

Chapter 7. Virtual Work Principles in Framework Analysis

The principle of virtual work can be conveniently used in the formulation of approximate solutions which the direct formulation can't achieve.

various applications { tapered member
distributed load
Geometric nonlinear and elastic critical load

exact or approximate displacement function

⇒ shape function

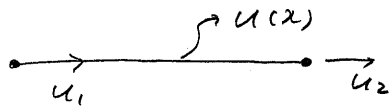
7.1 Description of the displaced state of elements

7.1.1 Definition of the shape function Mode of Description

Ingredients for construction of \underline{k}_e

- 1) Elastic constants $\underline{\epsilon} - \underline{\sigma}$ relationship
- 2) real and virtual displaced states ⇒ shape function
- 3) $\underline{\epsilon} - \underline{u}$ relationship

7.1.2 Formulation of shape function



$$u = N_1 u_1 + N_2 u_2$$

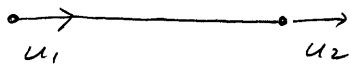
$$= [N_1 \ N_2] \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

$$\underline{\tilde{u}} = \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} : \text{nodal displacement}$$

$$\underline{\tilde{f}} = \begin{bmatrix} N_1 \\ N_2 \end{bmatrix} : \text{shape function}$$

$$u = \text{Generic displacement}$$

Axial force member



$$\text{Equilibrium: } \frac{dF}{dx} = \int u = 0 \quad F = EA \frac{du}{dx}$$

$$\Rightarrow \frac{d^2u}{dx^2} = 0$$

$$u = a_1 + a_2 x \quad \Rightarrow \quad \text{conditions} = \begin{cases} 1) & \text{1st order polynomial} \\ 2) & \text{two displ. B/c} \end{cases}$$

$$\text{B/c } x=0 \quad u = a_1 = u_1$$

$$x=L \quad u = a_1 + a_2 L = u_2$$

$$a_2 = \frac{1}{L}(u_2 - u_1)$$

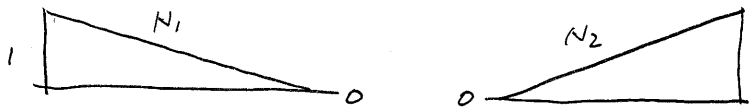
$$u = a_1 + a_2 x$$

$$= u_1 + \frac{1}{L} (u_2 - u_1) x$$

$$= \underbrace{\left(1 - \frac{x}{L}\right)}_{N_1} u_1 + \underbrace{\frac{x}{L}}_{N_2} u_2$$

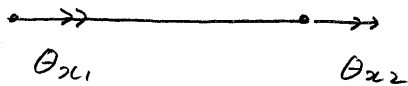
$$N_1 = 1 - \frac{x}{L}$$

$$N_2 = \frac{x}{L}$$



Shape function

Torsional Action



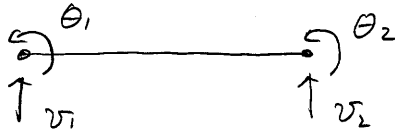
$$\text{Equilibrium} \quad \frac{dM_x}{dx} = m = 0 \quad M_x = GJ \frac{d\theta_x}{dx}$$

$$\Rightarrow \frac{d^2\theta_x}{dx^2} = 0$$

$$\theta_x = a_1 + a_2 x$$

$$= \underbrace{\left(1 - \frac{x}{L}\right)}_{N_1} \theta_1 + \underbrace{\frac{x}{L}}_{N_2} \theta_2$$

flexural Action



Equilibrium $\frac{d^2 M}{dx^2} = \delta v = 0$ $\frac{M}{EI} = \phi = \frac{d^2 v}{dx^2}$

$$\Rightarrow \frac{d^4 v}{dx^4} = 0$$

$$v = a_1 + a_2 x + a_3 x^2 + a_4 x^3 \Rightarrow \text{conditions} \begin{cases} (1) \frac{d^4 v}{dx^4} = 0 \\ (2) \text{ four displ.} \end{cases}$$

B/C

$$x=0 \quad v = a_1 = v_1$$

$$x=L \quad v = a_1 + a_2 L + a_3 L^2 + a_4 L^3 = v_2$$

$$v' = a_2 + 2a_3 x + 3a_4 x^2$$

$$x=0 \quad v'(0) = a_2 = \theta_1$$

$$x=L \quad v'(L) = a_2 + 2a_3 L + 3a_4 L^2 = \theta_2$$

$$\begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 1 & L & L^2 & L^3 \\ 0 & 1 & 2L & 3L^2 \end{bmatrix} \begin{bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \end{bmatrix} = \begin{bmatrix} v_1 \\ \theta_1 \\ v_2 \\ \theta_2 \end{bmatrix}$$


$$a_1 = v_1$$

$$a_2 = \theta_1$$

$$a_3 = \frac{1}{L^2} (-3v_1 + 3v_2 - 2\theta_1 L + \theta_2 L)$$

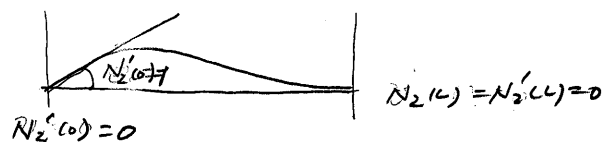
$$a_4 = \frac{1}{L^3} (2v_1 - 2v_2 + \theta_1 L + \theta_2 L)$$

$$v = N_1 v_1 + N_2 \theta_1 + N_3 v_2 + N_4 \theta_2$$

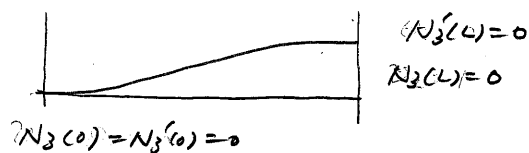
$$N_1 = 1 - 3\left(\frac{x}{L}\right)^2 + 2\left(\frac{x}{L}\right)^3$$


$N_1(0) = 1$ $N_1(L) = N_1'(L) = 0$

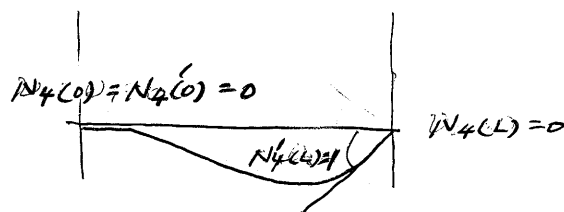
$$N_2 = x\left(1 - \frac{x}{L}\right)^2$$



$$N_3 = 3\left(\frac{x}{L}\right)^2 - 2\left(\frac{x}{L}\right)^3$$



$$N_4 = x\left[\left(\frac{x}{L}\right)^2 - \frac{x}{L}\right]$$



$$x=0, \quad v = v_1 \Rightarrow N_1 = 1 \quad N_2 = N_3 = N_4 = 0$$

$$x=0, \quad v' = \theta_1 \Rightarrow N_2' = 1 \quad N_1' = N_3' = N_4' = 0$$

7.1.3 characteristics of shape function

$$u = N_1 u_1 + N_2 u_2$$

$$\left. \begin{aligned} N_1 &= 1 - \frac{x}{L} \\ N_2 &= \frac{x}{L} \end{aligned} \right\}$$

Shape function

\Rightarrow weighting function

showing influence of u_1 and u_2

7.2 Virtual Displacements in the formulation of element stiffness equations

7.2.1 Construction of expressions for real and virtual Displacements

Axial force member

Generic displacement - nodal displacement

$$u = N_1 u_1 + N_2 u_2$$

$$= [N_1 \ N_2] \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

$$= \underline{N} \underline{q} \quad \underline{q} = \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = \text{nodal displacement vector}$$

Strain - Generic displacement

$$\epsilon = \frac{du}{dx}$$

Strain - nodal displacement

$$\epsilon = \frac{du}{dx} = \underline{N}' \underline{q}$$

$$= [N'_1 \ N'_2] \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

$$= \begin{bmatrix} -\frac{1}{L} & \frac{1}{L} \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

$$\underline{N}' = \begin{bmatrix} -\frac{1}{L} & \frac{1}{L} \end{bmatrix}$$

Stress - strain

$$\sigma = E \epsilon$$

$$= E \underline{N}' \underline{q}$$

Virtual strain Energy

$$\delta U = \int \delta \underline{\underline{\epsilon}} \cdot \underline{\underline{\sigma}} \, dV = \int \delta \underline{\underline{\epsilon}}^T \underline{\underline{\sigma}} \, dV$$

$$\delta \underline{\underline{\epsilon}} = \underline{\underline{N}}' \delta \underline{\underline{q}}$$

$$\underline{\underline{\sigma}} = E \underline{\underline{\epsilon}} = E \underline{\underline{N}}' \underline{\underline{q}}$$

$$\delta U = \int \delta \underline{\underline{\epsilon}}^T \underline{\underline{\sigma}} \, dV$$

$$= \int \delta \underline{\underline{q}}^T \underline{\underline{N}}'^T E \underline{\underline{N}}' \underline{\underline{q}} \, dV$$

$$= \delta \underline{\underline{q}}^T \left[\int \underline{\underline{N}}'^T E \underline{\underline{N}}' \, dV \right] \underline{\underline{q}}$$

Virtual external work due to nodal forces

$$\delta W = \delta \underline{\underline{q}}^T \underline{\underline{P}}$$

$$\delta U = \delta W \Rightarrow$$

$$\delta \underline{\underline{q}}^T \left[\int \underline{\underline{N}}'^T E \underline{\underline{N}}' \, dV \right] \underline{\underline{q}} = \delta \underline{\underline{q}}^T \underline{\underline{P}}$$

$$\Rightarrow \underbrace{\left[\int \underline{\underline{N}}'^T E \underline{\underline{N}}' \, dV \right]}_{\underline{\underline{K}}} \underline{\underline{q}} = \underline{\underline{P}}$$

$$\underline{\underline{K}} = \int \underline{\underline{N}}'^T E \underline{\underline{N}}' \, dV$$

$$= E \int \underline{\underline{N}}'^T \underline{\underline{N}}' \, dV = EA \int_0^L \begin{bmatrix} -\frac{1}{L} \\ \frac{1}{L} \end{bmatrix} \begin{bmatrix} -\frac{1}{L} & \frac{1}{L} \end{bmatrix} dx$$

$$= \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

Torsional member

Generic displacement - nodal displacement

$$\theta = N_1 \theta_1 + N_2 \theta_2$$

$$= \underline{N} \cdot \underline{\theta} \quad \underline{N} = \begin{bmatrix} N_1 \\ N_2 \end{bmatrix} \quad \underline{\theta} = \begin{bmatrix} \theta_1 \\ \theta_2 \end{bmatrix}$$

$$N_1 = 1 - \frac{x}{L}, \quad N_2 = \frac{x}{L}$$

strain - Generic displacement - nodal displacement

$$\gamma = r \frac{d\theta}{dx}$$

$$= r \underline{N}' \cdot \underline{\theta}$$

Stress - strain relationship

$$\tau = G\gamma = Gr \underline{N}' \cdot \underline{\theta}$$

Virtual Strain Energy

$$\delta U = \int \delta \gamma \tau \, dV$$

$$= \delta \underline{\theta}^T \left[\int Gr \underline{N}'^T \underline{N}' r^2 \, dV \right] \underline{\theta}$$

$$= \delta \underline{\theta}^T \left[\int GJ \underline{N}'^T \underline{N}' \, dx \right] \underline{\theta}$$

or

$$\delta U = \int \delta \beta^T M \, dx \quad \beta = \frac{d\theta}{dx} = \underline{N}' \cdot \underline{\theta}$$

$$M = GJ\beta = GJ \underline{N}' \cdot \underline{\theta}$$

$$= \delta \underline{\theta}^T \left[\int GJ \underline{N}'^T \underline{N}' \, dx \right] \underline{\theta}$$

External virtual work

$$\delta W = \delta \underline{\underline{q}}^T \underline{\underline{M}}$$

$$\delta U = \delta W$$

$$\left[\int GJ \underline{\underline{N}}'^T \underline{\underline{N}}' dx \right] \underline{\underline{q}} = \underline{\underline{M}}$$

$$\begin{aligned} \underline{\underline{K}} &= GJ \int \begin{bmatrix} -\frac{1}{L} \\ \frac{1}{L} \end{bmatrix} \begin{bmatrix} -\frac{1}{L} & \frac{1}{L} \end{bmatrix} dx \\ &= \frac{GJ}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \end{aligned}$$

Flexural member

Generic displacement - Nodal displacement

$$v = N_1 v_1 + N_2 \theta_1 + N_3 v_2 + N_4 \theta_2$$

$$N_1 = 1 - 3\left(\frac{x}{L}\right)^2 + 2\left(\frac{x}{L}\right)^3 \quad N_2 = 3\left(\frac{x}{L}\right)^2 - 2\left(\frac{x}{L}\right)^3$$

$$N_3 = x\left(1 - \frac{x}{L}\right)^2 \quad N_4 = x\left[\left(\frac{x}{L}\right)^2 - \frac{x}{L}\right]$$

$$v = \underline{\underline{N}}^T \underline{\underline{q}}$$

$$\underline{\underline{N}} = \begin{bmatrix} N_1 \\ N_2 \\ N_3 \\ N_4 \end{bmatrix} \quad \underline{\underline{q}} = [v_1, \theta_1, v_2, \theta_2]$$

strain - generic displacement - nodal displacement

$$\varepsilon = -\gamma \phi = -\gamma \frac{d^2 v}{dx^2}$$

$$= -\gamma \underline{\underline{N}}'' \underline{\underline{q}}$$

$$= -y \begin{bmatrix} N_1'' & N_2'' & N_3'' & N_4'' \end{bmatrix} \begin{bmatrix} v_1 \\ \theta_1 \\ v_2 \\ \theta_2 \end{bmatrix}$$

$$N_1'' = \frac{6}{L^2} \left(1 - \frac{2x}{L}\right) \quad N_2'' = \frac{2}{L} \left(\frac{3x}{L} - 1\right)$$

$$N_3'' = \frac{6}{L^2} \left(\frac{2x}{L} - 1\right) \quad N_4'' = \frac{2}{L} \left(\frac{3x}{L} - 2\right)$$

stress-strain relationship

$$\sigma = E\varepsilon = -E-y N'' \underline{q}$$

virtual strain energy

$$\begin{aligned} \delta U &= \int \delta \underline{\varepsilon}^T \underline{\sigma} \, dV \\ &= \delta \underline{q}^T \left[\int E y^2 N''^T N'' \, dV \right] \underline{q} \\ &= \delta \underline{q}^T \left[\int EI N''^T N'' \, dx \right] \underline{q} \end{aligned}$$

virtual external energy

$$\delta W = \delta \underline{q}^T \underline{P} \quad \underline{q} = \begin{bmatrix} v_1 \\ \theta_1 \\ v_2 \\ \theta_2 \end{bmatrix}$$

$$\delta U = \delta W \Rightarrow$$

$$\left[\int EI N''^T N'' \, dx \right] \underline{q} = \underline{P}$$

$$\underline{K} = \int EI \underline{N}''^T \underline{N}'' dx$$

$$= \frac{EI}{L} \begin{bmatrix} \frac{12}{L^2} & -\frac{6}{L} & -\frac{12}{L^2} & -\frac{6}{L} \\ & 4 & \frac{6}{L} & 2 \\ \text{sym.} & & \frac{12}{L^2} & \frac{6}{L} \\ & & & 4 \end{bmatrix}$$

Generally,

$$\delta U = \int \delta \underline{\underline{\epsilon}}^T \underline{\underline{\sigma}} dV$$

Generic Displacement - nodal displacement

$$\underline{u} = \underline{f} \underline{\delta} \quad \underline{f} = \text{shape function}$$

$$u = [f_1 \ f_2] \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

$$v = [f_1 \ f_2 \ f_3 \ f_4] \begin{bmatrix} v_1 \\ \theta_1 \\ v_2 \\ \theta_2 \end{bmatrix}$$

Strain - Generic Displacement

$$\underline{\underline{\epsilon}} = \underline{d} \underline{u}$$

$$\epsilon = \frac{du}{dx} \quad (d = \frac{d}{dx})$$

$$\epsilon = -y \frac{d^2 v}{dx^2} \quad (d = -y \frac{d^2}{dx^2})$$

Strain - nodal Displacement

$$\underline{\underline{\epsilon}} = \underline{d} \underline{u} = \underline{d} \underline{f} \underline{\delta} = \underline{B} \underline{\delta}$$

$$\epsilon = \frac{df}{dx} \underline{u} \quad (B = \frac{df}{dx} = [f_1' \ f_2'])$$

$$\epsilon = -y \frac{d^2 f}{dx^2} \underline{v} \quad (B = -y \frac{d^2 f}{dx^2} = -y [f_1'' \ f_2'' \ f_3'' \ f_4''])$$

Virtual strain

$$\delta \underline{\underline{\epsilon}} = \underline{B} \delta \underline{\delta}$$

stress

$$\underline{\sigma} = \underline{E} \underline{\varepsilon} = \underline{E} \underline{B} \underline{q}$$

$$\delta U = \int \delta \underline{\varepsilon}^T \underline{\sigma} dV$$

$$= \int \delta \underline{\varepsilon}^T \underline{B}^T \underline{E} \underline{B} \underline{q} dV$$

$$= \delta \underline{q}^T \underbrace{\left[\int \underline{B}^T \underline{E} \underline{B} dV \right]}_{\underline{k}} \underline{q} = \delta \underline{q}^T \underline{P} \quad (= \delta W)$$

$$\underline{k} = \int \underline{B}^T \underline{E} \underline{B} dV$$

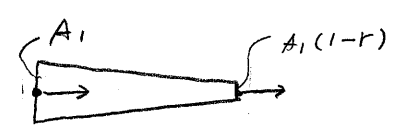
$$\underline{k} = \int \begin{bmatrix} f_1' \\ f_2' \end{bmatrix} [f_1' f_2'] E dA dx$$

$$= \int EA \begin{bmatrix} f_1' \\ f_2' \end{bmatrix} [f_1' f_2'] dx$$

$$\underline{k} = \int \begin{bmatrix} f_1'' \\ f_2'' \\ f_3'' \\ f_4'' \end{bmatrix} [f_1'' f_2'' f_3'' f_4''] \cdot E y^2 dA dx$$

$$= \int_0^L EI \begin{bmatrix} f_1'' \\ f_2'' \\ f_3'' \\ f_4'' \end{bmatrix} [f_1'' f_2'' f_3'' f_4''] dx$$

7.3 Nonuniform Elements



$$u = a_1 + a_2 x$$

$$u = (1 - \frac{x}{L}) u_1 + \frac{x}{L} u_2$$

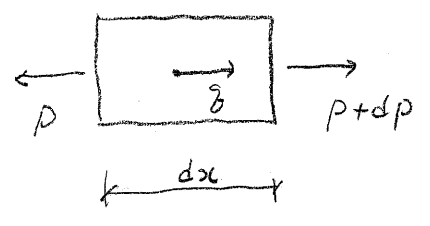
$$A = A_1 (1 - \frac{r x}{L})$$

if use $k_2 = \int EA N'{}^T N' dx$

$$= \int \begin{bmatrix} -\frac{1}{L} \\ \frac{1}{L} \end{bmatrix} \begin{bmatrix} -\frac{1}{L} & \frac{1}{L} \end{bmatrix} EA_1 (1 - \frac{r x}{L}) dx$$

$$= \frac{EA_1}{L} (1 - \frac{r}{2}) \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$A_{effective} = A_1 (1 - \frac{r}{2})$$



By equilibrium $-p + p + dp + \delta dx = 0$

$$\frac{dp}{dx} = -\delta$$

if $\delta = 0$, $\frac{dp}{dx} = 0$

$$\Rightarrow \frac{d}{dx} (A \delta x) = 0 \text{ or } \frac{d}{dx} (EA \epsilon_x) = 0$$

$$\Rightarrow \frac{d}{dx} (EA \frac{du}{dx}) = 0$$

$$EA \frac{du}{dx} = a_1 \quad \frac{du}{dx} = \frac{1}{EA} a_1 = \frac{a_1}{EA_1 (1 - \frac{r x}{L})}$$

$$u = \frac{a_1}{EA_1} \ln\left(1 - \frac{rx}{L}\right) \times \left(-\frac{L}{r}\right) + a_2$$

$$= \frac{a_1}{EA_1} \left(-\frac{L}{r}\right) \ln\left(1 - \frac{rx}{L}\right) + a_2 \Rightarrow \text{Exact displ. function satisfying the force-equilibrium}$$

$$x=0, \quad u = u_1 = a_2$$

$$x=L, \quad u = \frac{a_1}{EA_1} \left(-\frac{L}{r}\right) \ln(1-r) + u_1 = u_2$$

$$a_1 = (u_2 - u_1) \frac{EA_1}{\left(-\frac{L}{r}\right) \ln(1-r)}$$

$$u = \frac{\ln\left(1 - \frac{rx}{L}\right)}{\ln(1-r)} (u_2 - u_1) + u_1$$

$$= \underbrace{\left[1 - \frac{\ln\left(1 - \frac{rx}{L}\right)}{\ln(1-r)}\right]}_{N_1} u_1 + \underbrace{\left[\frac{\ln\left(1 - \frac{rx}{L}\right)}{\ln(1-r)}\right]}_{N_2} u_2$$

$$N_1' = \frac{+\left(\frac{r}{L}\right)}{\ln(1-r)} \frac{1}{\left(1 - \frac{rx}{L}\right)} = -N_2'$$

$$\underline{k} = \int EA_1 \frac{\left(1 - \frac{rx}{L}\right)}{\left(1 - \frac{rx}{L}\right)^2} - \frac{\left(\frac{r}{L}\right)^2}{\ln(1-r)^2} \begin{bmatrix} 1 \\ -1 \end{bmatrix} [1 \quad -1] dx$$

$$= \frac{EA_1}{L^2} \frac{r^2}{\ln(1-r)^2} \int \frac{1}{\left(1 - \frac{rx}{L}\right)} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} dx$$

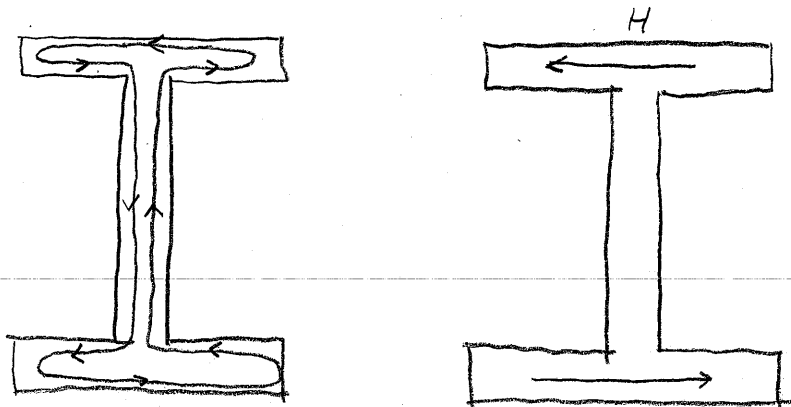
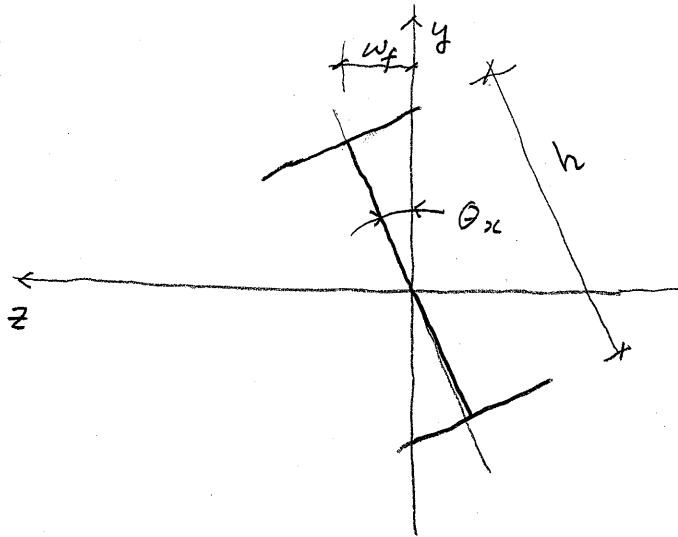
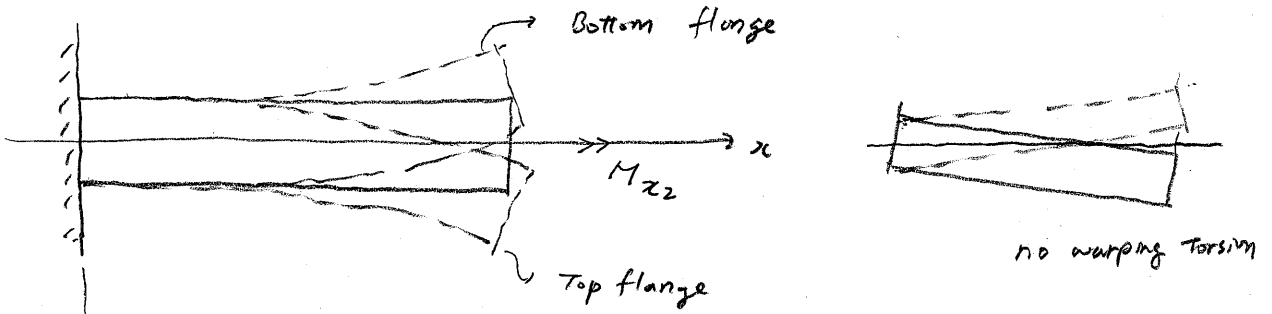
$$= \left[\frac{EA_1}{\ln(1-r)^2} \left(\frac{r}{L}\right)^2 \left[-\frac{1}{\left(\frac{r}{L}\right)} \ln\left(1 - \frac{rx}{L}\right) \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \right] \right]_0^L$$

$$= -\frac{EA_1}{L} \frac{r}{\ln(1-r)} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

7.4 Nonuniform Torsion

Torsional Resistance

- Pure Torsion - closed cross-section
- Warping Torsion - open cross-section



T_{sv}
Saint-Venant Torsion

H T_{wr}
Warping Torsion

St. Venant Torsion

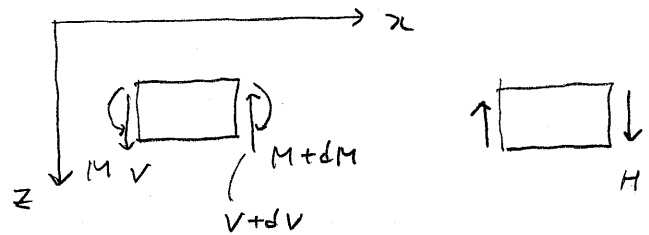
$$T_{sv} = GJ \theta_x' = GJ \frac{d\theta_x}{dx}$$

Warping Torsion

$$M_f = EI_f \frac{d^2 w_f}{dx^2} = E \frac{I_y h}{4} \left(\frac{d^2 \theta_x}{dx^2} \right)$$

$$w_f = \theta_x \frac{h}{2} \quad I_f = \frac{I_y}{2}$$

$$\frac{dM_f}{dx} = V = -H$$



$$T_{wr} = H \cdot h = - \frac{dM_f}{dx} \cdot h$$

$$= - E \frac{I_y h^2}{4} \theta_x'''$$

$$= - E C_w \theta_x'''$$

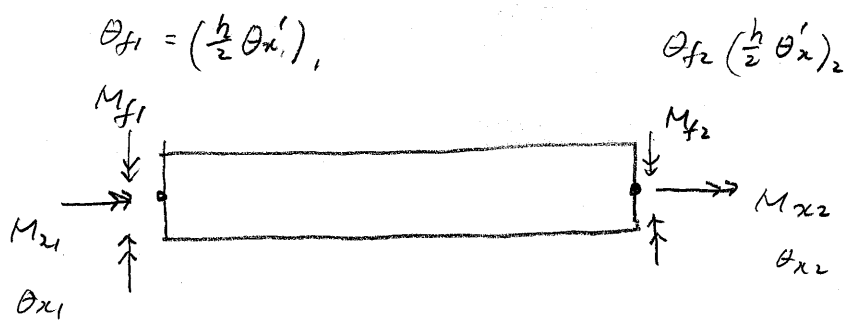
$$C_w = \frac{I_y h^2}{4} : \text{warping constant}$$

$$M_x = T_{sv} + T_{wr}$$

$$= GJ \theta_x' - E C_w \theta_x'''$$

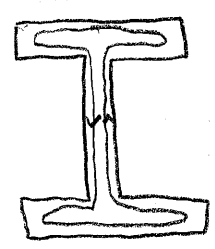
$$\text{if } m_x = 0 \Rightarrow \frac{dM_x}{dx} = 0 \Rightarrow \text{Equilibrium}$$

$$GJ \theta_x'' - E C_w \theta_x^{IV} = 0 \Rightarrow \text{Governing Equation}$$

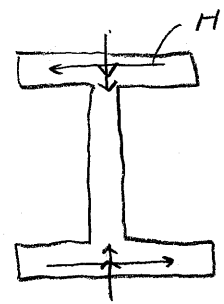


$$\theta_{f1} = \left(\frac{h}{2} \theta'_{x1}\right)_1$$

$$\theta_{f2} = \left(\frac{h}{2} \theta'_{x2}\right)_2$$



$$T_{sv}$$



$$M_f$$

$$T_{wp} = H \cdot h$$

$$\underline{P} = \begin{bmatrix} M_{x1} \\ M_{x2} \\ M_{f1} \\ M_{f2} \end{bmatrix}$$

$$M_{x1}, M_{x2} = \text{torque} (= T_{sv} + T_{wp})$$

$$M_{f1}, M_{f2} = \text{moment of flanges}$$

$$\theta_f = \frac{dM_f}{dx} = \frac{h}{2} \frac{d\theta_x}{dx} = \frac{h}{2} \theta'_x$$

$$\underline{\delta} = \begin{bmatrix} \theta_{x1} \\ \theta_{x2} \\ \left(\frac{h}{2} \theta'_{x1}\right) \\ \left(\frac{h}{2} \theta'_{x2}\right) \end{bmatrix}$$

$$\begin{aligned} \delta W_{ext} &= \underline{\delta}^T \cdot \underline{P} = \delta \theta_{x1} M_{x1} + \delta \theta_{x2} M_{x2} \\ &\quad + \delta \theta'_{x1} \underbrace{h M_{f1}}_{B_1} + \delta \theta'_{x2} \underbrace{h M_{f2}}_{B_2} \\ &\quad \Downarrow \\ &= 2 \delta \theta_f M_{f1} \end{aligned}$$

$$\delta W_{int} = \int \delta \theta_x' T_{sv} dx + 2 \int \delta \theta_f' M_f dx \quad \text{neglecting virtual work by shear force } H$$

$$T_{sv} = GJ \theta_x'$$

$$M_f = EC_w \left(\frac{1}{h} \right) \theta_x''$$

$$\theta_f' = \frac{h}{2} \theta_x''$$

$$\delta W_{int} = \int \delta \theta_x' GJ \theta_x' dx + \int \delta \theta_x'' EC_w \theta_x'' dx$$

Reset

$$\underline{P} = \begin{bmatrix} M_{x1} \\ M_{x2} \\ B_1 \\ B_2 \end{bmatrix}$$

$$\underline{q} = \begin{bmatrix} \theta_{x1} \\ \theta_{x2} \\ \theta_{x1}' \\ \theta_{x2}' \end{bmatrix}$$

$$\left(2 \int \delta \theta_f' M_f = \int \delta \theta_x'' \cdot \underline{B} \quad B_1 = h M_f \quad \theta_f' = \frac{h}{2} \theta_x'' \right)$$

$$\theta_x = f_1 \theta_{x1} + f_2 \theta_{x2} + f_3 \theta_{x1}' + f_4 \theta_{x2}'$$

$$= \underline{f} \underline{q}$$

$$\theta_x = a_1 + a_2 x + a_3 x^2 + a_4 x^3 \Rightarrow \langle \text{This function does not satisfy equilibrium} \rangle$$

$$\begin{cases} x=0 & \theta_x = \theta_{x1}, & \theta_x' = \theta_{x1}' \\ x=L & \theta_x = \theta_{x2}, & \theta_x' = \theta_{x2}' \end{cases}$$

for pure torsional action. >

$$\frac{dT_{sv}}{dx} = (m_x) = 0$$

$$\frac{d}{dx} (GJ \frac{d\theta_x}{dx}) = m_x$$

and

$$GJ \theta_x'' - EC_w \theta_x'' = 0$$

when $m_x = 0$

$$\Rightarrow f_1 = 1 - 3\left(\frac{x}{L}\right)^2 + 2\left(\frac{x}{L}\right)^3$$

$$f_2 = 3\left(\frac{x}{L}\right)^2 - 2\left(\frac{x}{L}\right)^3$$

$$f_3 = x \left(1 - \frac{x}{L}\right)^2$$

$$f_4 = x \left[\left(\frac{x}{L}\right)^2 - \frac{x}{L} \right]$$

$$\delta W_{ext} = \delta W_{int}$$

$$\int \delta \theta_x' GJ \theta_x' dx + \int \delta \theta_x'' ECW \theta_x'' dx = \delta \underline{\underline{\theta}}^T \underline{\underline{P}}$$

$$\delta \underline{\underline{\theta}}^T \left[\int \underline{\underline{f}}'^T GJ \underline{\underline{f}}' dx \right] \underline{\underline{\theta}} + \delta \underline{\underline{\theta}}^T \left[\int \underline{\underline{f}}''^T ECW \underline{\underline{f}}'' dx \right] \underline{\underline{\theta}} = \delta \underline{\underline{\theta}}^T \underline{\underline{P}}$$

$$\underline{\underline{k}} = \underbrace{\int \underline{\underline{f}}'^T GJ \underline{\underline{f}}' dx}_{\underline{\underline{k}}_{sv}} + \underbrace{\int \underline{\underline{f}}''^T ECW \underline{\underline{f}}'' dx}_{\underline{\underline{k}}_{wp}}$$

See eq. 7.28, 29, 30

$$\underline{\underline{k}}_{sv} = GJ \int_0^l \begin{bmatrix} f_1' \\ f_2' \\ f_3' \\ f_4' \end{bmatrix} [f_1' f_2' f_3' f_4'] dx$$

$$\underline{\underline{k}}_{wp} = ECW \int_0^l \begin{bmatrix} f_1'' \\ f_2'' \\ f_3'' \\ f_4'' \end{bmatrix} [f_1'' f_2'' f_3'' f_4''] dx$$

$$T_{sv} = GJ \theta_x'$$

$$T_{wp} = -ECW \theta_x'' \Rightarrow \text{423} \quad \theta_x = 3243 \Rightarrow \text{Approximation}$$

$$T_{sv} \text{ 2123} \quad T - T_{sv} = T_{wp} \text{ 1122 2123 2323}$$

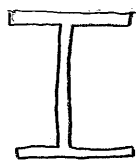
7.4.2 Applications and Examples

1. The relative magnitude of the warping restraint and St. Venant effects

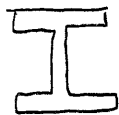
1) $\frac{C_w}{Jl^2} \Rightarrow$ warping torsion decreases with increase in beam length

$$k_{wp} \propto \frac{C_w}{l^3} \quad \text{vs} \quad k_{sv} \propto \frac{J}{l}$$

2)


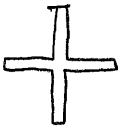


warping torsional rigidity is prevalent



St. Venant Torsional rigidity is prevalent

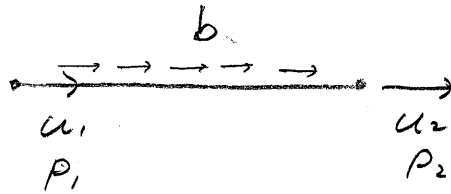
- 3) For warping torsion, boundary conditions for flanges & length play major role

4)  and  \Rightarrow warping resistance is negligible.

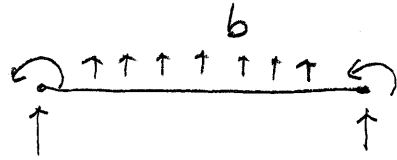
17.5 Loads between nodal points and initial strain effects

Body forces

Axial force member



flexural member



δW_b = virtual work due to virtual displacement on
body forces

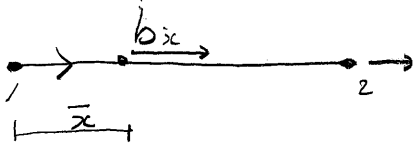
$$\begin{aligned}\delta W_b &= \int_0^l \delta \underline{u}^T \underline{b} \, dx \quad \left(= \int \delta \underline{u}^T \underline{b} \, dV \text{ in case of} \right. \\ &\quad \left. \text{body force per unit volume} \right) \\ &= \delta \underline{q}^T \int \underline{f}^T \underline{b} \, dx\end{aligned}$$

$$\delta U = \delta W$$

$$\delta \underline{q}^T \left[\int \underline{B}^T \underline{E} \underline{B} \, dV \right] \underline{q} = \delta \underline{q}^T \underline{P} + \delta \underline{q}^T \int \underline{f}^T \underline{b} \, dx$$

$$\begin{aligned}\left[\int \underline{B}^T \underline{E} \underline{B} \, dV \right] \underline{q} &= \underline{P} + \int \underline{f}^T \underline{b} \, dx \\ &= \underline{P} + \underline{P}_b\end{aligned}$$

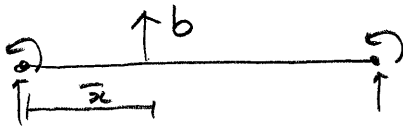
$$\begin{aligned}\underline{P}_b &= \text{equivalent nodal force due to body forces} \\ &= - \underline{FEP}\end{aligned}$$



$b_x =$ concentrated load
at $x = \bar{x}$

$$\delta W_b = \delta U_{x=\bar{x}}^T \cdot b_x = \delta \underline{q}^T \underline{f}_{x=\bar{x}}^T \cdot b_x$$

$$\underline{P}_b = \underline{f}_{x=\bar{x}}^T \cdot b_x = -\underline{F} \underline{e} \underline{F}$$



Initial Strain

$$\underline{\sigma} = E (\underline{\epsilon} - \underline{\epsilon}_0)$$

initial imperfection $\underline{\epsilon}_0 = \frac{\Delta_0}{L}$ for truss element

temperature change $\underline{\epsilon}_0 = \alpha \Delta T$

$$\delta U = \int \delta \underline{\epsilon}^T \underline{\sigma} dV$$

$$= \int \delta \underline{\epsilon}^T E (\underline{\epsilon} - \underline{\epsilon}_0) dV$$

$$= \int \delta \underline{\epsilon}^T E \underline{\epsilon} dV - \int \delta \underline{\epsilon}^T E \underline{\epsilon}_0 dV$$

$$(\underline{\epsilon} = \underline{B} \underline{\delta}, \quad \delta \underline{\epsilon} = \underline{B} \delta \underline{\delta})$$

$$= \delta \underline{\delta}^T \left[\int \underline{B}^T E \underline{B} dV \right] \underline{\delta} - \delta \underline{\delta}^T \left[\int \underline{B}^T E \underline{\epsilon}_0 dV \right]$$

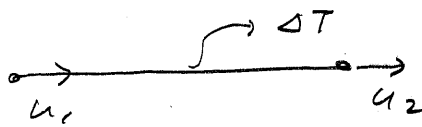
$$\delta U = \delta W$$

$$\left[\int \underline{B}^T E \underline{B} dV \right] \underline{\delta} = \underline{P} + \underline{P}_b + \underline{P}_0$$

$$\underline{P}_0 = \int \underline{B}^T E \underline{\epsilon}_0 dV = -\underline{FZF}$$

= equivalent nodal load due to initial strain

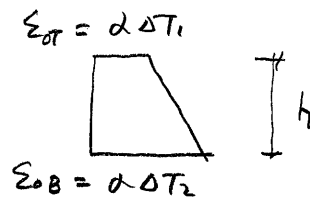
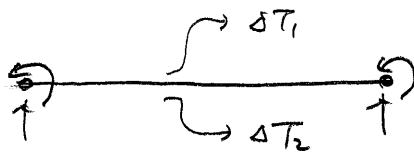
Truss element



$$\underline{\underline{\epsilon}}_0 = \alpha \Delta T$$

$$\begin{aligned} \underline{P}_0 &= \int \underline{B}^T \underline{E} \underline{\underline{\epsilon}}_0 dV = \int \underline{B}^T E \epsilon_0 A dx \\ &= \underline{B}^T EA \alpha \Delta T L \quad \underline{B} = \text{constant} \end{aligned}$$

Flexural element



$$\varphi_T = \frac{\alpha}{h} (\Delta T_2 - \Delta T_1)$$

$$\underline{\underline{\epsilon}}_T = \frac{\alpha}{h} (\Delta T_2 - \Delta T_1) (-y) = \underline{\underline{\epsilon}}_0$$

$$\begin{aligned} \underline{P}_0 &= \int \underline{B}^T \underline{E} \underline{\underline{\epsilon}}_0 dV = \iint \underline{B}^T E \underline{\underline{\epsilon}}_0 dA dx \\ &= \int (-y) \underline{f}''^T E \frac{\alpha}{h} (\Delta T_2 - \Delta T_1) (-y) dA dx \\ &= EI \int \underline{f}''^T \varphi_T dx \end{aligned}$$

7.6 Virtual Forces in the formulation of Element Force - Displacement Equations

7.6.1 Construction of Element Equations by the Principle of Virtual Forces

principle of virtual force is a basis of the direct formulation of element flexibility equations

the principle can be used to derive the element stiffness matrix.

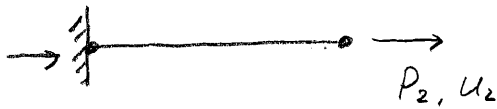
Since, the flexibility formulation is more straightforward, it is useful to derive the element stiffness (particularly for complex conditions

{ combined shear and bending deformation
tapered element
elements with curved or irregular axes

→ → → (o)

→ → → (x) inapplicable

Truss element



$$\begin{aligned}
 \delta U^* &= \int \delta \underline{\underline{\sigma}}^T \underline{\underline{\epsilon}} dV \\
 &= \int \delta \underline{\underline{\sigma}}^T \underline{\underline{E}}^{-1} \underline{\underline{\sigma}} dV \\
 &= \int \delta \underline{\underline{\sigma}}_x^T E^{-1} \underline{\underline{\sigma}}_x dV \\
 &= \int \delta F_x^T \cdot \frac{F_{2x}}{EA} dx
 \end{aligned}$$

$(F_{2x} = Q P_f)$ $Q =$ relationship between internal forces and nodal forces

$$= \delta \underline{\underline{P}}_f^T \left[\int \underline{\underline{Q}}^T \frac{1}{EA} \underline{\underline{Q}} dx \right] \underline{\underline{P}}_f$$

$$\delta W^* = \delta \underline{\underline{P}}_f^T \cdot \underline{\underline{q}}$$

$$\delta U^* = \delta W^*$$

$$\underbrace{\left[\int \underline{\underline{Q}}^T \frac{1}{EA} \underline{\underline{Q}} dx \right]}_{\underline{\underline{d}}} \underline{\underline{P}}_f = \underline{\underline{q}}$$

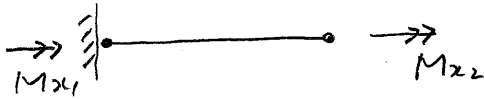
$$\underline{\underline{P}}_f = P_2 = F_{2x} \Rightarrow Q = 1$$

$$\underline{\underline{k}} = \begin{bmatrix} \underline{\underline{d}}^{-1} & \underline{\underline{d}}^{-1} \underline{\underline{\Phi}}^T \\ \underline{\underline{\Phi}} \underline{\underline{d}}^{-1} & \underline{\underline{\Phi}} \underline{\underline{d}} \underline{\underline{\Phi}}^T \end{bmatrix}$$

$$\underline{\underline{P}}_s = \underline{\underline{\Phi}} \underline{\underline{P}}_f$$

$\underline{\underline{\Phi}} =$ relationship between nodal forces

Torsional Element



$$\underline{P}_f = M_{x2} \quad \underline{P}_s = M_{x1}$$

$$\delta U^* = \int \delta \underline{\epsilon}^T \underline{\gamma} dV$$

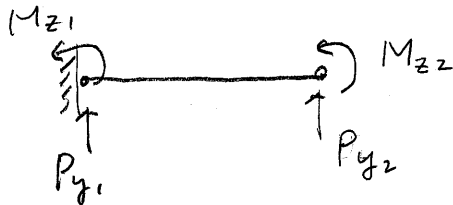
$$= \int \delta M_{x2} \frac{M_{x2}}{GJ} dx$$

$$M_{x2} = \underline{Q} \underline{P}_f = \underline{Q} M_{x1}$$

$$\underline{Q} = 1$$

$$= \delta \underline{P}_f^T \left[\underbrace{\int \underline{Q}^T \frac{1}{GJ} \underline{Q} dx}_{\underline{d}} \right] \underline{P}_f$$

flexural element



$$\underline{P}_f = \begin{bmatrix} P_{y2} \\ M_{z2} \end{bmatrix} \quad \underline{P}_s = \begin{bmatrix} P_{y1} \\ M_{z1} \end{bmatrix}$$

$$\delta U^* = \int \delta \underline{\sigma}^T \underline{\epsilon} dV$$

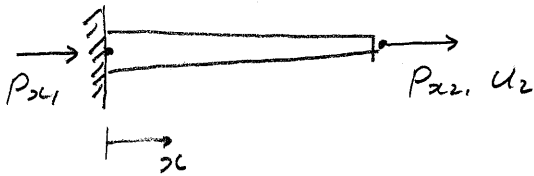
$$= \int \delta \sigma \cdot \frac{1}{E} \sigma dV$$

$$= \int \delta M_z \frac{M_z}{EI} dx$$

$$M_z = \underline{Q} \underline{P}_f$$

$$= \delta \underline{P}_f^T \left[\underbrace{\int \underline{Q}^T \frac{1}{EI} \underline{Q} dx}_{\underline{d}} \right] \underline{P}_f$$

Tapered axial member



$$A = A_1 \left(1 - \frac{rx}{L}\right)$$

$$\underline{P}_f = P_{x2}, \quad P_S = P_{x1}, \quad P_{x1} = -P_{x2} \Rightarrow \underline{\Phi} = -1$$

$$F_{xx} = P_{x2} \Rightarrow \underline{Q} = 1$$

$$\underline{d} = \int \underline{Q}^T \frac{1}{EA} \underline{Q} dx$$

$$= \int \frac{1}{EA} dx$$

$$= \frac{1}{EA_1} \int \frac{1}{\left(1 - \frac{rx}{L}\right)} dx$$

$$= -\frac{L}{rEA_1} \left[\ln\left(1 - \frac{rx}{L}\right) \right]_0^L$$

$$= -\frac{L}{rEA_1} \ln(1-r)$$

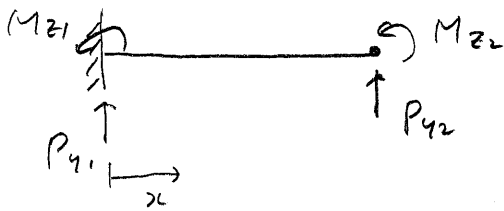
$$\underline{k} = \begin{bmatrix} \underline{d}^{-1} & \underline{d}^{-1} \underline{\Phi}^T \\ \underline{\Phi} \underline{d}^{-1} & \underline{\Phi} \underline{d}^{-1} \underline{\Phi}^T \end{bmatrix}$$

$$= -\frac{rEA_1}{L \ln(1-r)} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

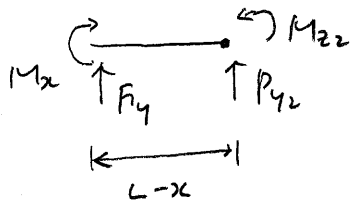
Why this exact solution can be obtained with ease.

- The virtual and real forces are given by the equation of statics. The variations of A , I , E do not affect these equations. in case of statically determinate structure
- whereas exact displacement functions can only be obtained through solution of differential equations with varying coefficients.

flexural element



$$\underline{P}_f = \begin{bmatrix} P_{y2} \\ M_{z2} \end{bmatrix}$$



$$M_x = M_{z2} + (L-x) P_{y2}$$

$$= \underbrace{[(L-x) \quad 1]}_{\underline{Q}} \begin{bmatrix} P_{y2} \\ M_{z2} \end{bmatrix}$$

$$\underline{d} = \int \underline{Q}^T \frac{1}{EI} \underline{Q} dx$$

$$= \int \begin{bmatrix} (L-x) \\ 1 \end{bmatrix} \frac{1}{EI} [(L-x) \quad 1] dx$$

$$= \frac{L}{EI} \begin{bmatrix} \frac{L^2}{3} & \frac{L}{2} \\ \frac{L}{2} & 1 \end{bmatrix}$$

$$\underline{P}_s = \begin{bmatrix} P_{y1} \\ M_{z1} \end{bmatrix}$$

$$P_{y1} = -P_{y2} \quad M_{z1} = -M_{z2} - L P_{y2}$$

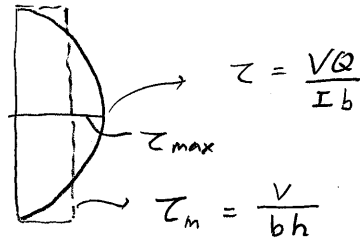
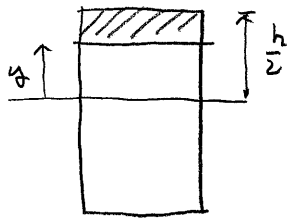
$$\begin{bmatrix} P_{y1} \\ M_{z1} \end{bmatrix} = \underbrace{\begin{bmatrix} -1 & 0 \\ -L & -1 \end{bmatrix}}_{\underline{\Phi}} \begin{bmatrix} P_{y2} \\ M_{z2} \end{bmatrix}$$

$$\underline{k} = \left[\begin{array}{c|c} \underline{d}^{-1} & \underline{d}^{-1} \underline{\Phi}^T \\ \hline \underline{\Phi} \underline{d}^{-1} & \underline{\Phi} \underline{d}^{-1} \underline{\Phi}^T \end{array} \right]$$

Element with Combined Mode

$$\delta U^* = \int \delta F_x \frac{F_x}{EA} dx + \int \delta M_z \frac{M_z}{EI} dx + \int \delta M_x \frac{M_x}{GJ} dx + \dots$$

Shear Area



$$\tau_{max} = 1.5 \tau_m$$

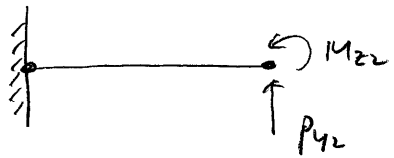
$$\int \tau z \gamma dA = dA \cdot \tau_m \cdot \gamma_m \quad \gamma_m = \frac{V}{G}$$

$$Q = b \cdot \left(\frac{h}{2} - y\right) \times \frac{1}{2} \left(\frac{h}{2} + y\right) = b \cdot \left(\frac{h^2}{4} - y^2\right)$$

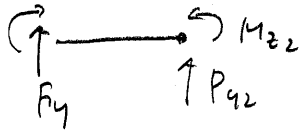
$$2 \int_0^{\frac{h}{2}} \frac{\delta V Q^2}{G I^2 b^2} \cdot V b dy = d(bh) \frac{\delta V}{d b h} \cdot \frac{V}{G b h d}$$

$$2 \frac{h}{I^2} \int Q^2 \cdot dy = \frac{1}{d}$$

Shearing Deformation of a Beam



$$F_y = -P_{y2} \quad Q = -1$$



$$z_{av} = \frac{F_y}{A_s} \quad \gamma = \frac{z_{av}}{G} = \frac{F_y}{GA_s}$$

$$\begin{aligned} \delta U^* &= \int \delta \underline{\sigma}^T \underline{\epsilon} dV \\ &= \int \delta \tau \cdot \gamma dV \\ &= \delta \underline{P}_f \int \underline{Q}^T \frac{1}{GA_s} \underline{Q} dx \underline{P}_f \\ &= \delta \underline{P}_{y2} \frac{L}{GA_s} P_{y2} P_{y2} \end{aligned}$$

$$\delta U^* = \delta U^*_{\text{bending}} + \delta U^*_{\text{shear}}$$

$$= [\delta P_{y2} \quad \delta M_{z2}] \underbrace{\begin{bmatrix} \frac{L^3}{3EI} + \frac{L}{A_s G} & \frac{L^2}{2EI} \\ \frac{L^2}{2EI} & \frac{L}{EI} \end{bmatrix}}_{\underline{d}} \begin{bmatrix} P_{y2} \\ M_{z2} \end{bmatrix}$$

$$\underline{k}_e = \begin{bmatrix} \underline{d}^{-1} & \underline{d}^{-1}\Phi^T \\ \Phi \underline{d}^{-1} & \Phi \underline{d}^{-1}\Phi^T \end{bmatrix}$$

$$\begin{bmatrix} P_{y1} \\ M_{z1} \end{bmatrix} = \underbrace{\begin{bmatrix} 1 & 0 \\ -L & -1 \end{bmatrix}}_{\Phi} \begin{bmatrix} P_{y2} \\ M_{z2} \end{bmatrix}$$

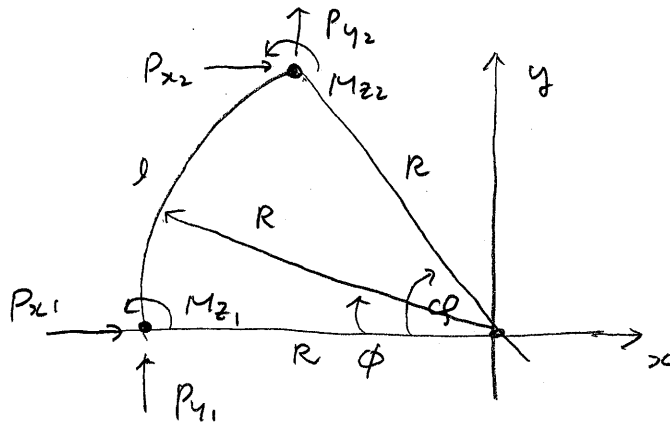
$$\begin{bmatrix} P_{y2} \\ M_{z2} \\ P_{y1} \\ M_{z1} \end{bmatrix} = \frac{EI}{L(\frac{L^2}{12} + \eta)} \underbrace{\begin{bmatrix} & -\frac{L}{2} & -1 & -\frac{L}{2} \\ & (\frac{L^2}{3} + \eta) & \frac{L}{2} & (\frac{L^2}{8} - \eta) \\ \text{Sym.} & & \frac{L}{2} & \\ & & & (\frac{L^2}{3} + \eta) \end{bmatrix}}_{\underline{k}_e} \begin{bmatrix} v_2 \\ \theta_{z2} \\ v_1 \\ \theta_{z1} \end{bmatrix}$$

$$\eta = \frac{EI}{A_s G}$$

Circular Ring Beam

Elementary flexure theory is used.

axial behavior, transverse shear deformation, and curved beam theory is disregarded.



$$\underline{P}_F = \begin{bmatrix} P_{x2} \\ P_{y2} \\ M_{z2} \end{bmatrix}$$

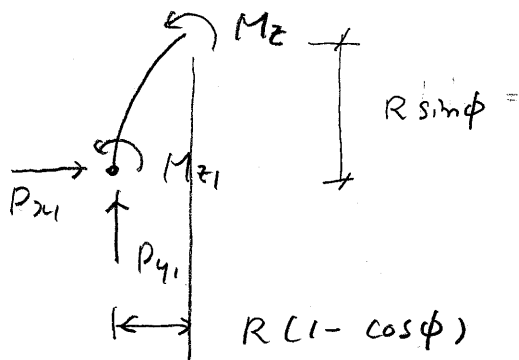
$$\underline{P}_S = \begin{bmatrix} P_{x1} \\ P_{y1} \\ M_{z1} \end{bmatrix}$$

$$\delta U^* = \int \delta \underline{\sigma}^T \underline{\epsilon} dV$$

$$= \int_0^l \delta M_z \frac{M_z}{EI} ds$$

$$(ds = R d\phi)$$

$$= \int_0^\phi \delta M_z \frac{M_z}{EI} R d\phi$$



$$M_z = -P_{x1} R \sin \phi + P_{y1} R (1 - \cos \phi) - M_{z1}$$

$$= \underbrace{\begin{bmatrix} -\sin \phi & (1 - \cos \phi) & -1 \end{bmatrix}}_{\underline{Q}} \begin{bmatrix} P_{x1} R \\ P_{y1} R \\ M_{z1} \end{bmatrix}$$

$$\delta U^* = \delta \underline{P}_f \left[\int_0^\varphi \underbrace{\underline{Q}^T \frac{1}{EI} \underline{Q}}_{\underline{d}} R d\phi \right] \underline{P}_f$$

$$\underline{d} = \int_0^\varphi \frac{R}{EI} \begin{bmatrix} -\sin \phi \\ (1 - \cos \phi) \\ -1 \end{bmatrix} [-\sin \phi \quad (1 - \cos \phi) \quad -1] d\phi$$

$$= E_s \quad 7.50$$

$$\underline{\delta}_f = \begin{bmatrix} u_1/R \\ v_1/R \\ Q_{z1} \end{bmatrix}$$

$$\underline{P}_s = \underline{\Phi} \underline{P}_f$$

$$\begin{bmatrix} P_{x2} R \\ P_{y2} R \\ M_{z2} \end{bmatrix} = \underbrace{\begin{bmatrix} -1 & 0 & 0 \\ 0 & -1 & 0 \\ -\sin \phi & (1 + \cos \phi) & -1 \end{bmatrix}}_{\underline{\Phi}} \begin{bmatrix} P_{x1} R \\ P_{y1} R \\ M_{z1} \end{bmatrix}$$

$\Rightarrow \underline{k}_R$ can be calculated with \underline{d} and $\underline{\Phi}$