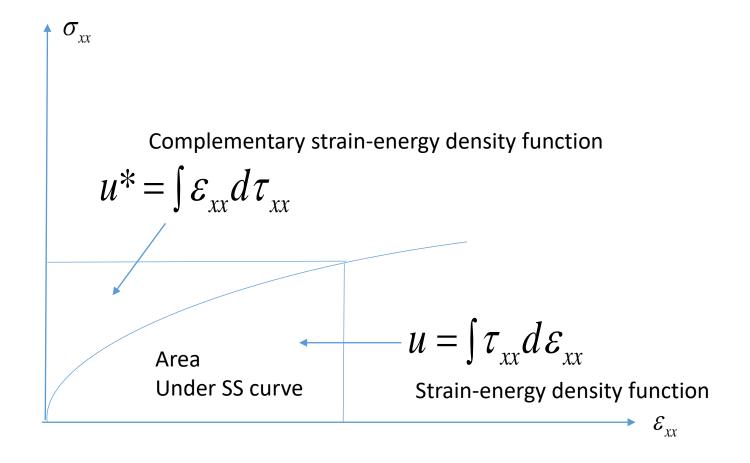
$$du = \frac{\tau_{xx}}{E} \Big[ d\tau_{xx} - v(\tau_{yy} + \tau_{zz}) \Big] + \frac{\tau_{yy}}{E} \Big[ d\tau_{yy} - v(\tau_{zz} + \tau_{xx}) \Big] + \frac{\tau_{zz}}{E} \Big[ d\tau_{zz} - v(\tau_{xx} + \tau_{yy}) \Big]$$
  
+  $\frac{\tau_{xy}}{G} d\tau_{xy} + \frac{\tau_{yz}}{G} d\tau_{yz} + \frac{\tau_{zx}}{G} d\tau_{xz}$ 

$$u = \frac{1}{2E} (\tau_{xx}^2 + \tau_{yy}^2 + \tau_{zz}^2) - \frac{v}{E} (\tau_{xx} \tau_{yy} + \tau_{yy} \tau_{zz} + \tau_{zz} \tau_{xx}) + \frac{1}{2G} (\tau_{xy}^2 + \tau_{yz}^2 + \tau_{zx}^2)$$

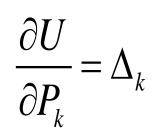
• As functions strain

$$u = \frac{Ev}{2(1+v)(1-2v)} (\varepsilon_{xx} + \varepsilon_{yy} + \varepsilon_{zz}) + G(\varepsilon_{xx}^2 + \varepsilon_{yy}^2 + \varepsilon_{zz}^2) + \frac{G}{2}(\gamma_{xy}^2 + \gamma_{yz}^2 + \gamma_{zx}^2)$$

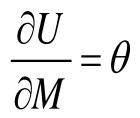
# Strain energy density: physical interpretation



### Energy method – Castigliano theorem II \* Derivation in the textbook



The rate of change of the strain energy for a body with respect to any statically independent force P<sub>k</sub> gives the deflection component of the point of application of this force in the direction of force



The rate of change of the strain energy for a body with respect to any statically independent couple M gives the amount of rotation at the point of application of the point couple about an axis collinear with the couple moment

For a rectangular parallelepiped with edges *dx*, *dy* and *dz* The volume in the strained condition is:

 $(1 + \varepsilon_x)(1 + \varepsilon_y)(1 + \varepsilon_z) dx dy dz$ 

The dilation (or volume strain)  $\Delta$  is given as

$$\Delta = \frac{(1 + \varepsilon_x)(1 + \varepsilon_x)(1 + \varepsilon_x)dx \, dy \, dz - dx \, dy \, dz}{dx \, dy \, dz}$$
$$= (1 + \varepsilon_x)(1 + \varepsilon_x)(1 + \varepsilon_x) - 1$$
for small strain
$$\Delta = \varepsilon_x + \varepsilon_y + \varepsilon_z$$

#### **Plane Stress** ( $\sigma_3 = 0$ )

This stress state is typically observed in

- a thin sheet loaded in the plane of the sheet, or

- a thin wall tube loaded by internal pressure where there is no stress normal to a free surface.

Set 
$$\sigma_{z} = \sigma_{3} = 0$$
.  
Therefore,  
 $\varepsilon_{1} = \frac{1}{E} [\sigma_{1} - v\sigma_{2}]$ 
 $\varepsilon_{1} = \frac{1}{E} [\sigma_{1} - v(\varepsilon_{2}E + v\sigma_{1})]$ 
 $\varepsilon_{2} = \frac{1}{E} [\sigma_{2} - v\sigma_{3}]$ 
 $= \frac{1}{E} [\sigma_{1}(1 - v^{2})] - \frac{1}{E} (v\varepsilon_{2}E)$ 
 $\varepsilon_{3} = -\frac{1}{E} v[\sigma_{1} + \sigma_{2}]$ 
 $\varepsilon_{1} + v\varepsilon_{2} = \frac{1 - v^{2}}{E} \sigma_{1}$ 
 $\sigma_{1} = \frac{E}{1 - v^{2}} [\varepsilon_{1} + v\varepsilon_{2}]$ 
Similarly,  
 $\sigma_{2} = \frac{E}{1 - v^{2}} [\varepsilon_{2} + v\varepsilon_{1}]$ 

## **Plane strain**

**Plane Strain** ( $\varepsilon_3 = 0$ ): This occurs typically when One dimension is much greater than the other two

Examples are a long rod or a cylinder with restrained ends.

$$\varepsilon_3 = \frac{1}{E} [\sigma_3 - \nu(\sigma_1 + \sigma_2)] = 0$$

but

$$\sigma_3 = \nu[\sigma_1 + \sigma_2]$$

This shows that a stress exists along direction-3 (z-axis) even though the strain is zero.

$$\varepsilon_{1} = \frac{1}{E} \left[ (1 - v^{2})\sigma_{1} - v(1 + v)\sigma_{2} \right]$$
  

$$\varepsilon_{2} = \frac{1}{E} \left[ (1 - v^{2})\sigma_{2} - v(1 + v)\sigma_{1} \right]$$
  

$$\varepsilon_{3} = 0$$

## **Questions ?**