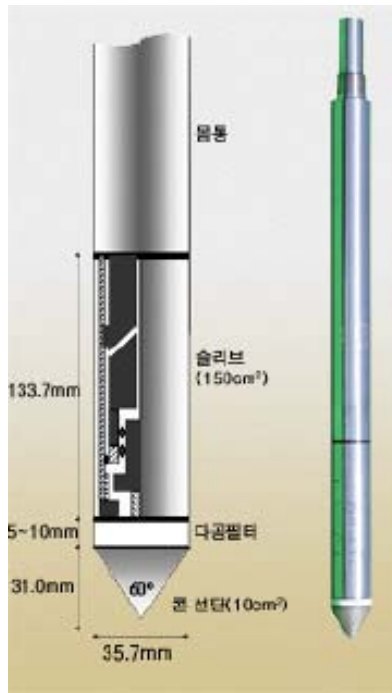


3.3 Cone Penetration Test (CPT)

(1) General

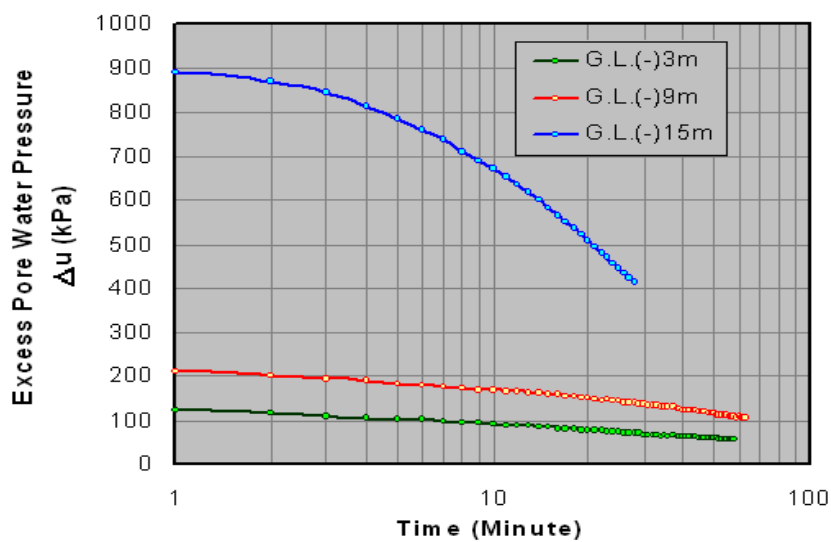


The CPT is carried out by pushing a 60° cone with a face area of 10 cm² into the ground at a constant speed (2 ± 0.5 cm/s), whilst measuring the force to do so. The shear force on a 150 cm² 'friction sleeve' and pore pressure are then also measured.

- Type of cone
 - Piezocone
 - Environmental cone
 - Seismic cone
 - Visual cone

- Dissipation test with piezocone

In clays, the horizontal coefficient of consolidation C_v can be determined by stopping the cone, and measuring pore pressure dissipation as a function of time.



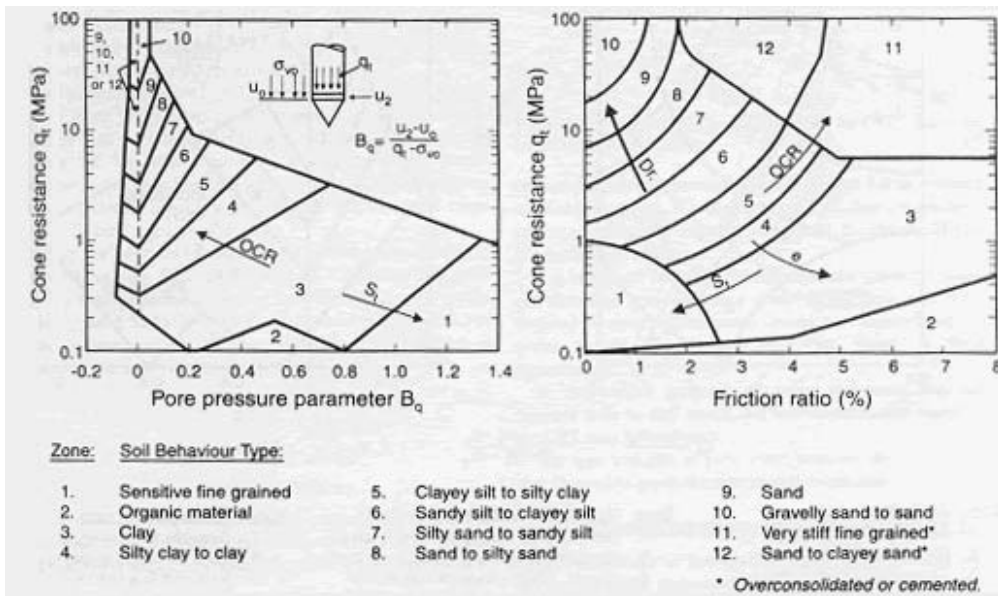
(2) Interpretation and use

i) soil classification

- Robertson and Campanella (1986)

$$B_q = \frac{\Delta u}{q_t - \sigma_{vo}} \quad \text{vs} \quad q_t$$

$$F_r = \frac{f_s}{q_t - \sigma_{vo}} \quad \text{vs} \quad q_t$$



ii) Undrained strength

- S_u is evaluated with cone factors (N_{kt} , N_{ke} , $N_{\Delta u}$)
- Schmertmann(1978), Lunne et al(1985) $s_u = \frac{q_T - \sigma_{v0}}{N_{kt}}$
- Senneset et al (1982), Campanella et al (1982) $s_u = \frac{q_T - u}{N_{ke}}$
- Lunne et al (1985) $s_u = \frac{\Delta u}{N_{\Delta u}}$

| regions | The test to evaluate cone factor | Cone factor |
|-------------------|----------------------------------|-------------------------|
| England | CIUC | $N_{kt} = 12 \sim 20$ |
| Norway | FVT | $N_{kt} = 12 \sim 19$ |
| Italy | FVT | $N_{kt} = 8 \sim 16$ |
| | CKOUC | $N_{kt} = 8 \sim 10$ |
| Vancouver, Canada | FVT | $N_{kt} = 8 \sim 10$ |
| | SBPT | |
| Japan | UCT | $N_{kt} = 8 \sim 16$ |
| | FVT | $N_{kt} = 9 \sim 14$ |
| Taiwan | CIUC | $N_{qu} = 5.0 \sim 6.8$ |
| | CAUC | $N_{qu} = 6.0 \sim 7.2$ |
| Canada | FVT | $N_{Du} = 6.2 \sim 7.0$ |

- Cone factor are very site –specific.

iii) Other usages

- The horizontal coefficient of consolidation(c_h)
- Friction angle(ϕ') and D_r in sandy soil
- OCR, E_s , Sensitivity, ...