

Theory of Beam, Plate and Shell:

Physical point of view of a structure:

- Geometrical point

- ~ Flat or Curved shape

- ~ Dimension ?

- ~ Thickness ?

- Material point

- ~ Elasticity : Isotropic or Composite

- ~ Functionally Graded Material

- ~ Plasticity, Viscoelasticity,

- Hygrothermal...

What is Modelling ?

- ~ Closely related in geometric shape !

v What is mechanics ? Concern on the causes and effects

Newton's law : $F \propto a \Rightarrow ma$

Cause ~ Force : Effect ~ Deformation(Solid mechanics)

or

Motion(Dynamics)

v Special simplification from 3-D body :

How can we logically follow 1D or 2D approximation ?

i) Stress field ii) Strain field iii) Deformation field

Signification and limitation of their use can best be understood

in terms of the general theory !

->

Final goal:

**Systematic and concise derivation of engineering theory of
plate and shell.**

: Linear, Nonlinear theory...

1. Preliminary mathematics

1-1 Indicical Notation

Compact notation --- Meaning ?

: Generalization of formulation

Ex : Circular, rectangular, elliptic plate models

Geometric or boundary shapes are different,

but the same assumptions are used.

: Mathematical expressions are different,

but the meanings are same !

~ Invariant !

Why we choose a special coordinate system?

Just for convenience !

~ FEM? Analytical approach?

Right-hand coordinate system

Vector : $x_i (i = 1, 2, 3)$
or
 $x_\alpha (\alpha = 1, 2), x_3$

Summation Convention (Dummy or repeated index)

$$: \vec{V} = V_x \vec{i} + V_y \vec{j} + V_z \vec{k} = V_\ell \vec{i}_\ell$$

$$\int_a^b f(x) dx \equiv \int_a^b f(y) dy$$

Orthogonality of unit vector : $(\vec{i}, \vec{j}, \vec{k})$

$$\vec{i} \cdot \vec{j} = 0, \vec{i} \cdot \vec{i} = 1 \dots$$

Kronecka Delta : $\delta_{ij} \rightarrow \begin{cases} i \neq j : 0 \\ i = j : 1 \end{cases}$

$$\vdots \begin{bmatrix} 1, 0, 0 \\ 0, 1, 0 \\ 0, 0, 1 \end{bmatrix}$$

Ex: $\delta_{ii} = \delta_{11} + \delta_{22} + \delta_{33} = 3!$

*** Einstein : Summation Convention**

$$a_i + b_{ij}c_j + d_{kl}q_{kli} = 0..(i, j, k, l = 1, \dots, N)$$

Stands for the N separate equations :

~ free index i .

o Live script must occur only once in each term of eqn.

o Dummy script occurs twice in a term and is to be summed over the range

If N=3 : $b_i \Rightarrow b_1, b_2, b_3$

~ 3 Components of a vector

$$A_{ij} \Rightarrow \begin{bmatrix} A_{11}, A_{12}, A_{13} \\ A_{21}, A_{22}, A_{23} \\ A_{31}, A_{32}, A_{33} \end{bmatrix}$$

~ $3 \times 3 = 9$ Components of 2nd order Tensor

*** Contracted product :**

$b_i A_{ij}$: Summations on index i : Free index j

- *Simply contracted product with 2 live script*

$$C_{ik} = A_{ij} B_{jk}$$

- *Doubly contracted product without live script : 1 component : Scalar !*

$$S = A_{ij} B_{ji}$$

Ex : $\mathbf{b} = (b_1, b_2, b_3)$

3 components of a vector : b_i

Scalar : $b_i b_i$

Tensor product or open product of 2 vectors : $b_i b_j$

Kronecker delta and alternating symbol ε

$$\delta_{ij} = \begin{cases} 1 : i = j \\ 0 : i \neq j \end{cases} \Rightarrow \begin{bmatrix} 1, 0, 0 \\ 0, 1, 0 \\ 0, 0, 0 \end{bmatrix}$$

$$\delta_{ii} = ?$$

$$A_i \delta_{ij} = A_j$$

$$\mathcal{E}_{ijk} = \begin{cases} 1 : \text{Even transposition of } 1, 2, 3 \\ -1 : \text{Odd transposition of } 1, 2, 3 \\ 0 : \text{At least 2 scripts are the same!} \end{cases}$$

$$\mathcal{E}_{123} = \mathcal{E}_{231} = \mathcal{E}_{321} = 1$$

$$\mathcal{E}_{321} = \mathcal{E}_{132} = \mathcal{E}_{213} = 1, \quad \text{All other terms} = 0$$

Symmetry tensor and Anti-symmetry tensor

$$\alpha_{ij} = \alpha_{ji}, A_{lmn} = A_{lmn} \quad \text{or} \quad \alpha_{ij} = -\alpha_{ji}, A_{lmn} = -A_{lmn}$$

Index notations

$$\frac{\partial \phi}{\partial x_i} = \phi_{,i}$$

$$\nabla \cdot \vec{V} = \frac{\partial V_x}{\partial x} + \frac{\partial V_y}{\partial y} + \frac{\partial V_z}{\partial z} = \frac{\partial V_i}{\partial x_i} = V_{i,i}$$

$$\nabla^2 \phi = \phi_{,ii}$$

$$\sigma_{\alpha\beta} = \frac{E}{1+\nu} (\epsilon_{\alpha\beta} + \frac{\nu}{1-\nu} \delta_{\alpha\beta} \epsilon_{\zeta\zeta}) : \text{plane.stress.problem!}$$

$$\vec{C} = \vec{A} \times \vec{B} \rightarrow C_i = \epsilon_{ijk} A_j B_k$$

Practice !!

1-2 Calculus of Variation

$$U = \frac{1}{2} \int \sigma_i \varepsilon_i dV$$

There are at least **3 important reasons** for taking up the ‘Calculus of Variation’ in the study of continuum mechanics

1. Basic minimum principles exist

(**Minimum Total Potential Energy : M.T.P.E.**)

2. The field eqns & associate B.Cs of many problems can be derived from “**Variational Principles**”.

~ In formulating an **approximate theory**, the shortest and clearest derivation is usually obtained through **Variational Calculus**

3. Direct method of solution of variational problem is one of the most powerful tools for obtaining numerical results in practical problems of engineering importance.

In this section, we shall **summarize** the relation and properties derived from Variational Principles briefly, and then discuss their applications

[A] Euler's Equations

To determine the function $y(x)$, for $x_0 \leq x \leq x_1$ that **minimize the definite integral**

$$V = \int_{x_1}^{x_2} F(x, y(x), y'(x), y''(x)) dx$$

we call it the **functional** that depends on the unknown function $y(x)$

in which $y(x)$: continuous & differentiable with continuous $y'(x), y''(x)$ for $x_0 \leq x \leq x_1$ and satisfies forced function B.C (= admissible function)

We assume that the function F continuous with all its partial derivatives to order 3 for all real values of $y(x), y'(x), y''(x)$ and for all values of x in $x_0 \leq x \leq x_1$

Admissible variation ?

Euler- Lagrange's eqns

$$F_{,y} - (F_{,y'})_{,x} + (F_{,y''})_{,xx} = 0$$

with Boundary Conditions

$$\varepsilon \eta(x) \left[F_{,y'} - F_{,y'} \right]_{x_1}^{x_2} + \varepsilon \eta(x) \left[F_{,y''} \right]_{x_1}^{x_2} = 0$$

\Rightarrow **Necessary condition** for V to have a **minimum value** since $\delta V = 0$

\sim **Sufficient condition** : $\delta^2 V = 0$: **positive definite**

[B] Variational notation

In the derivation of Euler-Lagrange eqn, $y(x)$ is augmented by an **infinitesimal** **ftn** $\varepsilon \eta(x)$ in which $\eta(x)$ is any admissible variation & ε is **arbitrary** constant
in practical application, $\varepsilon \eta(x) \equiv \delta y(x)$

$$\Rightarrow \delta y'(x) \equiv \varepsilon \eta'(x), \delta y''(x) \equiv \varepsilon \eta''(x). \text{etc.}$$

$$\Rightarrow \delta y'(x) \equiv (\delta y(x))', \delta y''(x) \equiv (\delta y(x))''$$

$$: \delta \int_{x_0}^{x_1} y(x) dx = \int_{x_0}^{x_1} \delta y(x) dx : \quad \Leftrightarrow ?$$

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$$\therefore \delta \int_{x_0}^{x_1} y(x) dx = \int_{x_0}^{x_1} \delta y(x) dx : \Leftrightarrow ?$$

Energy($y(x)$) \Leftrightarrow Equation of motion($\delta y(x)$) : ?

;Non-conservative forces:Damping , Follower

force...

Elastic material:

$$\sigma_{ij} = E_{ijkl} \varepsilon_{kl} : (i, j, k, l = 1..3)$$

$$U = \frac{1}{2} \int \sigma_{ij} \varepsilon_{ij} dV :$$

$$\delta U = \frac{1}{2} \int \sigma_{ij} \delta \varepsilon_{ij} dV + \frac{1}{2} \int \delta \sigma_{ij} \varepsilon_{ij} dV (?) :$$

: Displacement analysis ~ Stress analysis

$$\begin{aligned} \delta V &= \int_{x0}^{x1} F(x, y(x) + \delta y(x), y'(x) + \delta y'(x), y''(x) + \delta y''(x)) dx - \int_{x0}^{x1} F(x, y(x), y'(x), y''(x)) dx \\ &\equiv \int_{x0}^{x1} \delta F(...) dx \end{aligned}$$

Expand around (x, y, y', y'') !

$$= \int_{x_0}^{x_1} (F(x, y(x), y'(x), y''(x)),_y \delta y + F(...),_{y'} \delta y' + F(...),_{y''} \delta y'') dx$$

Integration by part,

$$\int_{x_0}^{x_1} (...) \delta y dx + [(...) \delta y]_{x_0}^{x_1} + [(...) \delta y']_{x_0}^{x_1}$$

Last two terms denote the Boundary Conditions for Forces and Displacements !

~ Dimensions ?