Damage Modelling in Ductile Materials

Specific research topic: A comparative study on three fracture models for incremental sheet metal forming with Al1050 aluminium alloy.

Degradation of material properties and ability of load carrying capacity of material termed as damage, after appreciable damage rupture/crack formation occurs.

Ductile damage: (local approaches or CDM)

Mechanism: void nucleation, growth and coalescence. (sometimes shear fracture in voids may happen in shear loading conditions)



Fig. 1. Schematic of nucleation, growth and coalescence of voids,

Source: https://doi.org/10.1016/j.engfracmech.2017.09.021

Damage Modelling in Ductile Materials

Major models used widely:

- 1. GTN Model : fully coupled (in sense of yield function and damage variable [f])
- 2. CDM Model (Lemaitre, Chaboche, Murakami)
- 3. Uncoupled fracture Models (BW, MMC, HC etc)



FIGURE 11.3

Schematic representation of the void coalescence through the process of void impingement. (a) Initial configuration, (b) necking starts, (c) necking progresses further and (d) fnal void impingement.





Source: https://doi.org/10.1016/j.engfracmech.2017.09.021

stress triaxiality



Fracture Model in stress triaxiality space



Source: https://doi.org/10.1016/j.engfracmech.2018.07.014

BW Model

Damage initiation based on estimating equivalent plastic strain at the onset of fracture wrt triaxiality

Three pronged model developed by conducting experiments with varying triaxialities and finding point of onset of fracture

$$\begin{split} \bar{\varepsilon}_{0}^{p} &= \frac{C_{1}}{(1+3\eta)} \quad \left\{-\frac{1}{3} \leq \eta \leq 0\right\} \\ \bar{\varepsilon}_{0}^{p} &= C_{1} + (C_{2} - C_{1})(\eta/\eta_{0})^{2} \quad \left\{0 \leq \eta \leq \eta_{0}\right\} \\ \bar{\varepsilon}_{0}^{p} &= C_{2}(\eta_{0}/\eta) \quad \left\{\eta \geq \eta_{0}\right\} \\ \omega &= \int \frac{d\varepsilon^{p}}{\bar{\varepsilon}_{0}^{p}} \end{split}$$

Source: https://doi.org/10.1016/j.engfracmech.2018.07.014

BW Model

C1: in pure shear; C2: in uniaxial tension; n₀ average triaxiality in uniaxial tension test

No fracture for triaxialities less than -1/3

Damage initiation marked by variable omega as it reaches unity. Takes care of history of triaxialities in the loading process.

Experimental determination of C2 has two major assumptions:

1. Location of onset of damage coincides with location of maximum equivalent plastic strain at the instant of onset of fracture.

2. The numerical simulation of tensile test with the best fit of piecewise linear extrapolation post necking.



Fig. 22 3D geometric representation of Mohr–Coulomb fracture locus for 2024-T351 aluminum alloy. (A = 740 MPa, n = 0.15, $c_1 = 0.0345$, $c_2 = 338.6$ MPa, $c_{\theta}^s = c_{\theta}^c = 1.0$)



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Stress state in terms of Triaxiality and Lode angle parameter

where [S] is the deviatoric stress tensor defined by, $[S] = [\sigma] + p[I],$ Mohr Coulomb in terms of ε , η , $\overline{\theta}$

$$\left(\sqrt{1+c_1^2}+c_1\right)\sigma_1 - \left(\sqrt{1+c_1^2}-c_1\right)\sigma_3 = 2c_2, \qquad \sigma_1 \ge \sigma_2 \ge \sigma_3.$$

$$\bar{\sigma} = c_2 \left[\sqrt{\frac{1+c_1^2}{3}}\cos\left(\frac{\pi}{6}-\theta\right) + c_1\left(\eta+\frac{1}{3}\sin\left(\frac{\pi}{6}-\theta\right)\right)\right]^{-1}$$

$$\bar{\sigma} = A\bar{\varepsilon}^n \left[1-c_\eta(\eta-\eta_\circ)\right] \left[c_\theta^s + (c_\theta^{ax}-c_\theta^s)\gamma\right]$$

$$\gamma = \frac{\sqrt{3}}{2-\sqrt{3}} \left[\sec\left(\theta-\frac{\pi}{6}\right)-1\right]$$

$$\bar{\varepsilon}_f = \left\{\frac{A}{c_2} \left[c_\theta^s + \frac{\sqrt{3}}{2-\sqrt{3}}\left(c_\theta^{ax}-c_\theta^s\right)\left(\sec\left(\frac{\bar{\theta}\pi}{6}\right)-1\right)\right]$$

$$\left[\sqrt{\frac{1+c_1^2}{3}}\cos\left(\frac{\bar{\theta}\pi}{6}\right)+c_1\left(\eta+\frac{1}{3}\sin\left(\frac{\bar{\theta}\pi}{6}\right)\right)\right]\right\}^{-\frac{1}{n}},$$

Hosford - Coulomb Criteria 2015: Dirk

2015: Dirk Mohr et al

Mohr-Coulomb criteria :

$$\max_{\mathbf{n}}[\tau + c_1 \sigma_n] = c_2, \quad \longrightarrow \quad (\sigma_I - \sigma_{III}) + c(\sigma_I + \sigma_{III}) = b, \qquad c = \frac{c_1}{\sqrt{1 + c_1^2}} \quad \text{and} \quad b = \frac{2c_2}{\sqrt{1 + c_1^2}},$$

an extension of the MC criterion is proposed by substituting the Tresca equivalent stress in by the Hosford (1972) equivalent stress

$$ar{\sigma}_{HF} + c(\sigma_I + \sigma_{III}) = b,$$
 $c = rac{c_1}{\sqrt{1 + c_1^2}}$ and $b = rac{2c_2}{\sqrt{1 + c_1^2}},$

$$\bar{\sigma}_{HF} = \left\{ \frac{1}{2} \left((\sigma_I - \sigma_{II})^a + (\sigma_{II} - \sigma_{III})^a + (\sigma_I - \sigma_{III})^a \right) \right\}^{\frac{1}{a}}, \qquad \{0 < a < 2\}$$

$$ar{\sigma}_{HF} + c(\sigma_I + \sigma_{III}) = b,$$
 $c = rac{c_1}{\sqrt{1 + c_1^2}}$ and $b = rac{2c_2}{\sqrt{1 + c_1^2}},$

$$\bar{\sigma}_{HF} = \left\{ \frac{1}{2} \left((\sigma_I - \sigma_{II})^a + (\sigma_{II} - \sigma_{III})^a + (\sigma_I - \sigma_{III})^a \right) \right\}^{\frac{1}{a}}, \qquad \qquad \{0 < a < 2\}$$

For a=1 The above criteria becomes Mohr-Coulomb Criteria

Note: The Hosford criterion becomes non-convex for a < 1. This requires special care when using the Hosford function as yield surface, but there is no restriction with respect to convexity when it is used as localization criterion

$$ar{\sigma}_{HF}+c(\sigma_I+\sigma_{III})=b, \qquad \qquad ar{\sigma}_{HF}=iggl\{rac{1}{2}igl((\sigma_I-\sigma_{II})^a+(\sigma_{II}-\sigma_{III})^a+(\sigma_I-\sigma_{III})^aigr)igr\}^{rac{1}{a}},$$



Source:https://www.sciencedirect.com/science/article/pii/S0020768315000700

$$ar{\sigma}_{HF}+c(\sigma_I+\sigma_{III})=b, \qquad ar{\sigma}_{HF}=iggl\{rac{1}{2}igl((\sigma_I-\sigma_{II})^a+(\sigma_{II}-\sigma_{III})^a+(\sigma_I-\sigma_{III})^aigr)igr\}^{rac{1}{a}},$$

D. Mohr, S.J. Marcadet/International Journal of Solids and Structures 67-68 (2015) 40-55



. Effect of the parameters of the Hosford–Coulomb (HC) model on the fracture envelope for plane stress loading.

Source:https://www.sciencedirect.com/science/article/pii/S0020768315000700

$$\begin{split} \bar{\sigma}_{HF} + c(\sigma_{I} + \sigma_{III}) = b, & \bar{\sigma}_{HF} = \left\{ \frac{1}{2} \left((\sigma_{I} - \sigma_{II})^{a} + (\sigma_{II} - \sigma_{III})^{a} + (\sigma_{I} - \sigma_{III})^{a} \right) \right\}^{\frac{1}{a}}, & \sigma_{I} = \bar{\sigma}(\eta + f_{1}), \\ \bar{\sigma} = \bar{\sigma}_{f}[\eta, \bar{\theta}] = \frac{b}{\{\frac{1}{2} ((f_{1} - f_{2})^{a} + (f_{2} - f_{3})^{a} + (f_{1} - f_{3})^{a})\}^{\frac{1}{a}} + c(2\eta + f_{1} + f_{3})}, & \sigma_{II} = \bar{\sigma}(\eta + f_{2}), \\ \bar{\sigma}_{III} = \bar{\sigma}(\eta + f_{3}), & \sigma_{III} = \bar{\sigma}(\eta + f_{3}), \\ \bar{v}_{III} = \bar{\sigma}(\eta + f_{3}), & f_{I}[\bar{\theta}] = \frac{2}{3} \cos\left[\frac{\pi}{6}(1 - \bar{\theta})\right], \\ \bar{v}_{f} = k^{-1}[\bar{\sigma}_{f}[\eta, \bar{\theta}]]. & f_{2}[\bar{\theta}] = \frac{2}{3} \cos\left[\frac{\pi}{6}(3 + \bar{\theta})\right], \\ \int_{0}^{\bar{v}_{f}} \frac{d\bar{v}_{p}}{\bar{v}_{f}^{pr}[\eta, \bar{\theta}]} = 1, & f_{3}[\bar{\theta}] = -\frac{2}{3} \cos\left[\frac{\pi}{6}(1 + \bar{\theta})\right] \end{split}$$

Vumat model and single element test





single element test validation (pure shear case) || Mohr Coulomb Criteria





single element test validation (pure shear case)

	Mechanical
	Constants
1	71659
2	0.33
3	5
4	120
5	120
6	1
7	1
8	1
9	101.9138
10	0
11	103.5364971
12	0.0005
13	104.7055913

С	User n	leeds	to	input				
С	props((1) -	You	ng's m	odulu:	з,е		
С	props((2) -	Poi	sson's	ratio	o, nu		
С	props((3) -	Con	е				
С	props((4) -	Ctw	0				
С	props((5) -	pow	er law	const	tant		
С	props ((6) -	str	ain ha	rdenin	ng exp	on	
С	props((7) -	har	dening	law (const	CS	
С	props ((8) -	har	dening	law (const	сс	
С	props(9)	- s	yield	and ha	ardeni	ng <mark>dat</mark>	a
F	nu	C1		(2	Δ	n	Cc	<u>(</u>

E	nu	C1	C2	А	n	Сс	Cs
71659	0.33	5	120	120	1	1	1

single element test validation (Pure shear case)

For uniaxial case,

Lode angle parameter ($\hat{\Theta}$) = 0, triaxiality= 0, putting below parameters in MC equation,



single element test validation (Pure shear case)

	X	Y	1
152	0.755	0.953788	
153	0.76	0.960038	
154	0.765	0.966288	
155	0.77	0.972538	
156	0.775	0.978788	
157	0.78	0.985038	
158	0.785	0.991288	
159	0.79	0.997539	
160	0.795	1.00303	
161	0.8	1.00803	
162	0.805	1.01303	
163	0.81	1.01803	1
164	0.815	1.02303	
165	0.82	1.02804	
466	0.005	1 02204	

i tumer se	x	Y	^
152	0.755	0.309076	
153	0.76	0.310951	
154	0.765	0.312827	
155	0.77	0.314702	
156	0.775	0.316577	
157	0.78	0.318452	
158	0.785	0.320328	
159	0.79	0.322203	
160	0.795	0.324078	
161	0.8	0.325953	
162	0.805	0.327829	
163	0.81	0.329704	
164	0.815	0.331579	
165	0.82	0.333454	
166	0.005	0 22522	~

	X	Y	~
34	0.165	0.339683	
35	0.17	0.339683	
36	0.175	0.339683	
37	0.18	0.339683	
38	0.185	0.339683	
39	0.19	0.339683	
40	0.195	0.339683	
41	0.2	0.339683	
42	0.205	0.339683	
43	0.21	0.339683	
44	0.215	0.339683	
45	0.22	0.339683	
46	0.225	0.339683	
47	0.23	0.339683	
48	0.235	0.339683	~

Damage parameter

Eq plastic strain

Fracture strain EPLO

Also, checked through state variables lode angle parameter comes nearly zero (in the order of E-6) and triaxiality fluctuates near zero (in the order of E-9)

single element test validation (Pure shear case)



Linear interpolation

fracture strain = 0.339683

1

Plastic strain

0.322203

0.324078

0.32303227

Omega

0.997539

1.00303

Model calibration and simulation results









Hardening Rule

• Like the approach utilized by Mohr and Marcadet (2015), in the present work, the hardening behaviour till necking point is supposed to be described using the Swift hardening law and after necking is expressed as a linear combination of the Swift equation and no hardening behaviour as follows:

$$\bar{\sigma} = \begin{cases} K(\varepsilon_0 + \bar{\varepsilon}^p)^n & \bar{\varepsilon}^p \leq \bar{\varepsilon}^p_{necking} \\ Q\left[K(\varepsilon_0 + \bar{\varepsilon}^p)^n\right] + (1 - Q)\left[\bar{\sigma}_{UTS}\right] & \bar{\varepsilon}^p > \bar{\varepsilon}^p_{necking} \end{cases}$$

К	eo	Sigma yield	Sigma UTS	n	E	ep_neck	Q
141.0735	0.002707			0.055			0.8
		101.9138	113.2987		71659	0.01962	



Estimation of displacement to fracture (Literature)



S. No	Displacement to fracture (mm)	% load drop
1	2.228	13.01 %



S.No	Displacement to fracture (mm)	% load drop
1	3.067	17.75%



S.No	Displacement to fracture (mm)	% load drop
1	0.742199361	26.466 %

Center Hole Specimen- Force Displacement Response



Comment:

Here, load drop is very high























Damage models

S. No	Damage model	Tests to calibrate
1	B W model	Two tests; 1) Uniaxial Tension(UT) 2) In Plane Shear (ST)
2	M C Model	 Three Tests: 1) ST 2) Notch Test (NT4) (4 mm radius) 3) Centre Hole Test (CH) (2.66 mm dia)
3	HC Model	 Three Tests: 1) ST 2) Notch Test (NT4) (4 mm radius) 3) Centre Hole Test (CH) (2.66 mm dia)

BW Criteria



Mohrs - Coulomb Criteria

% Shear
% NT4
% CH
% UT

90	ef	tri	Lod	e		time	Avg	Tri	
C=[1.1354	0.049035	5 0.23	23		0.53	0.02	616	;
	0.6167	0.617 -	-0.02	6	(0.737	0.59	867	;
	1.677	0.717	0.87	8	(0.702	0.55	284	;
	0.594774	0.6261	0.44	9	(0.742	0.37	77]	;
% C(onstants m	atrix [A]							
*	cl	c2		BW					
00	a	b	С	HC					
00	cl	c2	c3	MMC	2				
A =	[C(1,1)]	C(4,1)	0;		÷	BW			
	1.31	149.7	0.0	5;	÷	HC			
	0.067	80.85	0.9	3];	÷	MMC			
Repre	sentation of	iracture strai	n in HC	c mod	lel	in Haigh-V	Vester	gaard	space

S.No	Exp. Fracture Depth	Predicted
1	12 mm	10.5 mm

S.No	Exp. Fracture Depth	Predicted
1	16.82 mm	12.38

S.No	Exp. Fracture Depth	Predicted
1	12 mm	9.52 mm

S.No	Exp. Fracture Depth	Predicted
1	40 mm	18 mm

FFLD

FFLD Major - Minor Strain Space

FFLD Major - Minor Strain Space 1.28 0 Pyramid 5Lobe 1.27 VWACF ٥ 1.26 ٥ Major Strain, (e1) 1.23 1.23 e1 = -0.1553 e2 + 1.2867 1.22 $R^2 = 0.9852$ 1.21 1.2 0.1 0.15 0.2 0.25 0.3 0.35 0.4 0.45 0.5 Minor Strain, (e2)

Combined

Experimental Fracture Strain-FFLD

Summary

- Objective: to predict fracture for AL1050 in single point incremental sheet metal forming process (SPIF).
- Use of three uncoupled damage models i.e. BW (Bao-wierzbicki), MC (Mohr Coulomb), HC (Hosford Coulomb)
- Model the three model with help of damage parameter Omega which indicates fracture when it becomes unity.
- Material Model developed in ABAQUS subroutine VUMAT and UMAT for the three fracture models and validated by single element tests.
- Shear test, uniaxial tests, Notch test and central hole tests done to calibrate model and find model coefficients for all three models
- Finite element simulations are run with the developed material model (on ABAQUS with VUMAT) to predict fracture for various SPIF shapes like Line test, Pyramid, Five lobe, Variable wall angle conical frustum.
- Comparisons of models based on fracture predictions in SPIF simulations with respect to experimental observations.

Thank You.

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