

Where do we stand?

25 May 2009

10 March

Equation of Equilibrium and of fluid flow.

$$\tau_{ij} = 2\mu \varepsilon_{ij} + \lambda \varepsilon_{kk} \delta_{ij} + \alpha P_p \delta_{ij}$$

$$= 2\mu \left(\frac{1}{2} (u_{i,j} + u_{j,i}) \right) + \lambda \varepsilon_{kk} \delta_{ij} + \alpha P_p \delta_{ij}$$

$$\tau_{ij,j} + f_i = \mu (u_{i,jj} + u_{j,ij}) + \lambda u_{k,kj} \delta_{ij} + \alpha P_{p,j} \delta_{ij} = 0$$

$$= \mu u_{i,jj} + \mu u_{j,ji} + \lambda u_{k,kj} + \alpha P_{p,i} + f_i = 0$$

$$\mu u_{i,jj} + (\lambda + \mu) u_{j,ji} + (f_i + \alpha P_{p,i}) = 0$$

But coefficient $\left(\alpha \frac{\partial P_p}{\partial x_1}, \alpha \frac{\partial P_p}{\partial x_2}, \alpha \frac{\partial P_p}{\partial x_3} \right)$

looks like body force.

(mathematically) play the role of additional body force.

hw

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if you forget about blind it makes sense.

in terms of excess fluid content, ξ

$$\mu u_{i,jj} + (\lambda + \mu + \alpha^2 M) u_{j,ji} + f_i + \alpha M \xi_{,i} = 0$$

3 eqn & 4 unknowns.

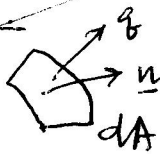
$\frac{BK}{\alpha(1-\alpha B)}$

We need one more eqn.

Start with fluid flow vector, g_i (m/s)



REV



we are not talking about individual parts.

$g \cdot n \cdot dA =$ volumetric flux through dA .

tangential component g_t is just circulating in the surface of dA .

$$g' = R g R' = \begin{pmatrix} u_s & s_h \\ -s_h & u_s \end{pmatrix}$$

total flow out of this region

$$= \iint_{\partial R} g \cdot n \cdot dA = \iint_{\partial R} g_i n_i dA = -\frac{\partial}{\partial t} \iiint_R \xi dV = -\iiint_R \frac{\partial \xi}{\partial t} dV$$

(we already subtracted the compressibility term).

$\frac{\partial}{\partial t} \iiint \xi dV$ - mass transfer across the outer boundary

$\iint g \cdot n \cdot dA$ - total volumetric flux of fluid leaving this region per unit time!

div theorem

$$\iiint_R q_{i,i} dV = - \iiint \frac{\partial \zeta}{\partial t} dV$$

$$\iiint_R \left(q_{i,i} + \frac{\partial \zeta}{\partial t} \right) dV = 0$$

$$q_{i,i} + \frac{\partial \zeta}{\partial t} = 0$$

How about $q_{i,i}$?

→ use Darcy's law.

$$q = -\frac{1}{\mu} \underline{K} \text{grad } P_p \Rightarrow q_i = -\frac{1}{\mu} K_{ij} P_{p,j}$$

$$q_1 = -\frac{1}{\mu} \left(K_{11} \frac{\partial P}{\partial x_1} + K_{12} \frac{\partial P}{\partial x_2} + K_{13} \frac{\partial P}{\partial x_3} \right)$$

similarly for q_2 & q_3 .

$$\underline{K} = \begin{pmatrix} K_{11} & K_{12} & K_{13} \\ K_{21} & K_{22} & K_{23} \\ K_{31} & K_{32} & K_{33} \end{pmatrix}$$

theorem. \underline{K} is symmetric, i.e., $K_{ij} = K_{ji}$ ex) $K_{12} = K_{21}$.

For isotropic

Robert think there is no proof about this empiricism

$$\underline{K} = K \delta_{ij} = \begin{pmatrix} K & 0 & 0 \\ 0 & K & 0 \\ 0 & 0 & K \end{pmatrix}$$

[Darcy $\doteq 10^{-12} \text{ m}^2$]

don't use K , hydraulic conductivity.

$$q_i = -\frac{K}{\mu} \delta_{ij} P_{p,j} = -\frac{K}{\mu} P_{p,i} = -\frac{K}{\mu} \text{grad } P$$

$$\cancel{q_i} - \frac{K}{\mu} P_{p,i,i} + \frac{\partial \zeta}{\partial t} = 0 \quad \frac{\partial \zeta}{\partial t} = \frac{K}{\mu} P_{p,i,i} = \frac{K}{\mu} \nabla^2 P_p$$

$$\frac{\partial \zeta}{\partial t} = \frac{K}{\mu} \nabla^2 P_p$$

$$\nabla^2 P_p = \oplus \quad \frac{\partial \zeta}{\partial t} > 0$$

$$\nabla^2 P_p = \ominus \quad \frac{\partial \zeta}{\partial t} < 0$$

h : a definition at a steady state ability to transmit fluid.

a measure of how quickly a material can carry flow away from a source.

$$d\zeta = -\phi \left[C_{pc} dp_c - (C_{pp} + C_f) dp_p \right]$$

special case $p_c = 0$ → uncoupled case.

$$d\zeta = \phi (C_{pp} + C_f) dp_p$$

$$\phi (C_{pp} + C_f) \frac{dp_p}{dt} = \frac{k}{\mu} \left(\frac{\partial^2 p_p}{\partial x^2} + \frac{\partial^2 p_p}{\partial y^2} + \frac{\partial^2 p_p}{\partial z^2} \right)$$

$$\frac{\partial p_p}{\partial t} = \frac{k}{\phi \mu (C_{pp} + C_f)} \nabla^2 p_p \rightarrow \text{important for petrole}$$

For non rigid case, rigid $\rightarrow C_{pp} = 0$
 $(C_{pc} = 0) \rightarrow \frac{\partial p_p}{\partial t} = \frac{k}{\phi \mu C_f} \nabla^2 p_p$

recall $p_p = M \zeta + \alpha M \epsilon_b$

$$\zeta = \frac{1}{M} p_p - \alpha \epsilon_b$$

$$\epsilon_b = \frac{1}{K} (p_c - \alpha p_p)$$

$$\zeta = -\frac{1}{K} p_c + \frac{\alpha}{K} p_p$$

$$\frac{1}{M} \frac{\partial p_p}{\partial t} - \alpha \frac{\partial \epsilon_b}{\partial t} = \frac{k}{\mu} \nabla^2 p_p$$

diffusivity for constant strain

$$\frac{\partial p_p}{\partial t} = \alpha M \frac{\partial \epsilon_b}{\partial t} + \frac{M k}{\mu} \nabla^2 p_p$$

→ fourth Equation.

express in terms of pressure/stress

$$\epsilon_b = \frac{1}{K} (p_c - \alpha p_p)$$

$$\frac{\partial p_p}{\partial t} = \frac{\alpha M}{K} \frac{\partial p_c}{\partial t} - \frac{\alpha^2 M}{K} \frac{\partial p_p}{\partial t} + \frac{M k}{\mu} \nabla^2 p_p$$

$$\frac{\partial p_p}{\partial t} \left(1 + \frac{\alpha^2 M}{K} \right) = \frac{\alpha M p_c}{K} + \frac{M k}{\mu} \nabla^2 p_p$$

$$M = \frac{BK}{\alpha(1-\alpha B)}$$

$$\frac{\partial p_p}{\partial t} \left(1 + \frac{\alpha B}{1-\alpha B} \right) = \frac{B}{1-\alpha B} \frac{\partial p_c}{\partial t} + \frac{BKk}{\alpha \mu (1-\alpha B)} \nabla^2 p_p$$

$$\frac{\partial p_p}{\partial t} = B \frac{\partial T_m}{\partial t} + \frac{BKk}{\mu \alpha} \nabla^2 p_p$$

diffusivity for constant stress.

if we don't consider fluid, we see Skempton

$$\frac{k}{\rho \alpha} = \alpha$$

specific heat thermal capacity diffusivity.

$$\frac{k}{\mu S}$$

Hydraulic diffusivity.

Special case 1, $\tau_m = \text{constant}$.

$$\frac{\partial p_p}{\partial t} = \frac{BKk}{\mu d} \nabla^2 p_p$$

↪ diffusivity coeff.

Special case 2, $\epsilon_b = \text{constant}$

$$\frac{\partial p_p}{\partial t} = \frac{BK}{\alpha(1-\alpha B)} \frac{k}{\mu} \nabla^2 p_p$$

↪ replace k with $\frac{K}{1-\alpha B}$.

See, p. ¹⁸⁸ 437-439

$$\Rightarrow \frac{\partial \Sigma}{\partial t} = \frac{K}{\mu S} \nabla^2 \Sigma, \quad S = \frac{1}{M} + \frac{\alpha^2}{\lambda + 2\mu} = \frac{1}{M} + \frac{\alpha^2}{K + \frac{2}{3}\mu}$$

no coupling terms? But coupled in the Boundary conditions
 $= (C_f - C_m) \phi + \left[1 + \frac{2(1-\nu)\alpha}{3(1-\nu)} \right] \alpha C_{BC}$
 (Zimmerman)

undrained moduli

$$K_u = \frac{K}{1-\alpha B}$$

$$(\mu_u = \mu)$$

$$G_u = G$$

recall, $\epsilon^{dev} = \frac{1}{2\mu} \epsilon_m$

$$\nu = \frac{3K-2\mu}{6K+2\mu}$$

$$\nu_u = \frac{3K_u-2\mu}{6K_u+2\mu}$$

$$\nu_u = \frac{3 \cdot \frac{K}{1-\alpha B} - 2\mu}{6 \cdot \frac{K}{1-\alpha B} + 2\mu} = \frac{3K-2\mu(1-\alpha B)}{6K+2\mu(1-\alpha B)}$$

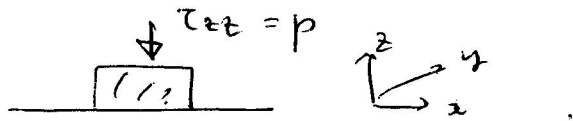
$$\nu_u > \nu$$

$$0 < \nu < \nu_u < 0.5$$

$$0.15-0.25 \quad 0.25-0.35$$

soil $\rightarrow 1/2$ not so different
 But water is different.

Terzaghi's problem. (consolidation)



time to equilibriate $\tau_{zz} > 0$
 $p_f \uparrow$

$$t = \frac{L^2}{D} = \frac{k^2}{k}$$

function of both size (L) & k .

General expression

$$\frac{\partial \Sigma}{\partial t} = \frac{k}{\mu} \nabla^2 p_p$$

① uncoupled case

$$\frac{\partial p_p}{\partial t} = \frac{k}{\phi \mu (c_{pp} + c_f)} \nabla^2 p_p,$$

② uncoupled case - rigid rock

$$\frac{\partial p_p}{\partial t} = \frac{k}{\phi \mu c_f} \nabla^2 p_p$$

$$\textcircled{3} \quad \frac{\partial p_p}{\partial t} = \alpha M \frac{\partial \epsilon_k}{\partial t} + \left(\frac{M k}{\mu} \right) \nabla^2 p_p$$

coupled case - expressed in terms of strain.
diffusivity under constant strain.

④ coupled case - expressed in terms of stress.

$$\frac{\partial p_p}{\partial t} = B \frac{\partial \tau_m}{\partial t} + \left(\frac{B K k}{\mu d} \right) \nabla^2 p_p$$

diffusivity for constant stress.

* Specific storage coefficient^{ate}, S , $(1/Pa)$
 ratio of the change in the volume of water added to the storage per unit aquifer volume divided by the change in pore pressure.

$$\frac{\Delta V_f}{V} = \frac{S}{P_p}$$

① rigid rock

$$S = \phi C_f \rightarrow \frac{\partial P_p}{\partial t} = \frac{h}{\phi \mu C_f} \nabla^2 P_p, \quad c = \frac{h}{\phi \mu C_f}$$

② under constant stress, S_σ $\left| \frac{\Delta V_f}{P_p} \right|_{\sigma=0}$

$$\Delta V_f = -\frac{\alpha}{K} P_c + \frac{\alpha}{K_B} P_p$$

$$S_\sigma = \frac{\alpha}{K_B}$$

$$C_{BC} = \frac{C_{PC}}{C_{PP} + C_f}$$

$$K = C_{BC}$$

$$C_{BC} - C_{PP} = C_m$$

$$C_{BP} = \phi C_{PC}$$

$$C_{BC} - C_{PP} = C_m$$

$$\frac{\partial P_p}{\partial t} = \frac{h}{\mu S_\sigma} \nabla^2 P_p - \frac{\partial P_c}{\partial t}$$

③ under constant strain, S_ϵ $\left| \frac{\Delta V_f}{P_p} \right|_{\epsilon=0}$

$$\epsilon = \frac{1}{K} P_c - \frac{\alpha}{K} P_p \rightarrow P_c = \alpha P_p$$

$$\Delta V_f = -\frac{\alpha}{K} P_c + \frac{\alpha}{K_B} P_p = -\frac{\alpha^2}{K} P_p + \frac{\alpha}{K_B} P_p = \frac{\alpha - \alpha^2 \beta}{K_B} P_p$$

$$\frac{\Delta V_f}{P_p} = \frac{\alpha(1-\alpha\beta)}{K_B} = \frac{\alpha}{K\alpha\beta}$$

$$\frac{K_B}{\alpha(1-\alpha\beta)} = \frac{\alpha(1-\alpha\beta)}{K_B}$$

$$\frac{\partial P_p}{\partial t} = \frac{h}{\mu S_\epsilon} \nabla^2 P_p - \alpha \mu \frac{\partial \epsilon}{\partial t}$$

④ 'hydrologic' definition = "the volume of water released ~~from~~ per unit decline of head per unit bulk volume while maintaining the REV in a state of zero lateral strain and constant vertical stress"

$$S_s = (\rho_f g) \cdot \frac{\Delta V_f}{P} \Big|_{\epsilon_{xx} = \epsilon_{yy} = 0, \tau_{zz} = 0}$$

$$\frac{\partial P_p}{\partial t} = \frac{h}{\mu S_{hydro}} \nabla^2 P_p$$

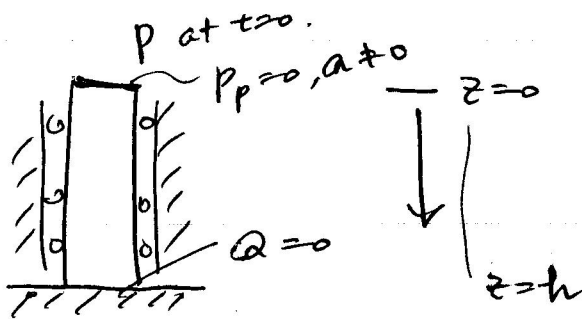
Specific storage coefficient for hydrologist, S_{hydro}

$$S_{\text{hydro}} = \frac{S_s}{\rho_f g} = S_0 \left(1 - \frac{4\eta B}{3}\right)$$

$$\eta = \frac{1-2\nu}{2(1-\nu)} \alpha$$

★

* 1D Consolidation,



at $t=0$, $\sigma = p$, pore pressure = $B \cdot p$.
displacement = undrained E .

$$\tau_{zz}^0 = p, \quad \tau_{xz}^0 = \tau_{xy}^0 = \nu P (1-\nu).$$

$$\epsilon_{zz}^0 = \frac{P}{(\lambda + 2G + \alpha^2 M)}, \quad \epsilon_{xz}^0 = \epsilon_{xy}^0 = 0.$$

shear stresses & strains are zero.

$$P_p^0 = \frac{\alpha M}{(\lambda + 2G + \alpha^2 M)} P.$$

$$w^0 = \frac{P}{\lambda + 2G + \alpha^2 M} (z-h)$$

satisfy Navier & diffusion equation.
with σ or ϵ .

$$G \nabla^2 \underline{u} + (\lambda + G) \nabla (\nabla \cdot \underline{u}) = -f - \alpha \nabla P_p.$$

$$(\lambda + 2G) \frac{\partial^2 w(z,t)}{\partial z^2} = -\alpha \frac{\partial P_p(z,t)}{\partial z}$$

by integrating,

$$\left(\lambda + 2G \right) \frac{\partial w(z,t)}{\partial z} + \alpha P_p(z,t) = g(t)$$

$$\tau_{zz}(z,t) = g(t). \quad z=0 \rightarrow \tau_{zz} = p.$$

$$\tau_{zz}(z,t) = p.$$

$$\epsilon_{zz} = \frac{\partial w}{\partial z} = \frac{1}{\lambda + 2G} [p - \alpha P_p(z,t)] \quad \text{Integrate}$$

in uniaxial strain $\Sigma_b = \epsilon_{zz}$

Date

No.

$$\frac{\partial \epsilon_b}{\partial t} = \frac{-\alpha}{\lambda + 2G} \frac{\partial P_p}{\partial t}$$

$$+ \frac{\partial P_p}{\partial t} = \frac{kM}{\mu} \nabla^2 P_p + \alpha M \frac{\partial \epsilon_b}{\partial t}$$

$$\frac{k}{\mu} \nabla^2 P_p = \left(\frac{1}{\mu} + \frac{\alpha^2}{\lambda + 2G} \right) \frac{\partial P_p}{\partial t} = S \frac{\partial P_p}{\partial t}$$

diffusivity $D = \frac{k}{\mu S}$: coefficient of consolidation.

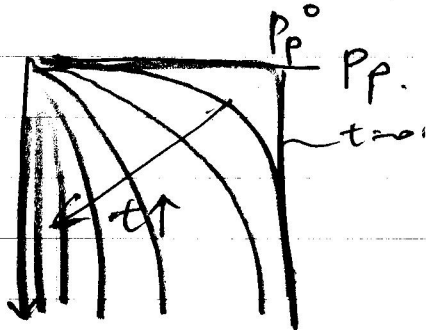
Solve this solution.

$$t=0, P_p(z, 0) = P_p^0 = \frac{\alpha M}{\lambda + 2G + \alpha^2 M} p$$

$$P_p(z=0, t) = 0, \quad \frac{\partial P_p}{\partial z}(z=h, t) = 0$$

The solution of this can be obtained from heat conduction Equation. (Carslaw & Jaeger 1959)

$$P_p(z, t) = \frac{\alpha M P_p^0}{\lambda + 2G + \alpha^2 M} \sum_{n=1,3,\dots}^{\infty} \frac{4}{n\pi} \sin\left(\frac{n\pi z}{2h}\right) \exp\left(-\frac{n^2 \pi^2 k t}{4\mu S h^2}\right)$$



$$D = 1 \times 10^{-2} \frac{m^2}{s}$$

until time, $\exp(-t) < 0$.

$$t_{95} = \frac{20 \mu S h^2}{\pi^2 k} \doteq 2 \mu S h^2 / k$$

t a few second ~ several years.

Berea sandstone

$$C_f = 5 \times 10^{-4} / \text{MPa}$$

$$h = 190 \text{ mD} = 190 \times 10^{-15} \text{ m}^2, \text{ Table 7.2.}$$

$$\mu = 10^3 \text{ Pa}\cdot\text{s.} \leftarrow \text{Berea Sandstone}$$

Westerly granite.

$$S = \phi C_f = 0.2 \times 5 \times 10^{-4} \times 10^{-6}$$

$$\frac{2 \times 10^{-3} \times 0.2 \times 5 \times 10^{-10} \times 1}{\pi^2 \times (90 \times 10^{-15})} \quad \text{Date } \cdot 10.0 \quad \text{No. } = \frac{2 \times 10^{-3}}{\pi^2 \times 10^0}$$

$$w(z, t) = \frac{P}{\lambda + 2G} \left[(z-h) + \frac{\alpha^2 M h}{\lambda + 2G + \alpha^2 M} \sum_{n=1,3}^{\infty} \frac{8}{n^2 \pi^2} \cos\left(\frac{n\pi z}{2h}\right) \times \exp\left(\frac{-n^2 \pi^2 k t}{4\mu S h^2}\right) \right]$$

w at z=0,

$$w(0, t) = \frac{-Ph}{\lambda + 2G} \left[1 - \frac{\alpha^2 M}{\lambda + 2G + \alpha^2 M} \sum_{n=1,3}^{\infty} \frac{8}{n^2 \pi^2} \exp\left(\frac{-n^2 \pi^2 k t}{4\mu S h^2}\right) \right]$$

$$t=0 \quad \frac{1}{1} + \frac{1}{3^2} + \frac{1}{5^2} + \dots = \frac{\pi^2}{8} \quad w = \frac{-Ph}{(\lambda + 2G + \alpha^2 M)}$$

undrained uniaxial strain.

$$t=\infty \quad w = \frac{-Ph}{\lambda + 2G}$$