

Airy Stress Function ϕ

①

노트 제목

① In Cartesian coords,

$$\nabla^4 \phi = \nabla^2 \nabla^2 \phi = \left(\frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2} \right)^2 \phi(x, y) = 0$$

$$\sigma_{xx} = \frac{\partial^2 \phi}{\partial y^2}, \quad \sigma_{yy} = \frac{\partial^2 \phi}{\partial x^2}, \quad \sigma_{xy} = -\frac{\partial^2 \phi}{\partial x \partial y} \quad (a)$$

Note: $\sigma_{xx} + \sigma_{yy} = \text{tr } \underline{\underline{\sigma}} = \nabla^2 \phi$

observation:

$$\begin{cases} \sigma_{xx} = \phi_{,yy} = \nabla^2 \phi - \phi_{,xx} \\ \sigma_{yy} = \phi_{,xx} = \nabla^2 \phi - \phi_{,yy} \\ \sigma_{xy} = -\phi_{,xy} = 0 - \phi_{,xy} \end{cases}$$

$$\Rightarrow \boxed{\sigma_{\alpha\beta} = \delta_{\alpha\beta} \nabla^2 \phi - \phi_{,\alpha\beta}} \quad (\alpha=1, 2)$$

$$\underline{\underline{i.e.}} \quad \boxed{\underline{\underline{\sigma}} = \nabla^2 \phi \underline{\underline{1}} - \underline{\underline{\nabla}} \underline{\underline{\nabla}} \phi} \quad (*)$$

symbolic form

now can be used in any coordinates!

② In polar coordinates,

use $\bullet (*)$

$$\begin{cases} \bullet \nabla^2 \phi(r, \theta) = \frac{\partial^2 \phi}{\partial r^2} + \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} \\ \bullet \underline{\underline{\nabla}} \phi = \underline{\underline{e}}_r \frac{\partial \phi}{\partial r} + \underline{\underline{e}}_\theta \frac{1}{r} \frac{\partial \phi}{\partial \theta} \end{cases}$$

(2)

Some details

$$\begin{aligned}
 \underline{\nabla} \underline{\nabla} \phi &= \left(\underline{e}_r \frac{\partial}{\partial r} + \underline{e}_\theta \frac{1}{r} \frac{\partial}{\partial \theta} \right) \otimes \left(\underline{e}_r \frac{\partial \phi}{\partial r} + \underline{e}_\theta \frac{1}{r} \frac{\partial \phi}{\partial \theta} \right) \\
 &= \underline{e}_r \otimes \underline{e}_r \frac{\partial^2 \phi}{\partial r^2} + \underline{e}_r \otimes \underline{e}_\theta \left(-\frac{1}{r^2} \frac{\partial \phi}{\partial \theta} + \frac{1}{r} \frac{\partial^2 \phi}{\partial r \partial \theta} \right) \\
 &\quad + \underline{e}_\theta \otimes \underline{e}_r \frac{1}{r} \frac{\partial^2 \phi}{\partial r \partial \theta} + \underline{e}_\theta \otimes \underline{e}_\theta \frac{1}{r} \frac{\partial \phi}{\partial r} \\
 &\quad - \underline{e}_\theta \otimes \underline{e}_r \frac{1}{r^2} \frac{\partial \phi}{\partial \theta} + \underline{e}_\theta \otimes \underline{e}_\theta \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} \\
 &= \frac{\partial^2 \phi}{\partial r^2} \underline{e}_r \otimes \underline{e}_r + \left(-\frac{1}{r^2} \frac{\partial \phi}{\partial \theta} + \frac{1}{r} \frac{\partial^2 \phi}{\partial r \partial \theta} \right) (\underline{e}_r \otimes \underline{e}_\theta \\
 &\quad + \underline{e}_\theta \otimes \underline{e}_r) + \underline{e}_\theta \otimes \underline{e}_\theta \left(\frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} \right)
 \end{aligned}$$

$$\left\{ \begin{aligned}
 \sigma_{rr} &= \nabla^2 \phi - \underline{e}_r \cdot (\underline{\nabla} \underline{\nabla} \phi) \underline{e}_r \\
 &= \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} \\
 \sigma_{\theta\theta} &= \nabla^2 \phi - \underline{e}_\theta \cdot (\underline{\nabla} \underline{\nabla} \phi) \underline{e}_\theta = \frac{\partial^2 \phi}{\partial r^2} \\
 \sigma_{r\theta} &= -\underline{e}_r \cdot (\underline{\nabla} \underline{\nabla} \phi) \underline{e}_\theta \quad (= -\underline{e}_\theta \cdot \underline{\nabla} \underline{\nabla} \phi \underline{e}_r) \\
 &= \frac{1}{r^2} \frac{\partial \phi}{\partial \theta} - \frac{1}{r} \frac{\partial^2 \phi}{\partial r \partial \theta}
 \end{aligned} \right.$$

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Some typical solutions of $\nabla^4 \phi(x, y) = 0$
in polynomial form

a) Choose $\phi = \frac{a_2}{2} x^2 + b_2 xy + \frac{c_2}{2} y^2 \equiv \phi_2$
 \rightarrow automatically satisfies $\nabla^4 \phi_2 = 0$
for any a_2, b_2, c_2

b) Choose $\phi = \frac{a_4}{12} x^4 + \frac{b_4}{6} x^2 y +$
 $\frac{c_4}{2} x^2 y^2 + \frac{d_4}{6} xy^3 + \frac{e_4}{12} y^4 \equiv \phi_4$

To satisfy $\nabla^4 \phi_4 = 0$

$$e_4 = -2(2c_4 + a_4),$$

a_4, b_4, c_4, d_4 arbitrary

Among these, consider the stress
field due to nonzero b_2 and d_4

$$\phi = b_2 xy + \frac{d_4}{6} xy^3$$

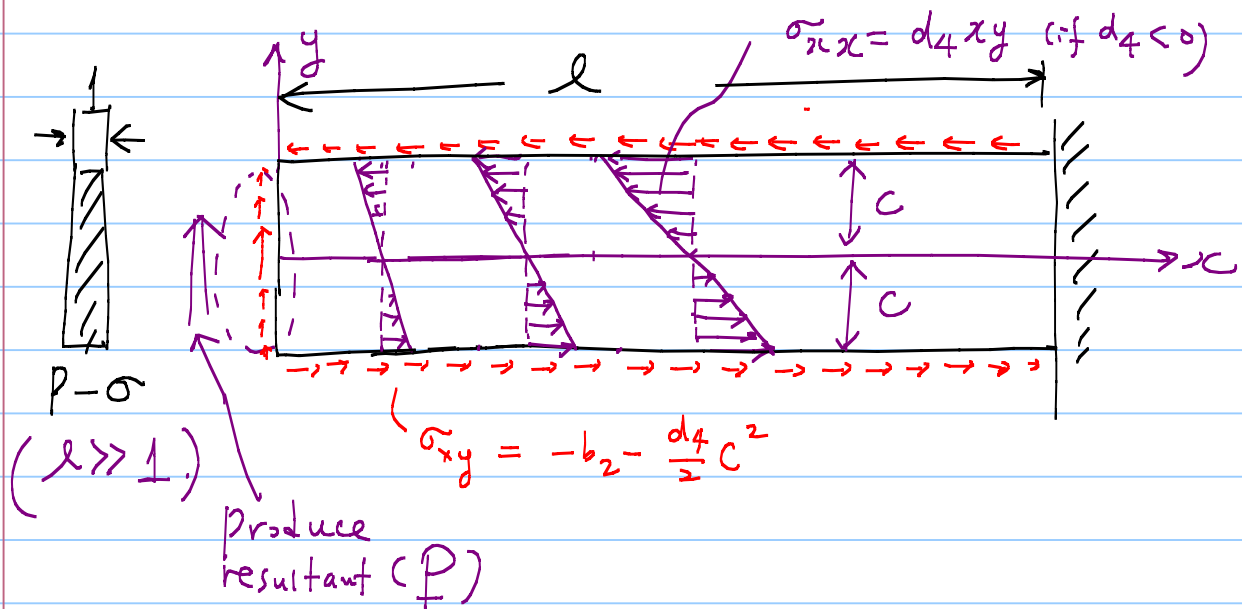
$$\cdot \sigma_{xx} = \frac{\partial^2 \phi}{\partial y^2} = d_4 xy$$

④

$$\sigma_{yy} = \frac{\partial^2 \phi}{\partial x^2} = 0$$

$$\sigma_{xy} = -b_2 - \frac{d_4}{2} y^2$$

• See what problem can be solved by this distribution



• Consider the following case \rightarrow "Cantilever Beam"

i) traction free on $y = \pm c$

ii) the sum of shear stress $\Big|_{x=0} = P$

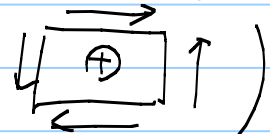
(iii) Let's not worry about the BC at $x=l$ in the meantime.)

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$$i) \quad \sigma_{xy} \Big|_{y=\pm c} = -b_2 - \frac{d_4}{2} c^2 = 0$$

$$\therefore d_4 = -\frac{2b_2}{c^2}$$

$$ii) \quad P = - \int_{-c}^c \sigma_{xy} dy \quad (\text{positive } \sigma_{xy} \text{ upward})$$


$$= \int_{-c}^c (b_2 - \frac{b^2}{c^2} y^2) dy$$

$$\Rightarrow b_2 = \frac{3}{4} \frac{P}{c} \quad \& \quad d_4 = -\frac{3P}{2c^3}$$

Thus

$$\begin{cases} \sigma_{xx} = -\frac{3}{2} \frac{P}{c^3} xy \\ \sigma_{yy} = 0 \\ \sigma_{xy} = -\frac{3P}{4c} \left(1 - \frac{y^2}{c^2}\right) \end{cases}$$

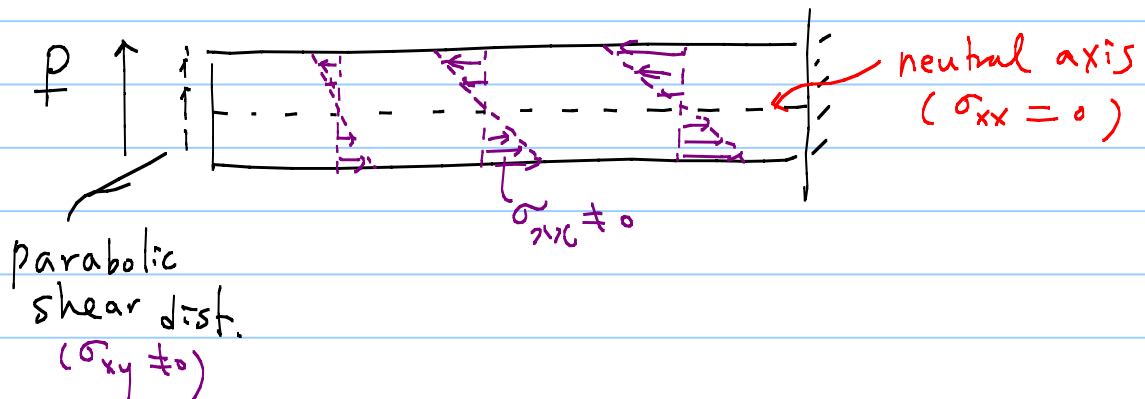
Using the definition of moment of inertia I

$$I = \frac{(1)}{12} (2c)^3 = \frac{2}{3} c^3$$

⑥

The stress components can be written as

$$\begin{aligned}\sigma_{xx} &= -\frac{P}{I} xy \\ \sigma_{yy} &= 0 \\ \sigma_{xy} &= -\frac{P}{I} \frac{1}{2} (c^2 - y^2)\end{aligned} \quad (*)$$



Observations

i) (*) \Rightarrow the result by the elementary beam theory

ii) (*) is exact only if the shear force applied at both ends are distributed according to the parabolic law.

①

□ Determine Disp Field due to (*) ?

$$(\sigma_{xx}, \sigma_{xy}, \sigma_{yy})$$

⇓ ← --- constitutive relation

$$(\epsilon_{xx}, \epsilon_{xy}, \epsilon_{yy})$$

⇓ ← --- Integration

$$(u_x, u_y)$$

<Analysis>

$$\begin{pmatrix} u = u_x \\ v = u_y \end{pmatrix}$$

$$\epsilon_{xx} = \frac{\partial u}{\partial x} = \frac{1}{E} (\sigma_{xx} - \nu \sigma_{yy}) = -\frac{Pxy}{EI} \quad (a)$$

$$\epsilon_{yy} = \frac{\partial v}{\partial y} = \frac{1}{E} (\sigma_{yy} - \nu \sigma_{xx}) = \frac{\nu Pxy}{EI} \quad (b)$$

$$2\epsilon_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} = \frac{\sigma_{xy}}{G} = -\frac{P}{2GI} (c^2 - y^2) \quad (c)$$

Integrate (a, b):

$$u = -\frac{Px^2y}{2EI} + f(y) \quad (d)$$

$$v = \frac{\nu Pxy^2}{2EI} + f_1(x) \quad (e)$$

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(d, e) → (c)

$$-\frac{Px^2}{2EI} + \frac{df(y)}{dy} + \frac{\nu Py^2}{2EI} + \frac{df_1(x)}{dx} = -\frac{P}{2GI}(c^2 - y^2)$$

OR

$$-\underbrace{\frac{Px^2}{2EI} + \frac{df_1(x)}{dx}}_{\text{fn of } x} = -\underbrace{\frac{df(y)}{dy} - \frac{\nu Py^2}{2EI} + \frac{P}{2GI}(y^2 - c^2)}_{\text{fn of } y}$$

≡ d
↑ constant

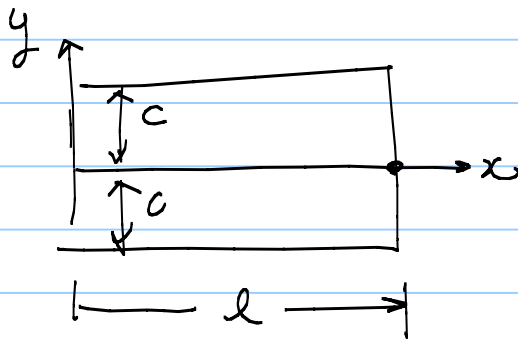
$$\Rightarrow \left\{ \begin{array}{l} \frac{df_1(x)}{dx} = \frac{Px^2}{2EI} + d \quad (f) \\ \frac{df}{dy} = -\frac{\nu Py^2}{2EI} + \frac{P}{2GI}(y^2 - c^2) - d \quad (g) \end{array} \right.$$

Integrating (f, g) yields

$$\left\{ \begin{array}{l} f_1(x) = \frac{Px^3}{6EI} + dx + h \quad \leftarrow \text{Integration const} \\ f(y) = -\frac{\nu Py^3}{6EI} + \frac{Py^3}{6GI} - \left(\frac{Pc^2}{2GI} + d\right)y + g \quad \leftarrow \text{integration constant} \end{array} \right.$$

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Remark: to determine d, h, g
 We need to provide three conditions
 \Rightarrow consider the disp condition $x=l$:



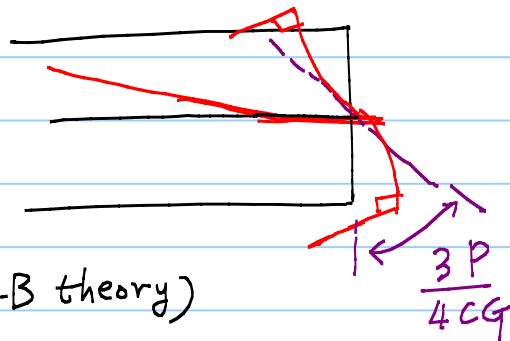
at $x=l$ and $y=0$

① $u = 0$

② $v = 0$

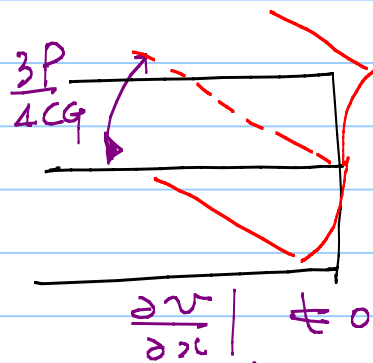
③ $\frac{\partial v}{\partial x} \Big|_{y=0} = 0$

Slope of neutral axis $\equiv 0$
 (Usual condition in E-B theory)



OR

③ $\frac{\partial u}{\partial y} \Big|_{y=0} = 0$
 Slope of normal $\equiv 0$



$\frac{\partial v}{\partial x} \Big|_{x=l, y=0} \neq 0$

(Due to shear angle)

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Case I: Solving for ①, ②, ③

$$d = -\frac{Pl^2}{2EI}, \quad h = \frac{Pl^3}{3EI}, \quad g = 0$$

$$u = -\frac{Px^2y}{2EI} - \frac{vPy^3}{6EI} + \frac{Py^3}{6GI} + \left(\frac{Pl^2}{2EI} - \frac{Pc^2}{2IG}\right)y$$

$$v = \frac{vPxy^2}{2EI} + \frac{Px^3}{6EI} - \frac{Pl^2x}{2EI} + \frac{Pl^3}{3EI}$$

1) Along $y=0$

$$\bullet \quad v|_{y=0} = \frac{Px^3}{6EI} - \frac{Pl^2x}{2EI} + \frac{Pl^3}{3EI}$$

(Deflection of neutral axis)

$$\bullet \quad \therefore v|_{\substack{x=0 \\ y=0}} = \frac{Pl^3}{3EI}$$

(tip displacement)

2) Check u displacement at $x=l$

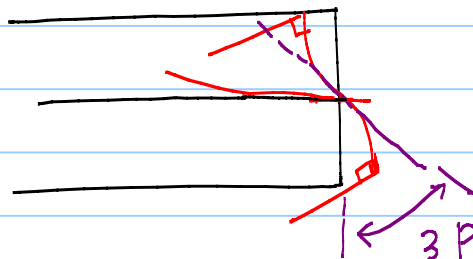
$$u|_{x=l} = -\frac{vPy^3}{6EI} + \frac{Py^3}{6GI} - \frac{Pc^2y}{2GI}$$



①

$$\left. \frac{\partial u}{\partial y} \right|_{x=l} = -\frac{\nu P y^2}{2EI} + \frac{P y^2}{2GI} - \frac{Pc^2}{2GI}$$

$$\therefore \left. \frac{\partial u}{\partial y} \right|_{\substack{x=l \\ y=0}} = -\frac{Pc^2}{2GI} = -\frac{3}{4} \frac{P}{CG}$$



$\frac{3P}{4CG}$ ← due to shear
stress $\sigma_{xy}|_{y=0} = -\frac{3P}{4C}$

→ Normals do not remain normal; "distortion" occurs.

(II) (Case II: Solving for ①, ②, ③')

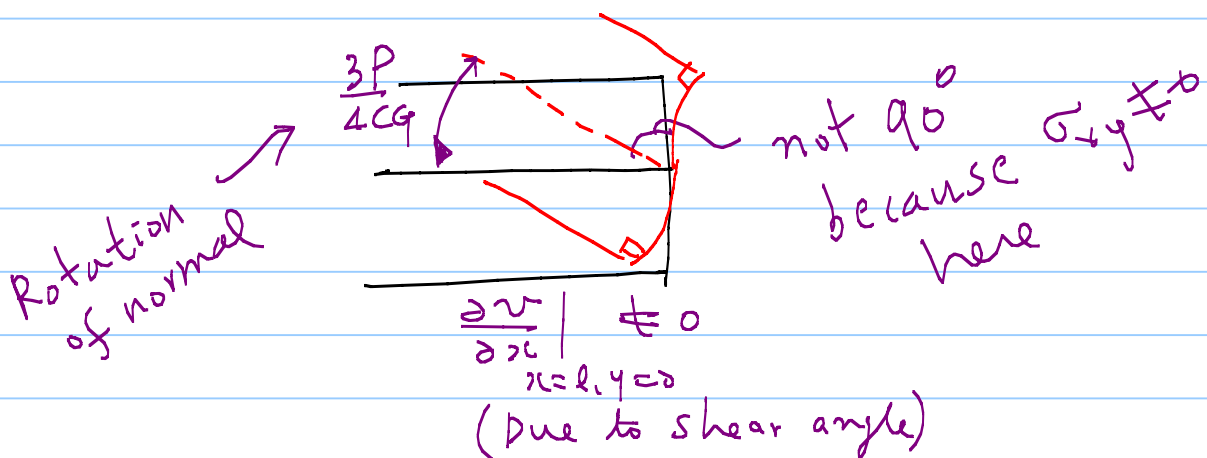
$$w|_{y=0} = \frac{Px^3}{6EI} - \frac{Pl^2x}{2EI} + \frac{Pl^3}{3EI}$$

Shear modulus ← $\left(\frac{Pc^2}{2GI} (l-x) \right)$ ← additional term

(2)

$$\left. \frac{\partial v}{\partial x} \right|_{y=0} = \frac{Px^2}{2EI} - \frac{Pl^2}{2EI} - \frac{Pc^2}{2GI}$$

$$\left. \frac{\partial v}{\partial x} \right|_{y=0, x=l} = -\frac{Pc^2}{2GI} = -\frac{3P}{4CG}$$



* Observation from CASE I & CASE II

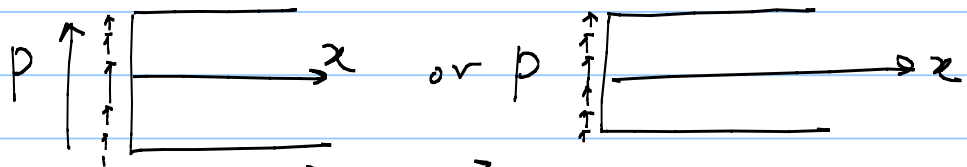
$$\varnothing \left. v \right|_{y=0, \text{CASE II}} = \left. v \right|_{y=0, \text{CASE I}} + \underbrace{\frac{Pc^2}{2IG}(l-x)}_{\text{positive term}}$$

additional deflection due to rotation of the neutral axis

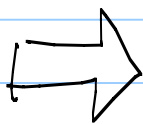
(13)

$\therefore \frac{Pc^2}{2IG} (l-x)$: gives an estimate of the effect of shearing force on the deflection

② What if the applied load is not distributed as a power law?



- same "Resultant" but different distribution



Solutions (stress field) away from the loaded end are virtually same for the two loads as long as the resultant is the same

"St. Venant Principle"

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- Stress difference in two solutions?
→ decays rapidly (usually exponentially)

These local solutions: "End-effects" (solution)