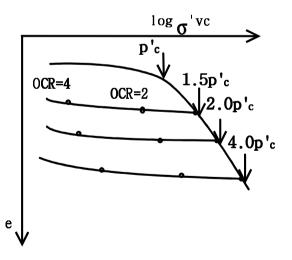
## (3) <u>SHANSEP</u> (Charles C. Ladd (MIT))

Stress History And Normalized Soil Engineering Properties.

 $\Rightarrow$  Undrained shearing behaviors of clays can be represented by normalized strength concept. ( $s_u / \sigma_{vc} = f(\text{soil type & OCR...})$ 

① Sample disturbance + consolidation stress state + stress history

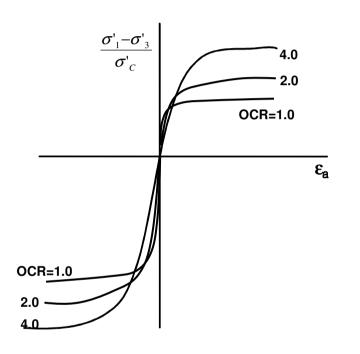
- NC soils  $\rightarrow$  consolidate samples to 1.5 , 2.0 and 4.0 times larger than p'<sub>c</sub> (maximum past pressure) with <u>K<sub>0</sub> state</u>.
- OC soils  $\rightarrow$  follow the same procedure as NC soils and then unload to a given value of OCR.



② Strain rate + anisotropy

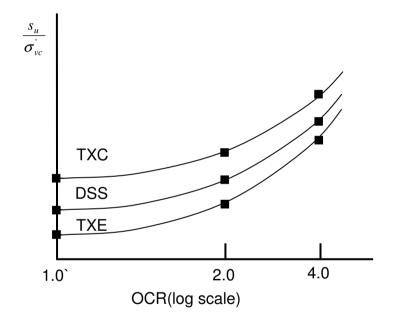
 $\Rightarrow$  shear samples with <u>0.5%/hr strain rate</u> for CK<sub>0</sub>U <u>TXC</u> and <u>RTXE</u>, <u>DSS</u> conditions.

③ We get normalized results from step ②.



- + Normalized Pore
  - Pressure Response.

④ Combine the results.



## Problems with SHANSEP.

- 1. Determination of p'c (max. past pressure).
  - $\rightarrow$  Sample disturbance obscures to obtain p'<sub>c</sub> and lowers it.
  - $\rightarrow$  Secondary compression can have a large influence on measured value of p'<sub>c</sub> for highly compressible clays.
  - $\rightarrow$  Knowledge of geologic history is very important.
- 2. Determination and duplication of stress systems (stress ratio of consolidation  $(\sigma_h^{\cdot}/\sigma_v^{\cdot}) \rightarrow K_0$  value.)
  - $\rightarrow$  Difficult to measure it and apply it for lab testing.

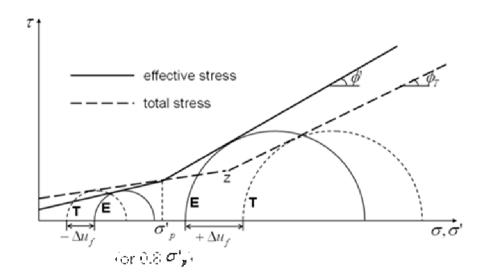
- 3. Difficult to get  $s_u$  for heavily overconsolidated clays. (*with high p'c*)
  - $\rightarrow$  Practically difficult to reach 1.5 to 4.0 p'<sub>c</sub> because of high p'<sub>c</sub> and OCR.
  - $\rightarrow$  Use recompression techniques.
- 4. Not acceptable for sensitive or structured clays.
- ← SHANSEP employs mechanical stress history approach.
- 5. In situ soil has some variations of water content even if we assume it is homogeneous.  $\rightarrow$  For more precise estimation of strength, we have to consider water content variation.

6. A lot of works are required.

Notes.

1. Mohr-Coulomb failure envelopes over a range of stress spanning the preconsolidation

stress,  $\sigma'_p$ .



Typical point  $z \approx 2\sigma'_p$  (Hirschfeld, 1963)

2.  $s_u$  vs.  $\tau_{ff}$  (shear stress on failure plane at failure.)

