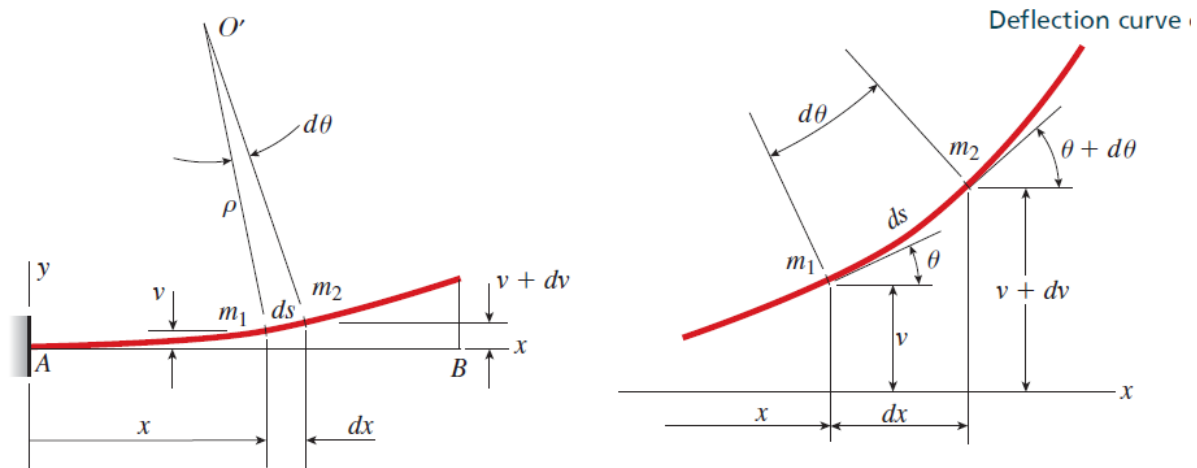


Chapter 9 Deflections of Beams

9.2 Differential Equations of the Deflection Curve



⊙ Sign Conventions and Main Concepts

1. **Deflection** v : Displacement in y -direction at a point (upward positive)
2. **Angle of rotation** θ : Angle between x -axis and tangent to the deflection curve (counterclockwise positive)
3. **Center of curvature** O' : Intersection of the orthogonal lines at two points on the curve
4. **Radius of curvature** ρ : Distance between O' and the deflection curve (Recall $\rho d\theta = ds$)
5. **Curvature** κ : Reciprocal of the radius of curvature, i.e. $\kappa = 1/\rho = d\theta/ds$ (positive)

6. Slope of the deflection curve

$$\frac{dv}{dx} = \tan \theta \quad \theta = \arctan \frac{dv}{dx}$$

In a similar manner

$$\cos \theta = \frac{dx}{ds} \quad \sin \theta = \frac{dv}{ds}$$

7. These are all based only on geometric considerations, and thus valid for beams of any material. No restrictions on the magnitudes of the slopes and deflections.

⊙ Beams with Small Angles of Rotation

1. Very small angle of rotations and deflections → approximation $ds \approx dx$ greatly simplify beam analysis

2. Curvature

$$\kappa = \frac{1}{\rho} \approx \frac{d\theta}{dx}$$

3. Angle of rotation (equal to the slope; angle should be given in radians)

$$\theta \approx \tan \theta = \frac{dv}{dx}$$

4. Curvature (in terms of deflection curve):

$$\kappa = \frac{1}{\rho} \approx \frac{d\theta}{dx} = \frac{d^2v}{dx^2}$$

Note: Exact curvature (See textbook)

$$\kappa = \frac{d\theta}{ds} = \frac{v''}{[1 + (v')^2]^{3/2}}$$

5. For a **linearly elastic** beam (i.e. following Hooke's law), $\kappa = 1/\rho = M/EI$, we now obtain **differential equation of the deflection curve** as

$$\frac{d^2v}{dx^2} = v'' = \frac{M}{EI}$$

⊙ Nonprismatic Beams

1. From above, but with flexural rigidity varying over x , i.e. EI_x ,

$$EI_x \frac{d^2v}{dx^2} = M$$

2. From $dM/dx = V$ and $dV/dx = -q$

$$\frac{d}{dx} \left(EI_x \frac{d^2v}{dx^2} \right) = \frac{dM}{dx} = V$$

$$\frac{d^2}{dx^2} \left(EI_x \frac{d^2v}{dx^2} \right) = \frac{d^2M}{dx^2} = \frac{dV}{dx} = -q$$

⊙ Prismatic Beams

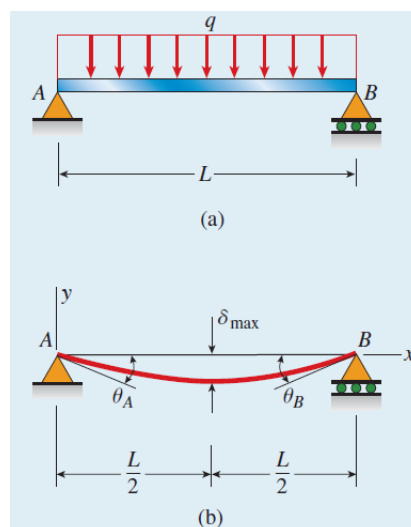
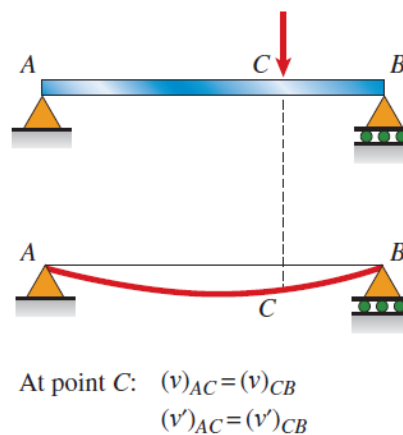
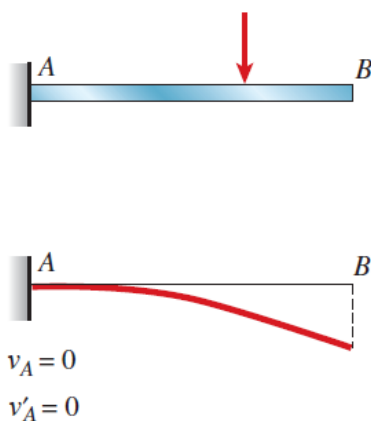
1. Differential equations for prismatic beams, i.e. $EI_x = EI$

- 1) Bending-moment equation $EIv'' = M$
- 2) Shear-force equation $EIv''' = V$
- 3) Load equation $EIv'''' = -q$

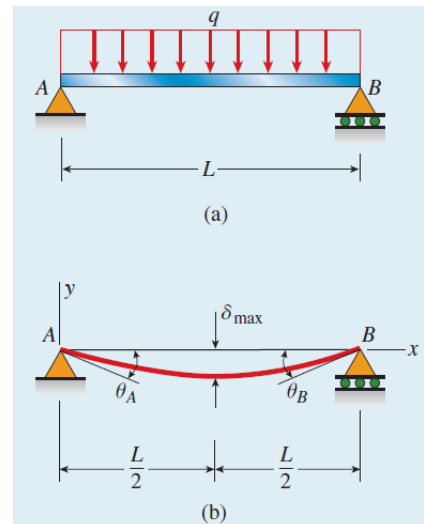
9.3 Deflections by Integration of the Bending-Moment Equation

⊙ Conditions needed for Solving Bending-Moment Equations by **Method of Successive Integrations** (i.e. integrate the differential equation and find the undetermined coefficients by given conditions)

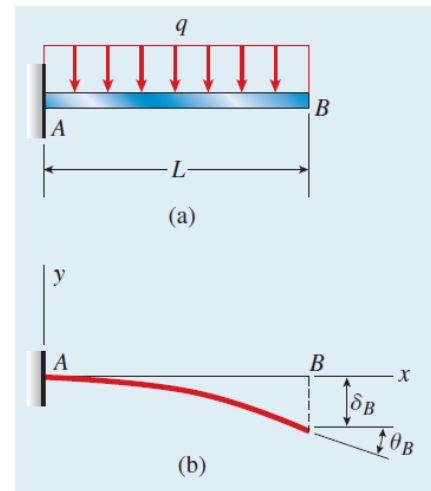
1. **Boundary conditions:** Deflection and slope at boundaries
2. **Continuity conditions:** At a given point, the deflections (or slopes) obtained for the left- and right-hand parts should be equal
3. **Symmetry conditions:** For example, the slope of the deflection curve at the midpoint is zero (for a symmetric beam under symmetric loads)



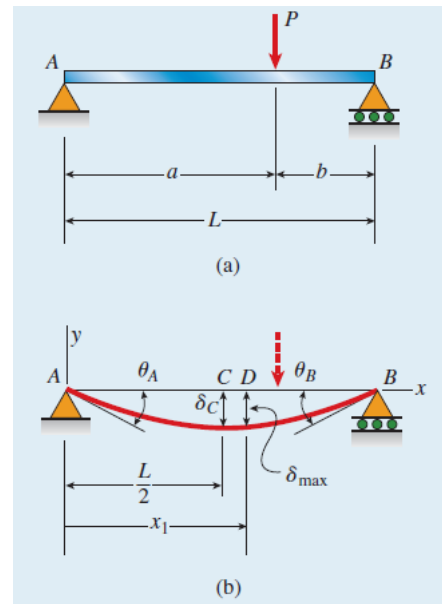
- ⊙ **Example 9-1:** Determine the equation of the deflection curve for a simple beam supporting a uniform load of intensity q . Also determine the maximum deflection δ_{\max} at the midpoint of the beam, and the angles of rotation at the supports, i.e. θ_A and θ_B . The beam has length L and constant flexural rigidity EI .



- ⊙ **Example 9-2:** Determine the equation of the deflection curve for a cantilever beam subjected to a uniform load of intensity q . Also determine the angle of rotation θ_B and the deflection δ_B at the free end. The beam has length L and constant flexural rigidity EI .



- ⊙ **Example 9-3:** Determine the equation of the deflection curve for a simple beam subjected to a concentrated load P acting at distances a and b from the left- and right-hand supports, respectively. Also determine the angles of rotation θ_A and θ_B , the maximum deflection δ_{\max} , and the deflection δ_C at the midpoint of the beam. The beam has length L and constant flexural rigidity EI .



9.4 Deflections by Integration of the Shear-Force and Load Equations

⊙ Solving Differential Equations for Deflections

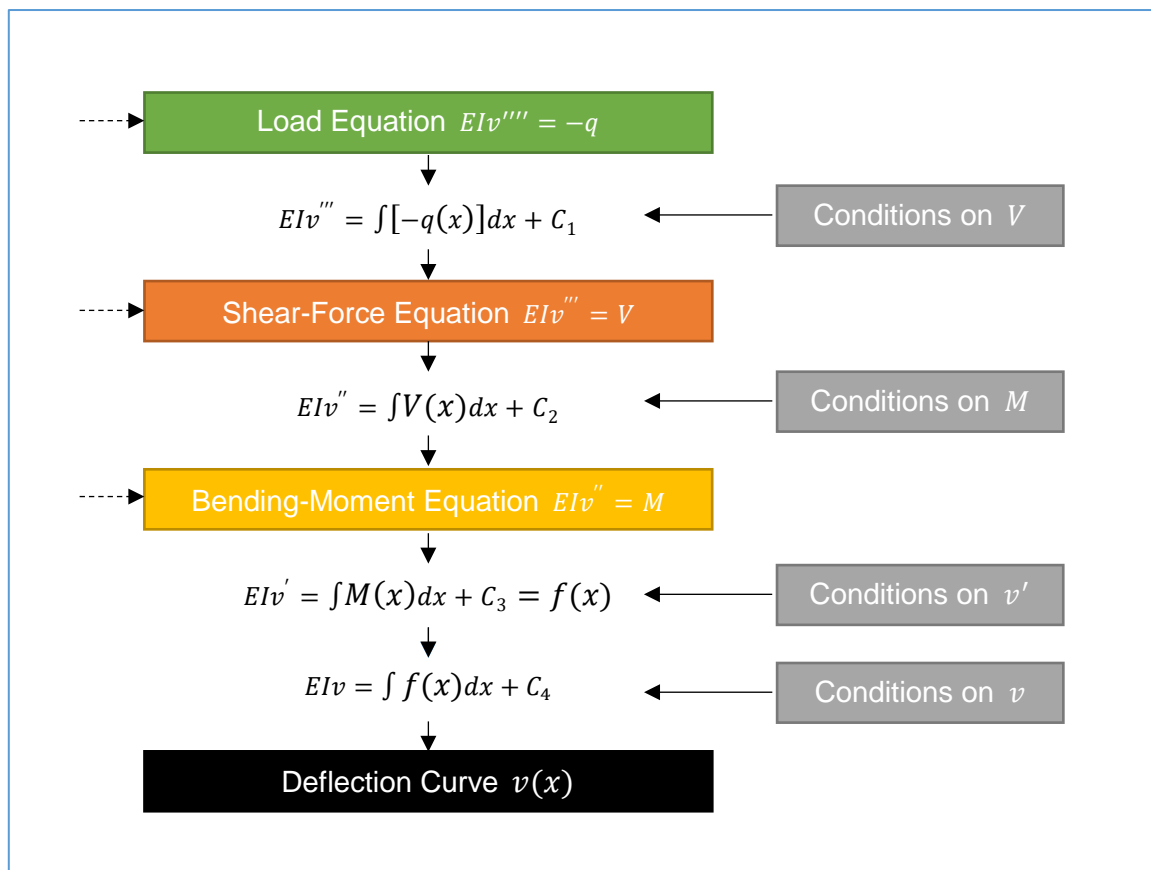
1. Among three differential equations for deflections, i.e.

- 1) Bending-moment equation $EIv'' = M$ (_____ unknowns)
- 2) Shear-force equation $EIv''' = V$ (_____ unknowns)
- 3) Load equation $EIv'''' = -q$ (_____ unknowns)

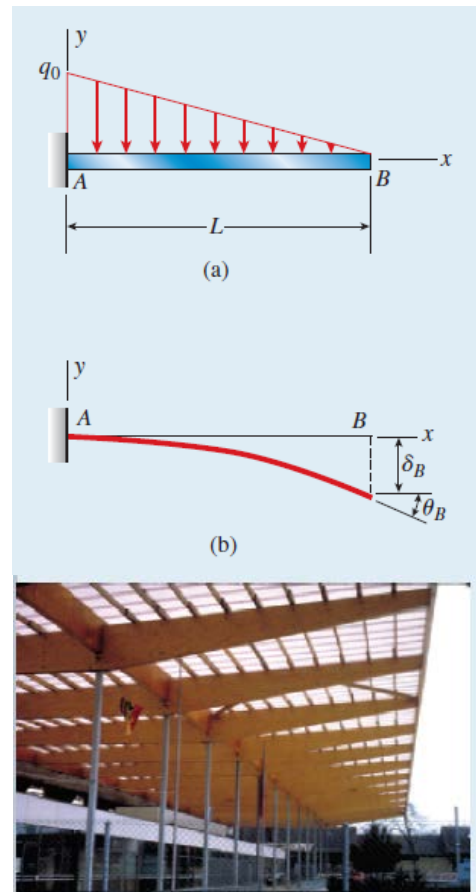
One can solve a differential equation based on available conditions

2. Boundary, continuity and symmetry conditions are available in terms of

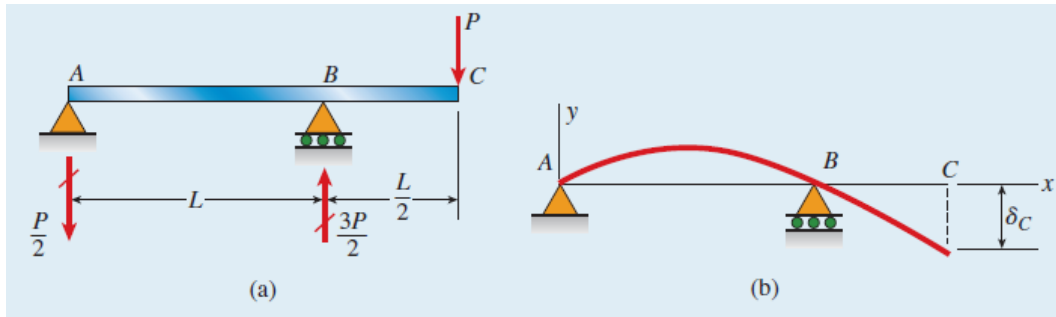
- 1) Deflection (v) and slope (v') for bending-moment equation (Section 9.3)
- 2) In addition, moment (M) conditions can be used for the shear-force equation because $EIv'' = M$
- 3) In addition, shear force (V) conditions can be used for solving the load equation because $EIv''' = V$



- ⊙ **Example 9-4:** Determine the equation of the deflection curve for a cantilever beam AB supporting a triangularly distributed load of maximum intensity q_0 . Also determine the deflection δ_B and angle of rotation θ_B at the free end. Use the load equation.



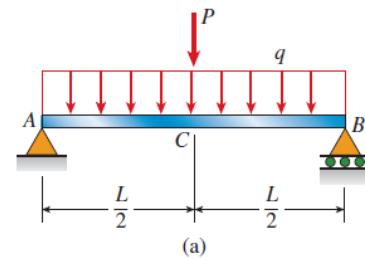
- ⊙ **Example 9-5:** Determine the equation of the deflection curve for a simple beam with an overhang under concentrated load P at the end. Also determine the deflection δ_C . Use the third-order differential equation, i.e. shear-force equation. The beam has constant flexural rigidity EI .



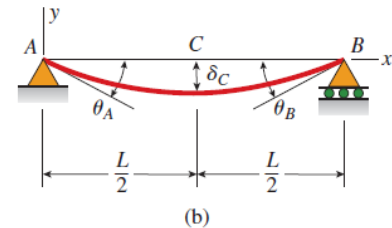
9.5 Method of Superposition

⊙ Method of Superposition

- Under suitable conditions, the deflection of a beam produced by several different loads acting simultaneously can be found by summing the deflections produced by the same loads acting separately.



- At a given location, the deflection v_1 by a load q_1 (alone) and v_2 by a load q_2 (alone) → the deflection under loads q_1 and q_2 is _____



- Justification: the differential equation for deflection is _____ ~ “Principle of Superposition” (mathematics)

- “Principle of Superposition” is valid under the following conditions:

(1) Hooke’s law for the material

(2) Deflections and rotations are small

(3) Presence of deflection does not alter the actions of the applied loads

→ Three sources of nonlinearity in general structural engineering problems: nonlinear material property (constitutive law, large deformation, load-displacement interactions)

⊙ Tables of Beam Deflections

- Superpose the deflection equations or deflections and slopes at specific locations determined for each type of the loads

Table G-1	
Deflections and Slopes of Cantilever Beams	
	$v =$ deflection in the y direction (positive upward) $v' = dv/dx =$ slope of the deflection curve $\delta_B = -v(L) =$ deflection at end B of the beam (positive downward) $\theta_B = -v'(L) =$ angle of rotation at end B of the beam (positive clockwise) $EI =$ constant
1	$v = -\frac{qx^2}{24EI}(6L^2 - 4Lx + x^2) \quad v' = -\frac{qx}{6EI}(3L^2 - 3Lx + x^2)$ $\delta_B = \frac{qL^4}{8EI} \quad \theta_B = \frac{qL^3}{6EI}$
2	$v = -\frac{qx^2}{24EI}(6a^2 - 4ax + x^2) \quad (0 \leq x \leq a)$ $v' = -\frac{qx}{6EI}(3a^2 - 3ax + x^2) \quad (0 \leq x \leq a)$ $v = -\frac{qa^3}{24EI}(4x - a) \quad v' = -\frac{qa^2}{6EI} \quad (a \leq x \leq L)$ $\text{At } x = a: v = -\frac{qa^3}{8EI} \quad v' = -\frac{qa^2}{6EI}$ $\delta_B = \frac{qa^3}{24EI}(4L - a) \quad \theta_B = \frac{qa^2}{6EI}$

- If a given pattern of distributed loads is not available in the table, one can use the results about concentrated loads as shown in the following example

3. From Appendix G, the midpoint deflection caused by a concentrated load P at $x = a$ ($a \leq b$) is

$$\frac{Pa}{48EI} (3L^2 - 4a^2)$$

4. The midpoint deflection caused by $q dx$ at location x is thus

$$\frac{(q dx)x}{48EI} (3L^2 - 4x^2)$$

5. In the example, the distributed load is given as

$$q(x) = \frac{2q_0x}{L}$$

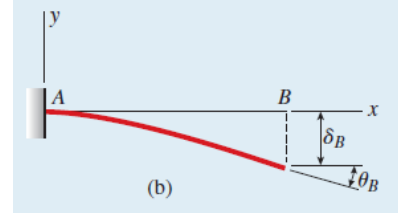
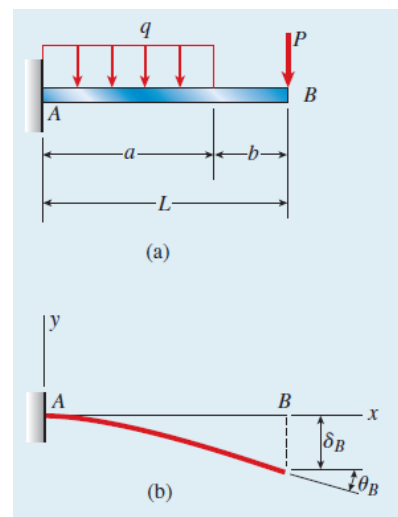
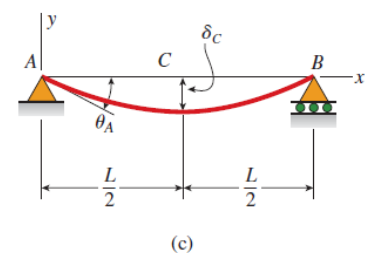
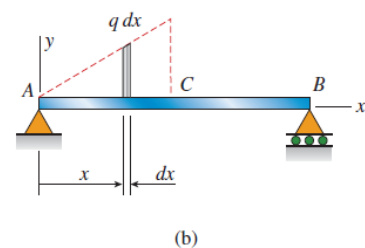
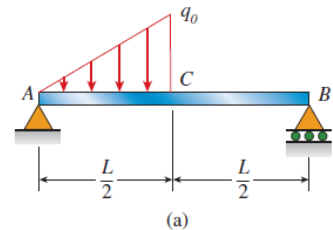
6. Substituting this into the equation in "4",

$$\frac{q_0x^2}{24EI} (3L^2 - 4x^2) dx$$

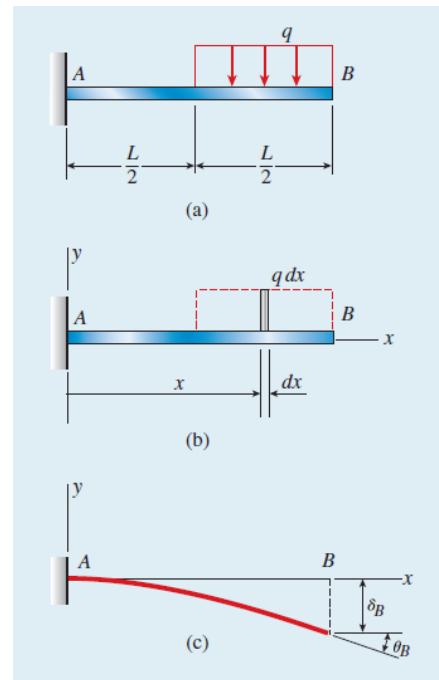
7. Finally, the deflection at C is obtained by the integral

$$\delta_C = \int_0^{\frac{L}{2}} \frac{q_0x^2}{24EI} (3L^2 - 4x^2) dx = \frac{q_0L^4}{240EI}$$

- ⊙ **Example 9-6:** A cantilever beam AB supports a uniform load of intensity q acting over part of the span and a concentrated load P acting at the free end. Determine the deflection δ_B and the angle of rotation θ_B at end B .



- ⊙ **Example 9-7:** A cantilever beam AB supports a uniform load of intensity q acting on the right-hand half of the beam. Determine the deflection δ_B and the angle of rotation θ_B at the free end.



9.6 Moment-Area Method

⊙ Moment-Area Method

1. Refers to two theorems related to the area of the b_____ -m_____ diagram
 $M(x) \rightarrow$ Find the angle and deflection of a beam
2. Assumptions: L_____ e_____ materials and s_____ deformation

⊙ First Moment-Area Theorem

1. The angle between the two tangents at points A and B

$$\theta_{B/A} = \theta_B - \theta_A$$

2. The angle between two orthogonal lines at m_1 and m_2

$$d\theta = \frac{ds}{\rho} \cong \frac{dx}{\rho}$$

3. For a beam consisting of a linear elastic material $1/\rho = M/EI$

$$d\theta = \frac{Mdx}{EI}$$

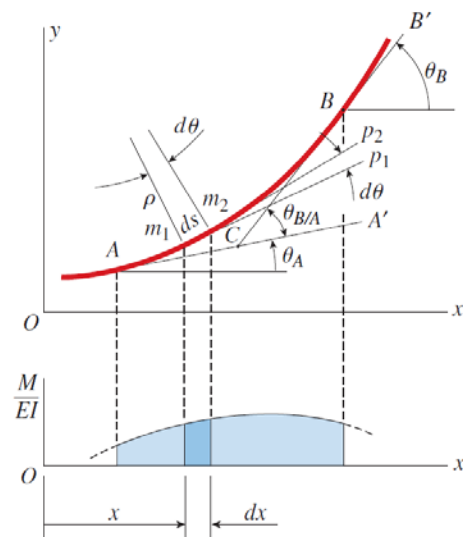
4. Integrating the change in the angle from point A to point B ,

$$\int_A^B d\theta = \theta_{B/A}$$

$$\theta_{B/A} = \int_A^B \frac{Mdx}{EI}$$

= Area of the M/EI diagram between points A and B

5. **First moment-area theorem:** The angle $\theta_{B/A}$ between the tangents to the deflection curve at two points A and B is equal to the area of the M/EI diagram between those points



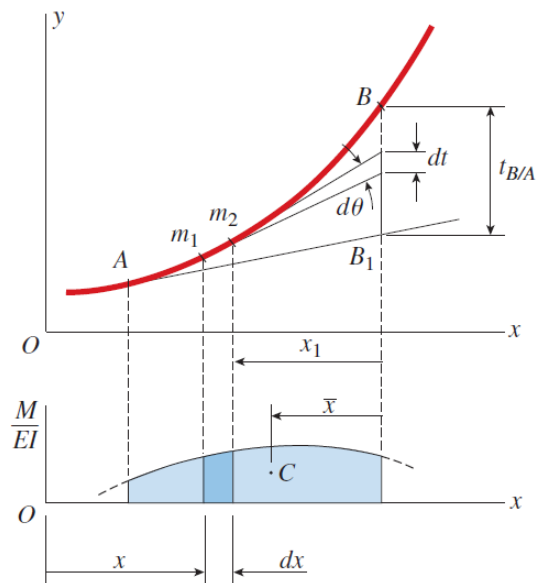
⊙ Second Moment-Area Theorem

1. Tangent deviation of point B with respect to A : $t_{B/A}$
2. Tangent deviation of point B made by the element m_1m_2 :

$$dt = x_1 d\theta = x_1 \frac{M dx}{EI}$$

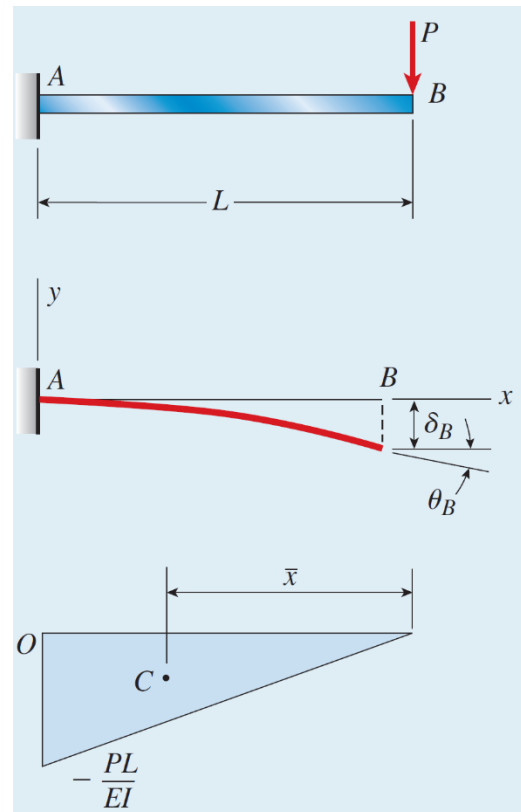
3. This is the contribution of the element m_1m_2 to the total tangent deviation. Thus, the total tangent deviation is

$$t_{B/A} = \int_A^B dt = \int_A^B x_1 \frac{M dx}{EI}$$

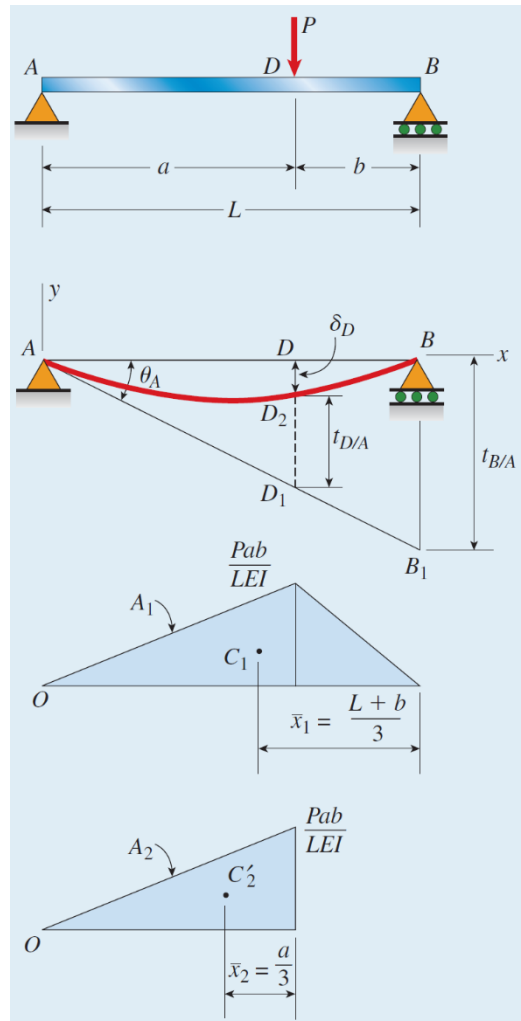


4. This is the first moment of the area of the M/EI diagram between points A and B , evaluated with respect to point B
5. **Second moment-area theorem:** The tangential deviation $t_{B/A}$ of point B from the tangent at point A is equal to the first moment of the area of the M/EI diagram between A and B , evaluated with respect to B .
6. **Note:** The first moment can be computed alternative by () \times ()

- ⊙ **Example 9-10:** Determine the angle of rotation θ_B and deflection δ_B at the free end B of a cantilever beam AB supporting a concentrated load P using the moment-area method.



- ⊙ **Example 9-12:** A simple beam ADB supports a concentrated load P acting at the position shown in the figure. Determine the angle of rotation θ_A at support A and the deflection δ_D under the load P using the moment area method.



9.8 Strain Energy of Bending

⊙ Strain Energy of a Beam under Bending

1. Consider a linear elastic beam under pure bending showing small deformation, i.e. the bending moment is constant and Hooke's law works
2. The angle of the circular arc is

$$\theta = \frac{L}{\rho} = \kappa L = \frac{ML}{EI}$$

3. Linear relationship between M and θ → Therefore, when the bending moment gradually increases, the work and the stored strain energy are

$$W = U = \frac{M\theta}{2}$$

4. Using the linear relationship in Item 3, the strain energy can be expressed as

$$U = \frac{M^2 L}{2EI} \quad \text{or} \quad U = \frac{EI\theta^2}{2L}$$

5. If the bending moment varies, the angle between the side faces of a small element with length dx is

$$d\theta = \kappa dx = \frac{d^2 v}{dx^2} dx$$

6. In analogy to Item 4, the strain energy in the element is

$$dU = \frac{M^2 dx}{2EI} \quad \text{or} \quad dU = \frac{EI(d\theta)^2}{2dx} = \frac{EI}{2dx} \left(\frac{d^2 v}{dx^2} dx \right)^2 = \frac{EI}{2} \left(\frac{d^2 v}{dx^2} \right)^2 dx$$

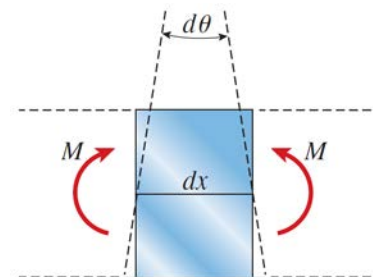
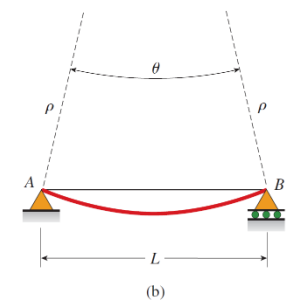
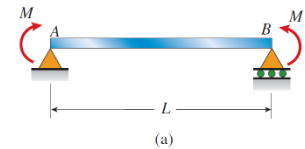
7. By integrating the strain energy along the beam, the strain energy is derived as

$$U = \int \frac{M^2 dx}{2EI} \quad \text{or} \quad U = \int \frac{EI}{2} \left(\frac{d^2 v}{dx^2} \right)^2 dx$$

⊙ Deflections Caused by a Single Load

1. Strain energy U by a concentrated load P and a couple M_0 is $\frac{P\delta}{2}$ and $\frac{M_0\theta}{2}$
2. Deflection and the rotation can be computed by

$$\delta = \frac{2U}{P} \quad \text{and} \quad \theta = \frac{2U}{M_0}$$



- ⊙ **Example 9-6:** Cantilever beam AB is subjected to three different loading conditions: (a) a concentrated load P at its free end, (b) a couple M_0 at its free end, and (c) both loads acting simultaneously. For each loading condition, determine the strain energy of the beam. Also, determine the vertical deflection δ_A at end A of the beam due to the load P acting alone, and determine the angle of rotation θ_A at end A due to the moment M_0 acting alone.

