

## 1.7 Direct Shear Test

- A test to determine the shearing characteristics of soils. (Based on stress states on failure plane)
- The shear strength parameters,  $c'$  and  $\phi'$  (based on Mohr-Coulomb failure criteria ( $s = c' + (\sigma - \Delta u)\tan\phi'$ )) can be easily determined.

### 1) Test conditions and procedures

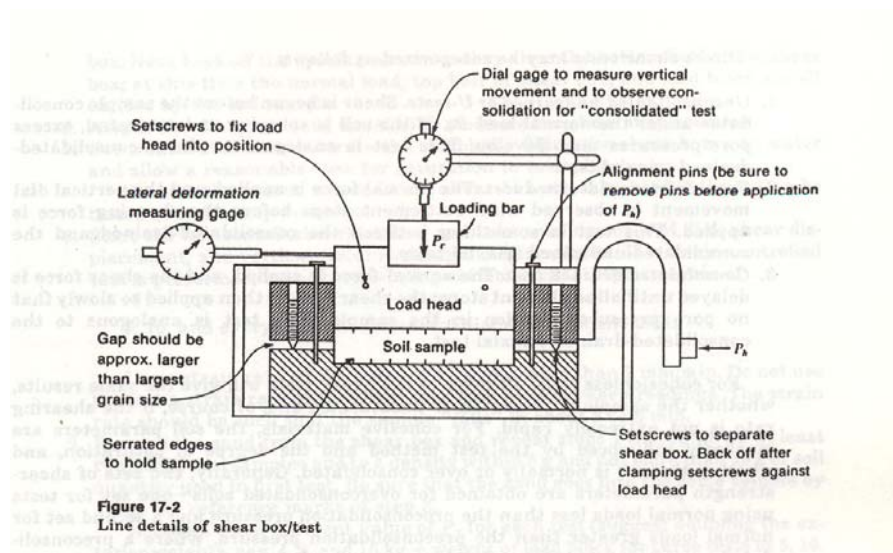


Fig. 17-2

- Apply the desired vertical load (normal force)  $P_v$  and measure the vertical displacements (with time).
- Start horizontal loading at a constant displacement rate and measure horizontal load (shearing force)  $P_h$ , horizontal displacement (shear displacement), and vertical (volume change) displacement.
- Repeat i) – ii) for additional samples of identical condition at different values of  $P_v$ . (At least, 3 tests are required to compute  $c'$  and  $\phi'$ )

2) Categorization of shear test based on drained condition.

- i) Unconsolidated – Undrained test (UU test)  $\Rightarrow$  Undrained strength for saturated cohesive soils.  $\Rightarrow$  Shearing process is begun as soon as the sample is put in the machine.
  - ii) Consolidated – Undrained test (CU test)  $\Rightarrow$  Undrained strength for saturated cohesive soils  $\Rightarrow$  Soil sample is sheared at high strain rate after completion of consolidation under applied vertical load.
  - iii) Consolidated – Drained test (CD test)  $\Rightarrow$  Effective (Drained) Strength parameters,  $c'$  and  $\phi'$   $\Rightarrow$  The shear force is applied so slowly that the small pore pressure cannot be developed after completion of consolidation.
- For cohesionless soils, it is reasonable to assume “drained” condition, due to short drainage path of sample (thickness of sample = 20 to 25 mm) and its high permeability.
  - For cohesive soils, drained condition can be simulated with low strain rate while undrained condition can be simulated with high strain rate.

- Undrained and Drained Strength

For cohesionless soils  $\Rightarrow$  drained strength

For cohesive soils  $\Rightarrow$  drained strength and undrained strength

3) Typical test results

$$\sigma_n = P_v/A \text{ and } s = P_h/A$$

where A is the nominal area of the sample (or of the shear box).

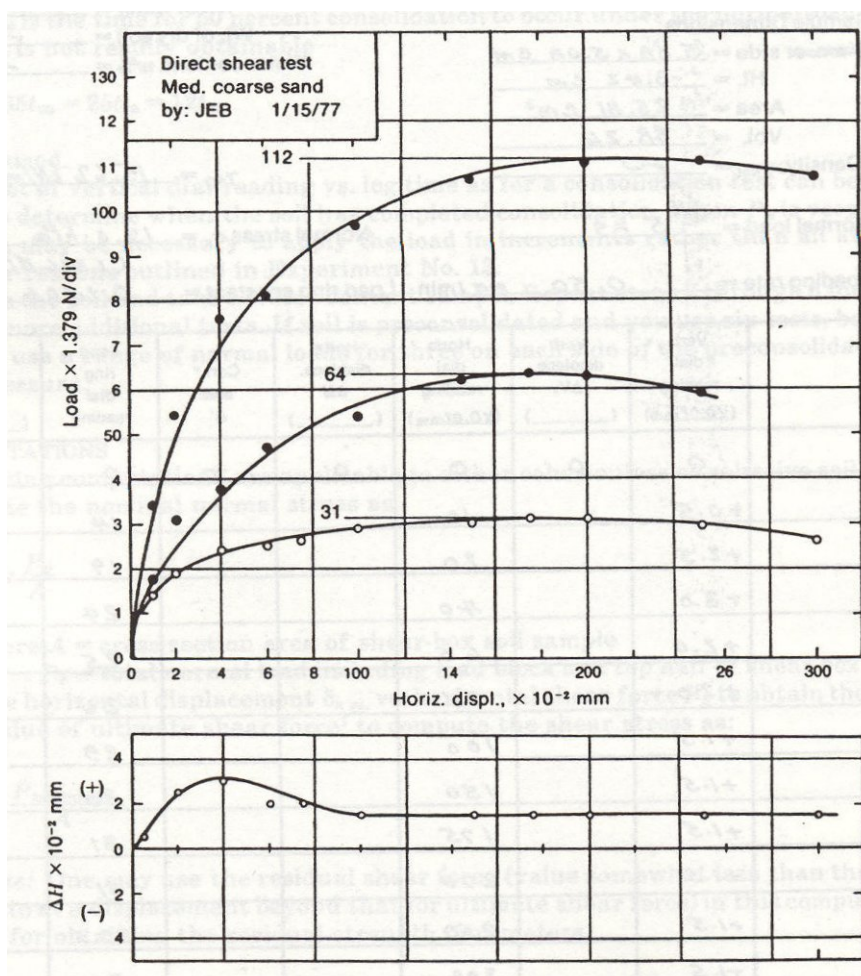


Figure 17-4

- Graphical determination of shear strength parameters

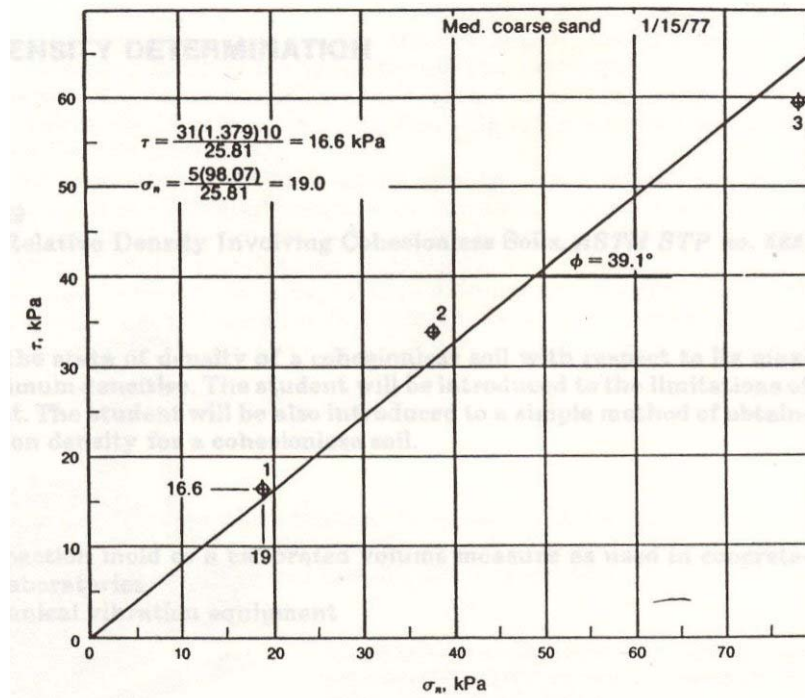


Figure 17-5

- A small “apparent cohesion” (less than 10 – 15 kPa) should be neglected.

#### 4) Advantages

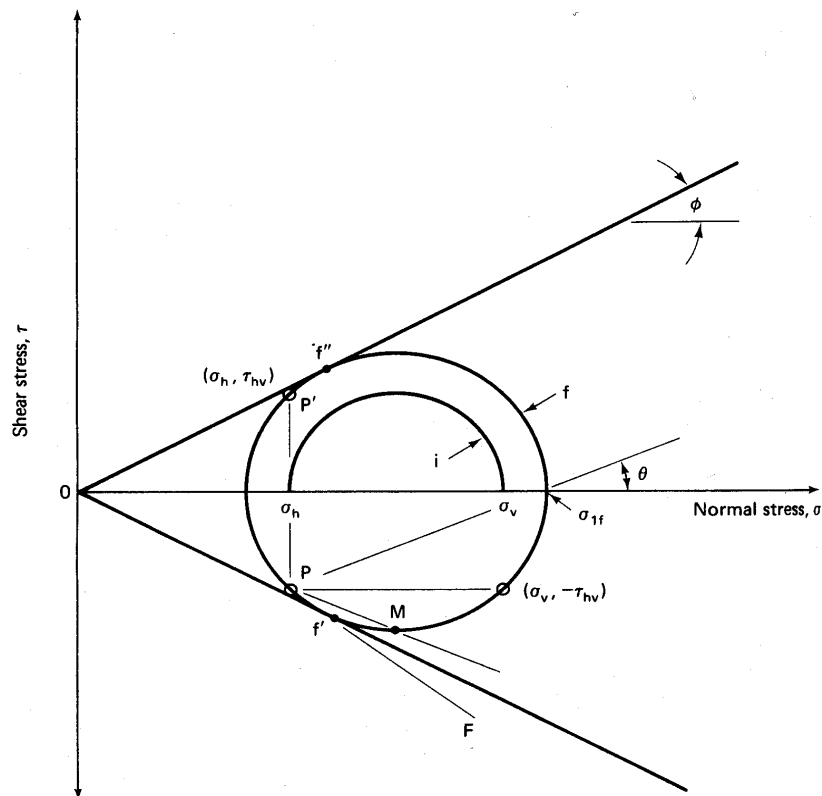
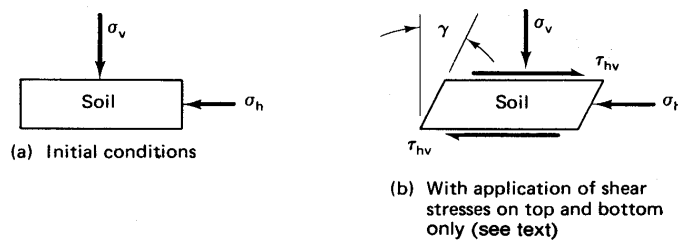
- It is easy and simple to perform the tests and interpret test results.
- Drainage path is short. The time for consolidation is relatively short.
- The direct-shear machine is quite adaptable to electronic readout equipment.

#### 5) Disadvantages and limitations

- Cannot control drainage and thus cannot measure volume change or pore pressure.
- The area of sample changes as the test progresses.
- The failure surface is predetermined as a plane.

- iv) The shearing stress is not uniformly distributed over the failure plane.
- v) The test uses a small sample with the result so that preparation errors can become relatively important.
- vi) Values of deformation modulus ( $E_s$  or  $G_s$ ) cannot be determined.

6) Direct Simple Shear Test



- i) The Direct simple shear uses a closed shear box fixed at the base with the top free to translate under a horizontal force. The shear box may
  - a) Use hinged sides.
  - b) Use a wire-reinforced rubber membrane for the sides.
- ii) Direct simple shear test simulates the pure shear condition.
- iii) Drainage condition can be controlled (partly).
- iv) Cyclic stresses can be applied.  $\Rightarrow$  Particular application in liquefaction studies.

Advantages

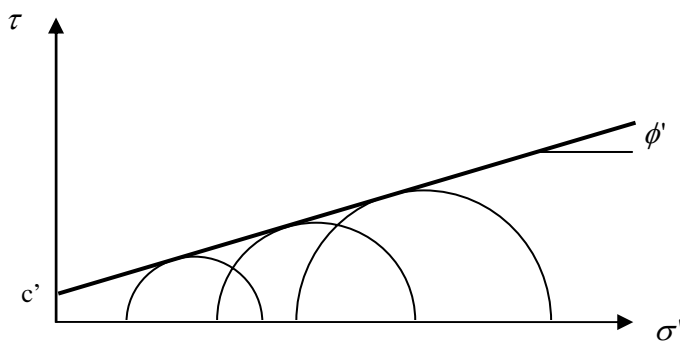
1. Use in special design practice.
  - $s_u$  of soft clays for use in slope stability calculations.
2.  $K_0$  consolidation.
3. Can be used for cyclic behaviors of soils.

$$\phi'_{DSS} \approx \phi'_{TX} \quad (\text{sands})$$

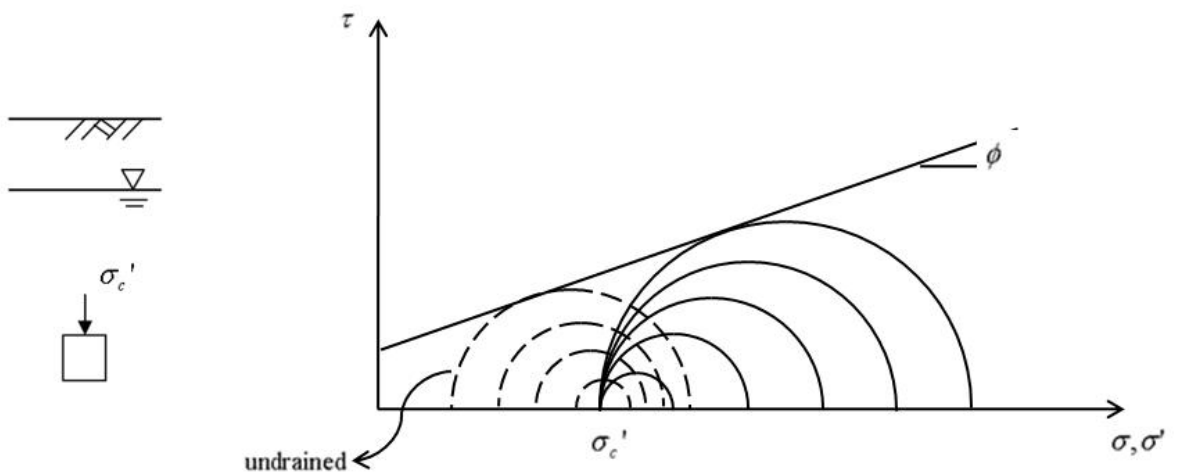
$$s_{u(TXE)} \leq s_{u(DSS)} \leq s_{u(TXC)} \quad \text{for NC to lightly OC clays}$$

**\* Undrained Strength vs. Drained Strength**

- drained strength : no  $u_e$  during loading. (volume change)
  - ◀ sands and clays for long-term stability
- undrained strength : no volume change during loading ( $\rightarrow u_e$ )
  - ◀ clays at the end of construction
- Whatever drainage condition is, failure is governed by effective stress.

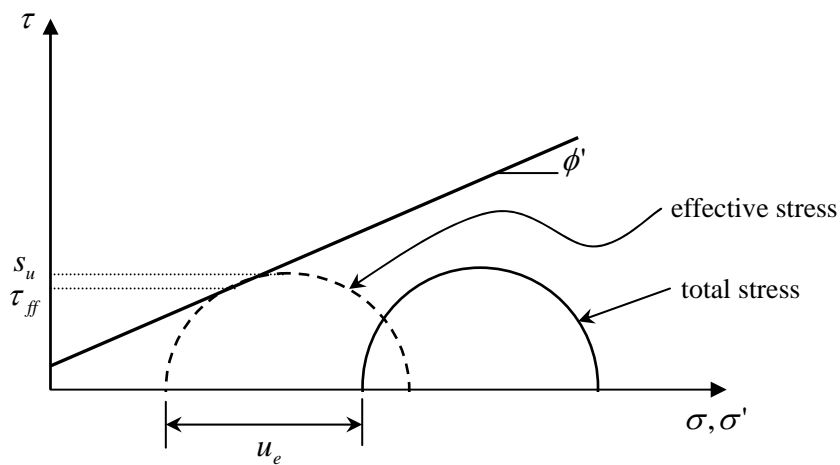


For TX condition,



drained strength parameter :  $c', \phi'$

undrained strength parameter :  $c_u$  or  $s_u$



for  $\tau_{ff}$   $\Rightarrow$  we must know  $\phi'$  (and  $c'$ ) and we need to find out  $u_e$ .

But  $u_e$  cannot be determined from simple test.

(unconfined test or unconsolidated-undrained test).



**use**  $s_u$