

Announcement

- To be updated

Chap. 5 Stresses in Beams



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5.1 Introduction

5.2 Pure Bending and Nonuniform Bending

5.3 Curvature of Beam

5.4 Longitudinal Strains in Beams

5.5 Normal Stress in Beams

5.6 Design of Beams for Bending Stresses

5.7 Nonprismatic Beams

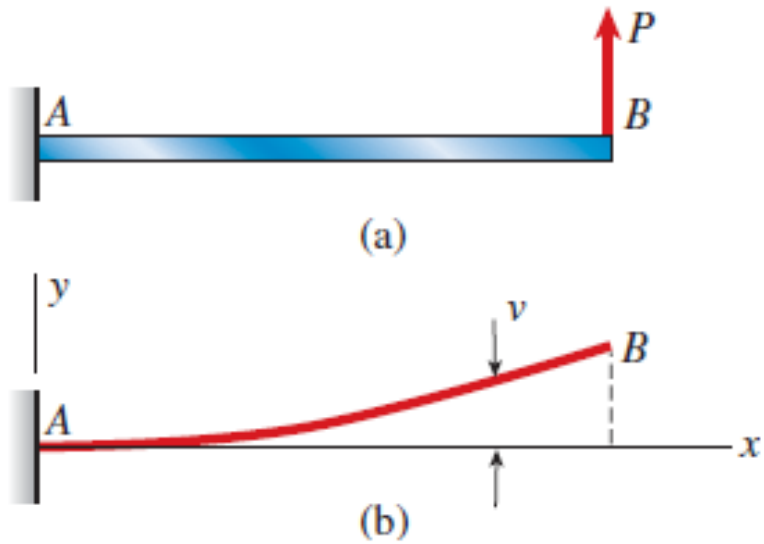
5.8 Shear Stresses in Beams of Rectangular Cross Section

5.9 Shear Stresses in Beams of Circular Cross Section

5.10 Shear Stresses in the Webs of Beams with Flanges

Introduction

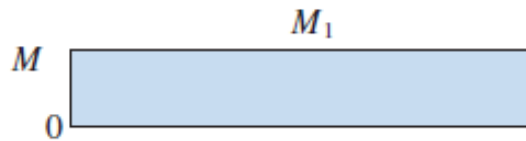
Bending of a cantilever beam: beam with load and resulting deflection curve



Pure Bending and Non-uniform Bending



(a)

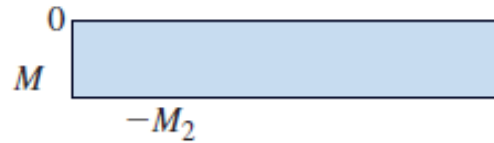


(b)

<Simple beam in pure bending>

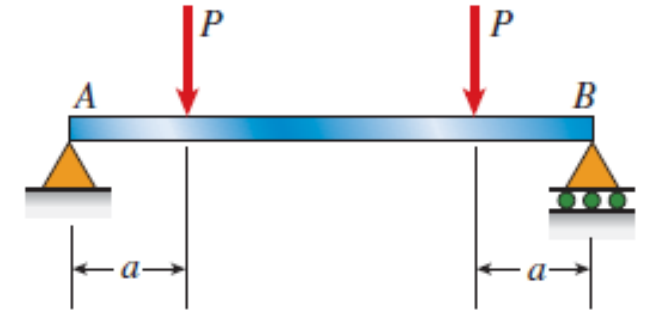


(a)



(b)

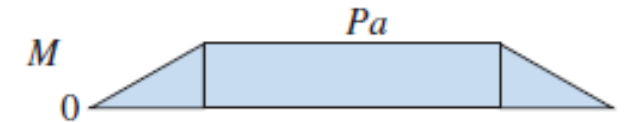
<Cantilever beam in pure bending>



(a)



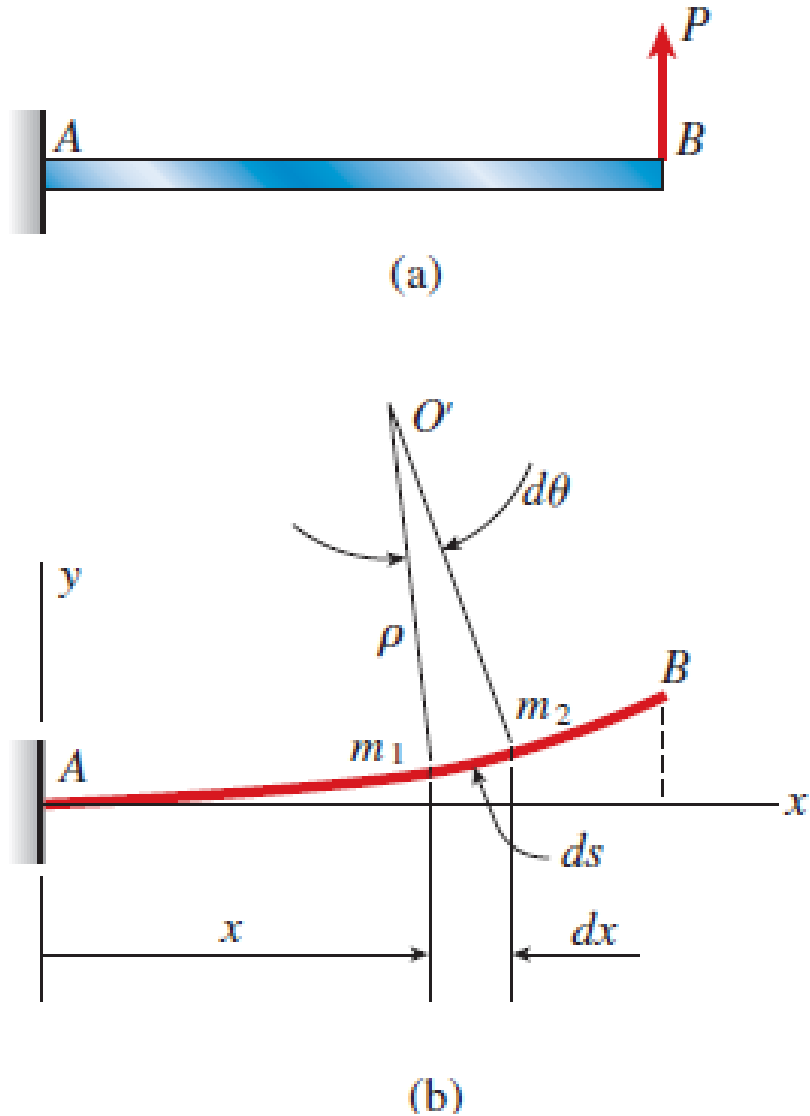
(b)



(c)

<Simple beam in non-uniform bending>

Curvature of a beam



$$\text{Radius of curvature} = \rho = m_1 O' = m_2 O'$$

$$\text{Reciprocal of the radius of curvature} = \kappa$$

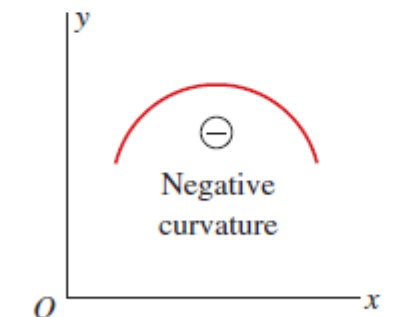
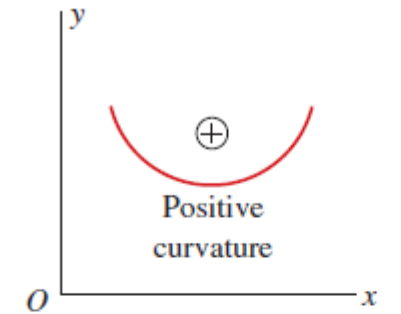
$$\kappa = \frac{1}{\rho}$$

$$\rho d\theta = ds$$

$$\kappa = \frac{1}{\rho} = \frac{d\theta}{ds}$$

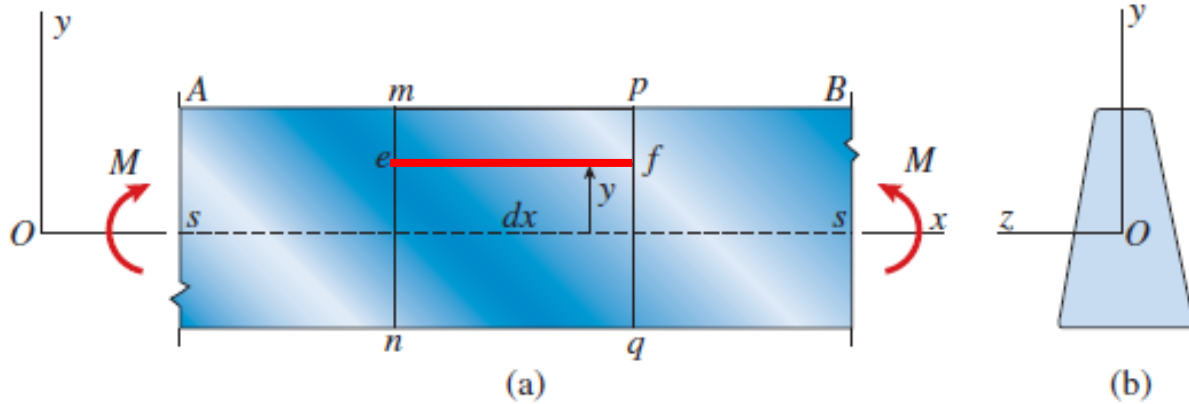
$$\text{Assumption: } ds = dx$$

$$\kappa = \frac{1}{\rho} = \frac{d\theta}{dx}$$



Sign convention for curvature

Longitudinal Strains in Beams



original length of line ef is dx ,

length L_1 of line ef after bending

$$L_1 = (\rho - y) d\theta = dx - \frac{y}{\rho} dx$$

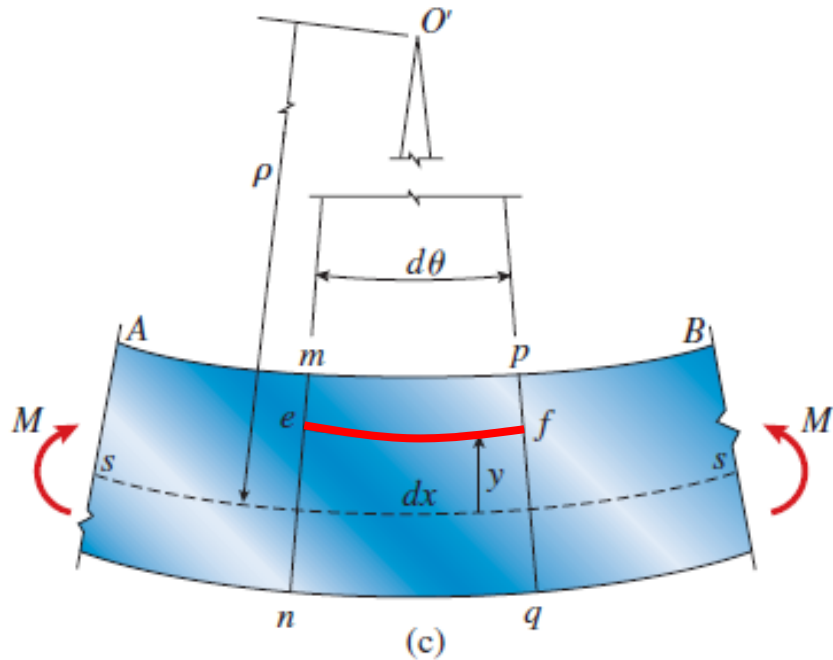
elongation

$$L_1 - dx, \text{ or } -y dx/\rho$$

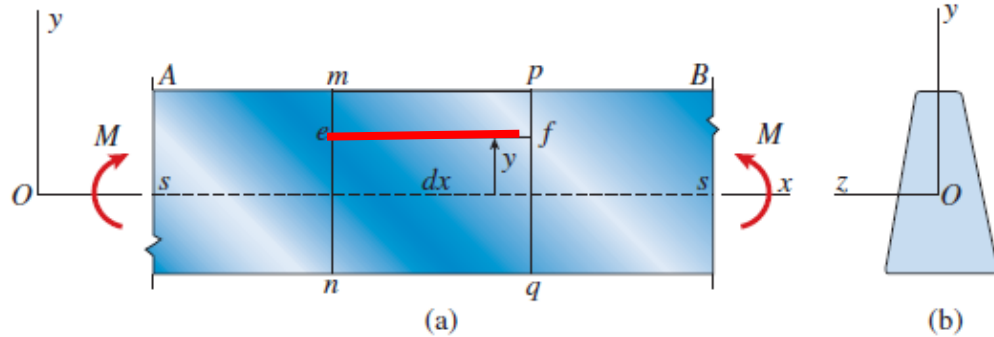
Strain (=elongation / original length of dx)

$$\epsilon_x = -\frac{y}{\rho} = -\kappa y$$

Strain-curvature relation



Longitudinal Strains in Beams

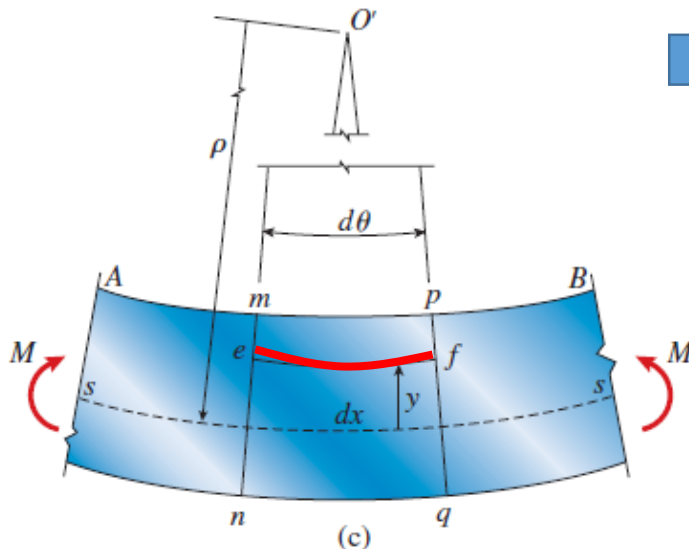


Strain

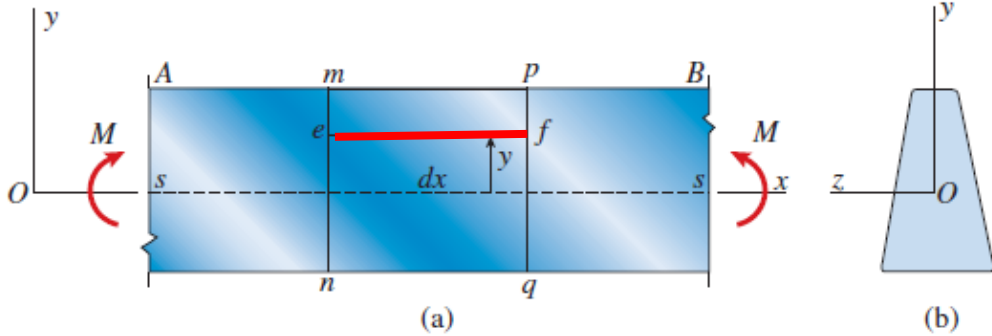
$$\epsilon_x = -\frac{y}{\rho} = -\kappa y$$

Strain-curvature relation

- Normal strains in a beam was derived solely from the geometry of the deformed beam.
- Properties of the material did NOT enter into the derivation.
- The strains in a beam in pure bending vary linearly with distance from the neutral surface regardless of the shape of the stress-strain curve of the material.

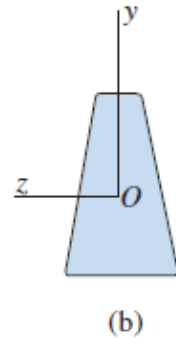


Normal Stresses in Beams (Linearly Elastic Materials)



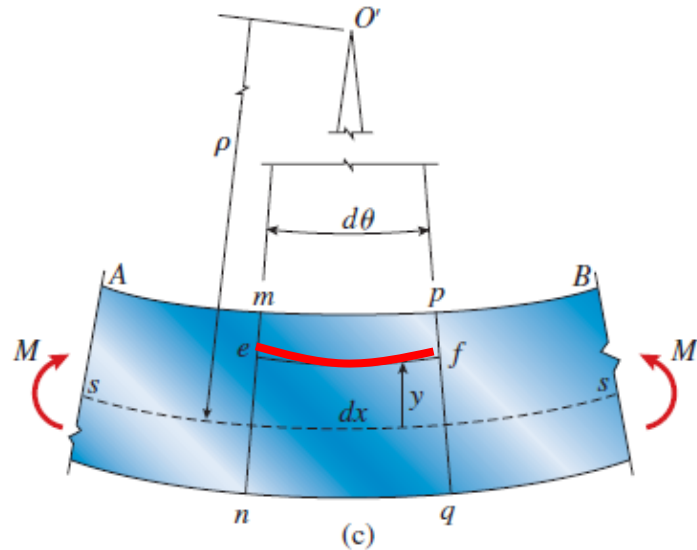
Strain in bending

$$\epsilon_x = -\frac{y}{\rho} = -\kappa y$$

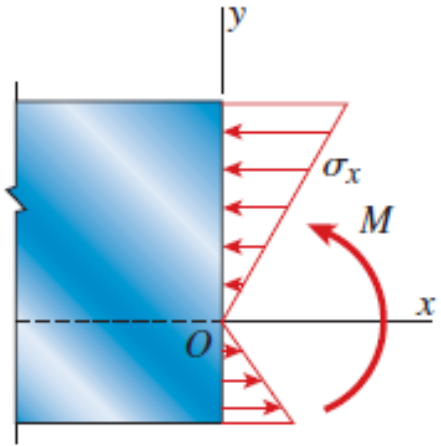


Linearly Elastic Materials

$$\sigma_x = E\epsilon_x$$



Normal Stresses in Beams (Linearly Elastic Materials)



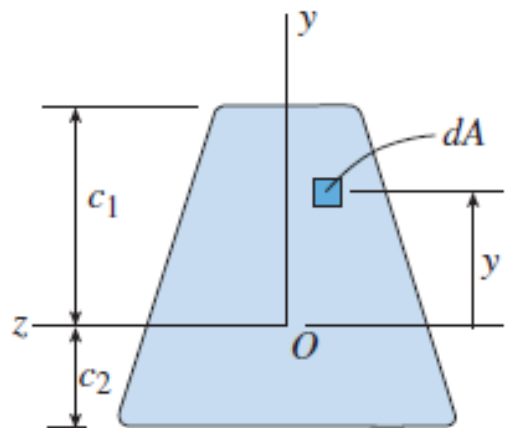
(a)

Strain in bending

$$\epsilon_x = -\frac{y}{\rho} = -\kappa y$$

Linearly Elastic Materials

$$\sigma_x = E\epsilon_x = -\frac{Ey}{\rho} = -E\kappa y$$



(b)

$$\sigma_x = E\epsilon_x$$

Normal Stresses in Beams (Linearly Elastic Materials)

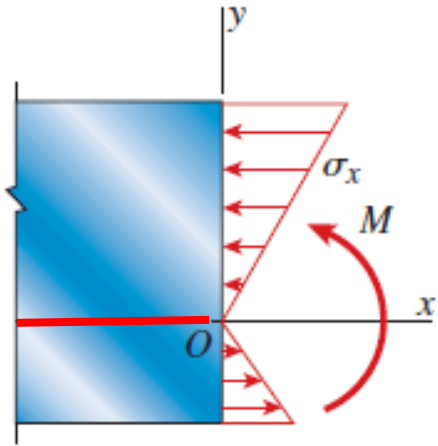
But... Where is origin axis?

$$\int_A \sigma_x dA = - \int_A E \kappa y dA = 0$$

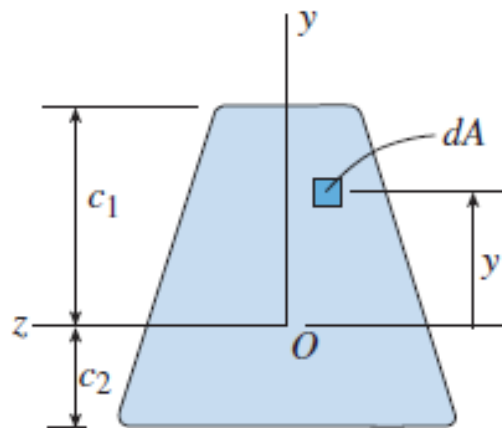


$$\int_A y dA = 0$$

Neutral axis



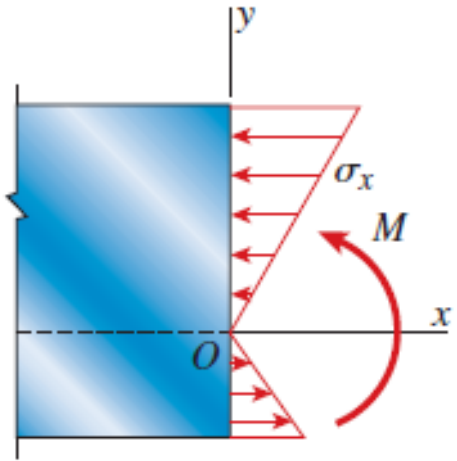
(a)



(b)

- Neutral axis passes through the centroid of the cross-sectional area when the material follows Hooke's law and there is no axial force acting on the cross section.

Moment-Curvature Relationship



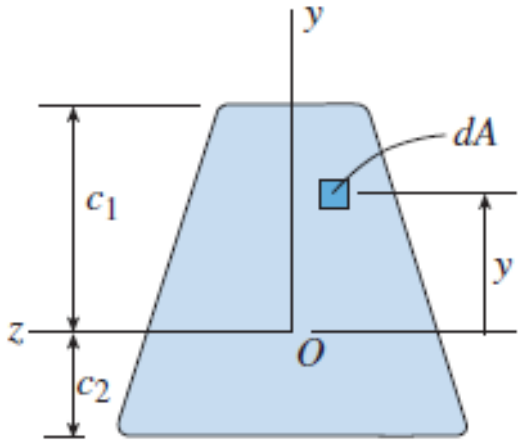
(a)

$$dM = -\sigma_x y dA$$

$$M = - \int_A \sigma_x y dA$$

$$M = \int_A \kappa E y^2 dA = \kappa E \int_A y^2 dA$$

$$\sigma_x = -E\kappa y$$



(b)

$$I = \int_A y^2 dA$$

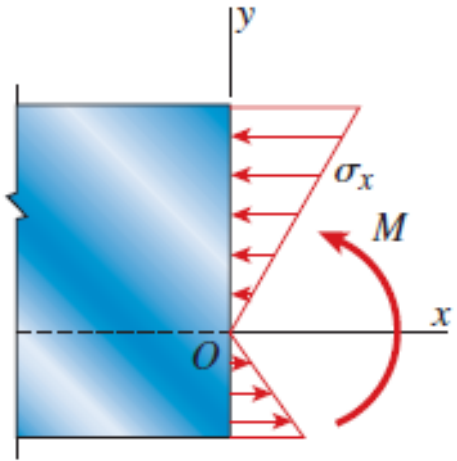
2nd moment of inertia

$$M = \kappa EI$$

$$\kappa = \frac{1}{\rho} = \frac{M}{EI}$$

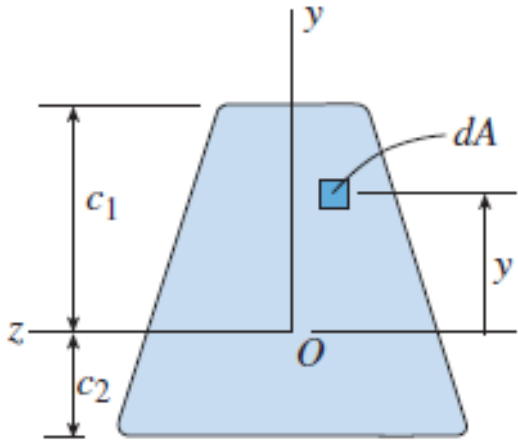
Curvature-Moment

Flexure Formula



(a)

$$\sigma_x = E\epsilon_x = -\frac{Ey}{\rho} = -E\kappa y$$



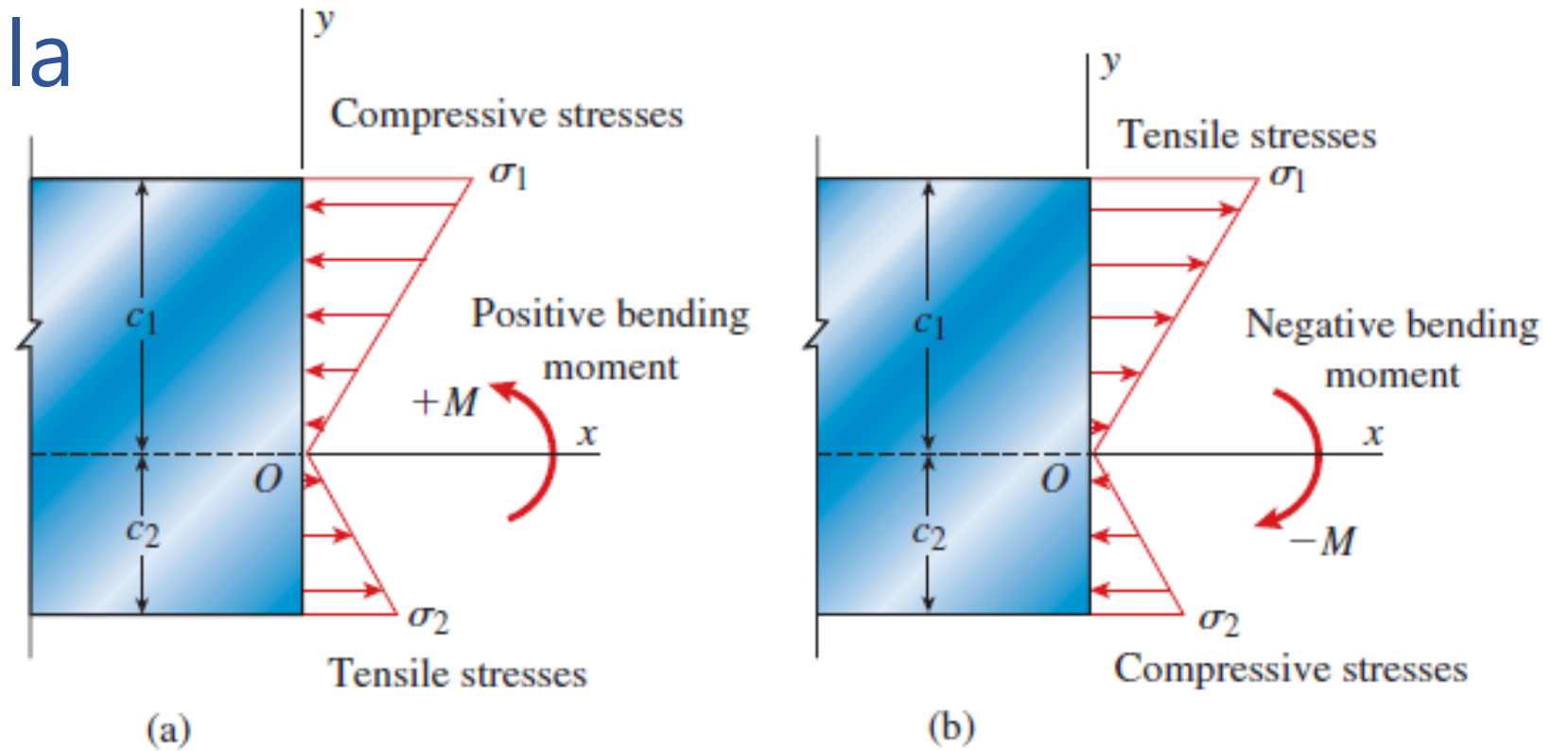
(b)

$$\kappa = \frac{1}{\rho} = \frac{M}{EI}$$

$$\sigma_x = -\frac{My}{I}$$

Flexure Formula

$$\sigma_x = -\frac{My}{I}$$



$$\sigma_1 = -\frac{Mc_1}{I} = -\frac{M}{S_1}$$

$$\sigma_2 = \frac{Mc_2}{I} = \frac{M}{S_2}$$

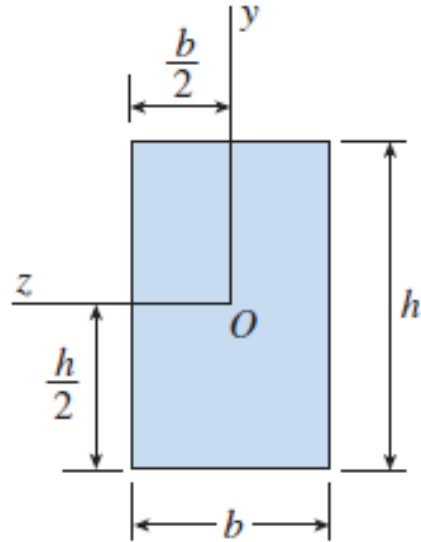
where $S_1 = \frac{I}{c_1}$

$$S_2 = \frac{I}{c_2}$$

Section moduli of the cross-sectional area

Moment of Inertia (I) and Section Modulus (S)

<Rectangular cross section>



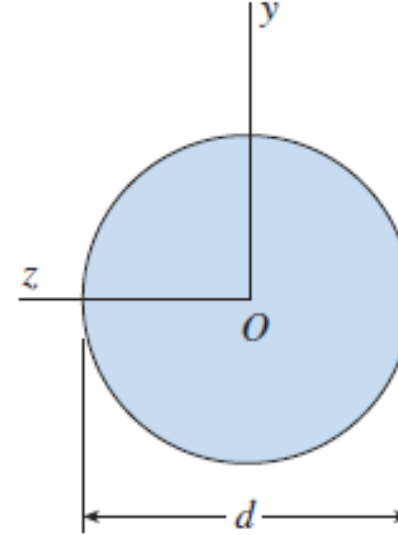
$$I = \int_A y^2 dA$$

$$I = \frac{bh^3}{12}$$

$$S = \frac{I}{c}$$

$$S = \frac{bh^2}{6}$$

<Circular cross section>



$$I = \frac{\pi d^4}{64}$$

$$S = \frac{\pi d^3}{32}$$

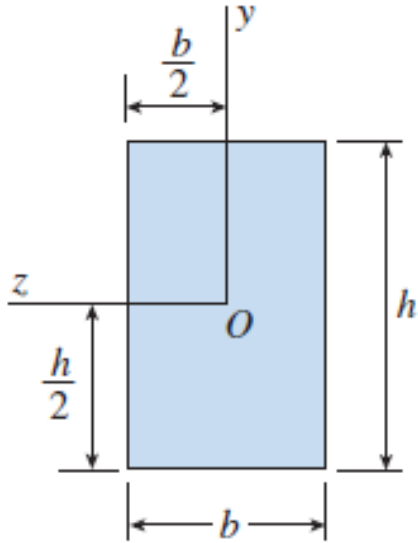
Polar moment of inertia

$$I_P = \int_A \rho^2 dA$$

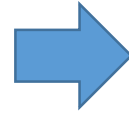
$$I_P = \frac{\pi r^4}{2} = \frac{\pi d^4}{32}$$

Moment of Inertia (I) derivation

<Rectangular cross section>



$$I = \int_A y^2 dA$$



$$I_x = \int_{-b/2}^{b/2} \int_{-h/2}^{h/2} y^2 dy dx$$

$$I_x = \int_{-b/2}^{b/2} \left. \frac{1}{3} y^3 \right|_{-h/2}^{h/2} dy dx$$

$$I_x = \int_{-b/2}^{b/2} \frac{1}{3} \frac{h^3}{4} dx$$

$$I_x = \left. \frac{1}{3} \frac{h^3}{4} x \right|_{-b/2}^{b/2}$$

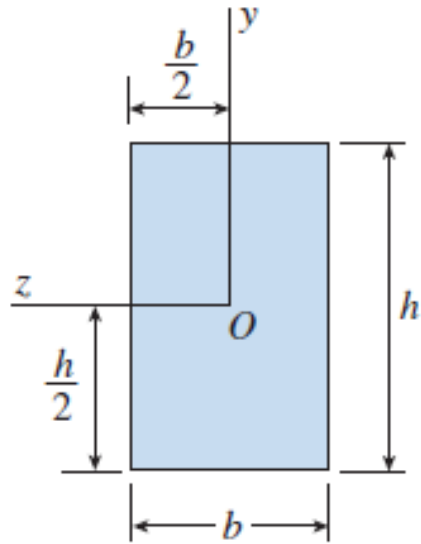
$$I_x = \frac{bh^3}{12}$$



$$I = \frac{bh^3}{12}$$

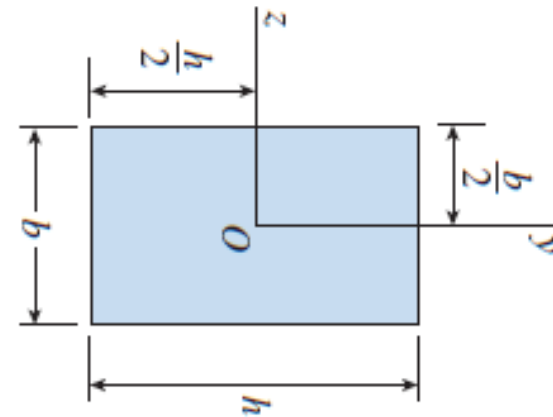
Moment of Inertia (I) derivation

<Rectangular cross section>



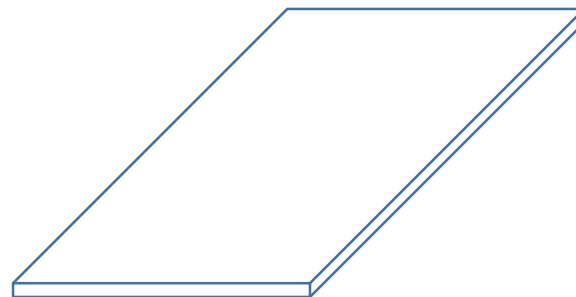
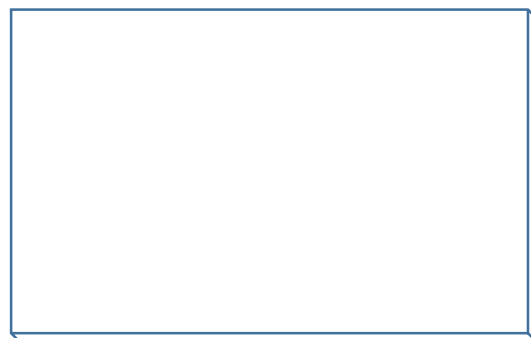
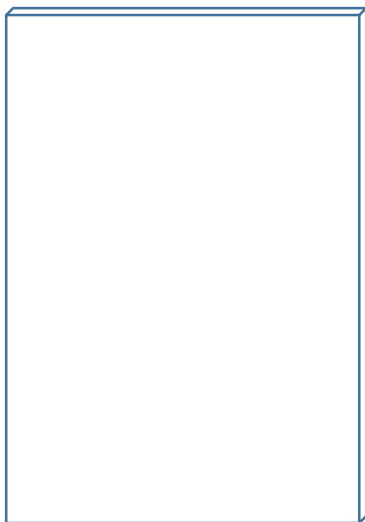
$$I = \frac{bh^3}{12}$$

<Rectangular cross section>



$$I = \frac{hb^3}{12}$$

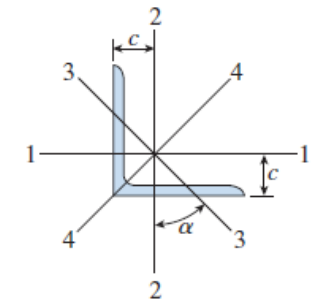
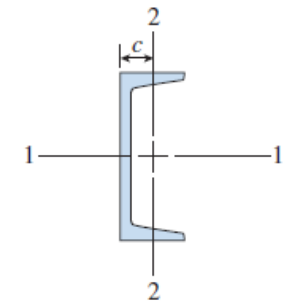
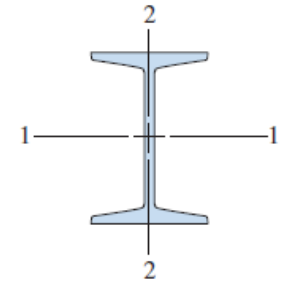
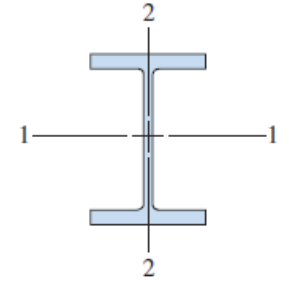
Paper folding



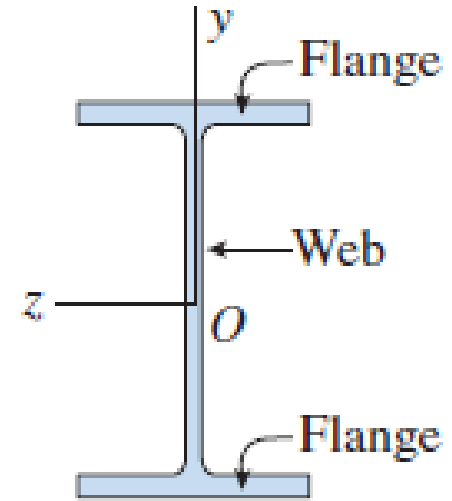
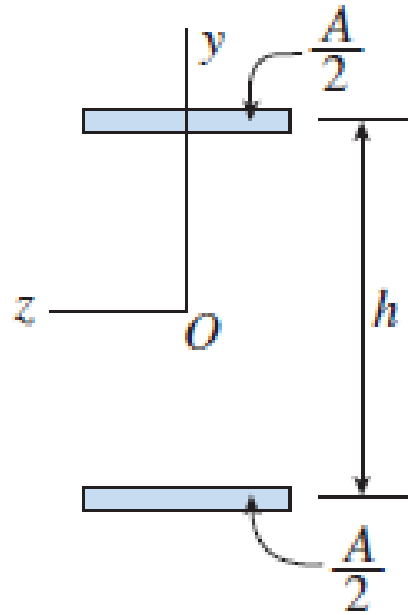
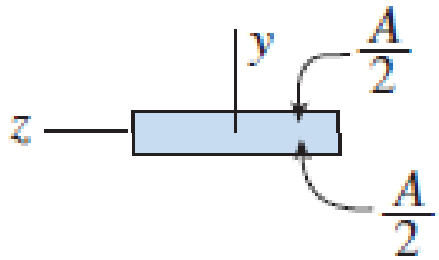
Engineering



APPENDIX E

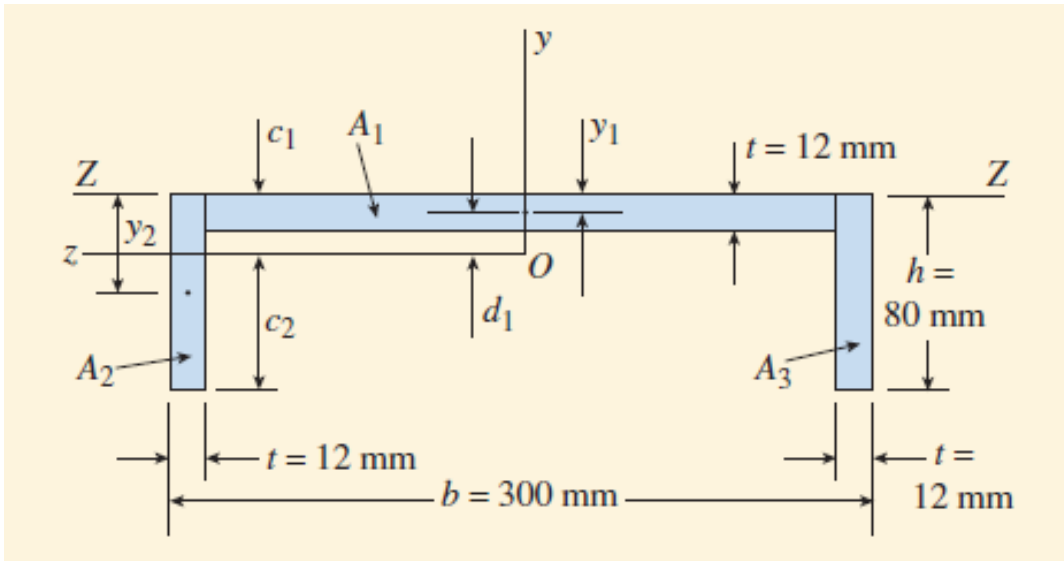


Customized section



Customized section

Where is the neutral axis?



Area 1: $y_1 = t/2 = 6 \text{ mm}$
 $A_1 = (b - 2t)(t) = (276 \text{ mm})(12 \text{ mm}) = 3312 \text{ mm}^2$

Area 2: $y_2 = h/2 = 40 \text{ mm}$
 $A_2 = ht = (80 \text{ mm})(12 \text{ mm}) = 960 \text{ mm}^2$

Area 3: $y_3 = y_2$ $A_3 = A_2$

$$c_1 = \frac{\sum y_i A_i}{\sum A_i} = \frac{y_1 A_1 + 2y_2 A_2}{A_1 + 2A_2}$$

$$= \frac{(6 \text{ mm})(3312 \text{ mm}^2) + 2(40 \text{ mm})(960 \text{ mm}^2)}{3312 \text{ mm}^2 + 2(960 \text{ mm}^2)} = 18.48 \text{ mm}$$

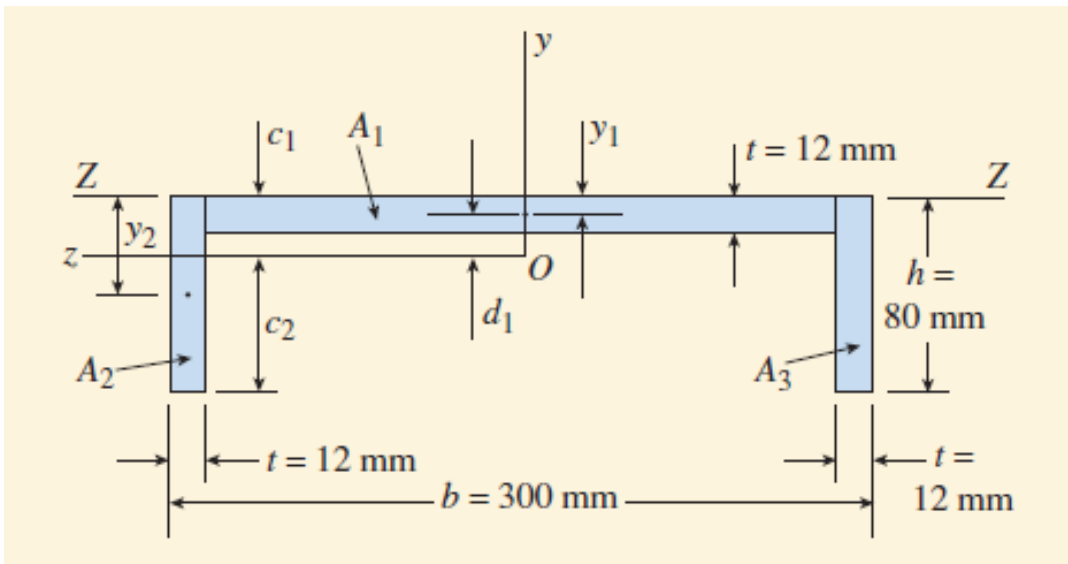
$$c_2 = h - c_1 = 80 \text{ mm} - 18.48 \text{ mm} = 61.52 \text{ mm}$$

Customized section

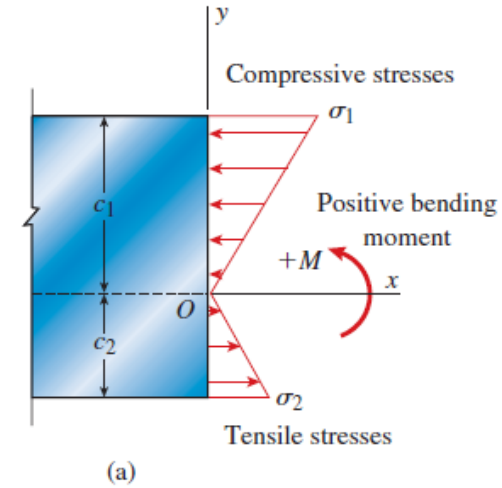
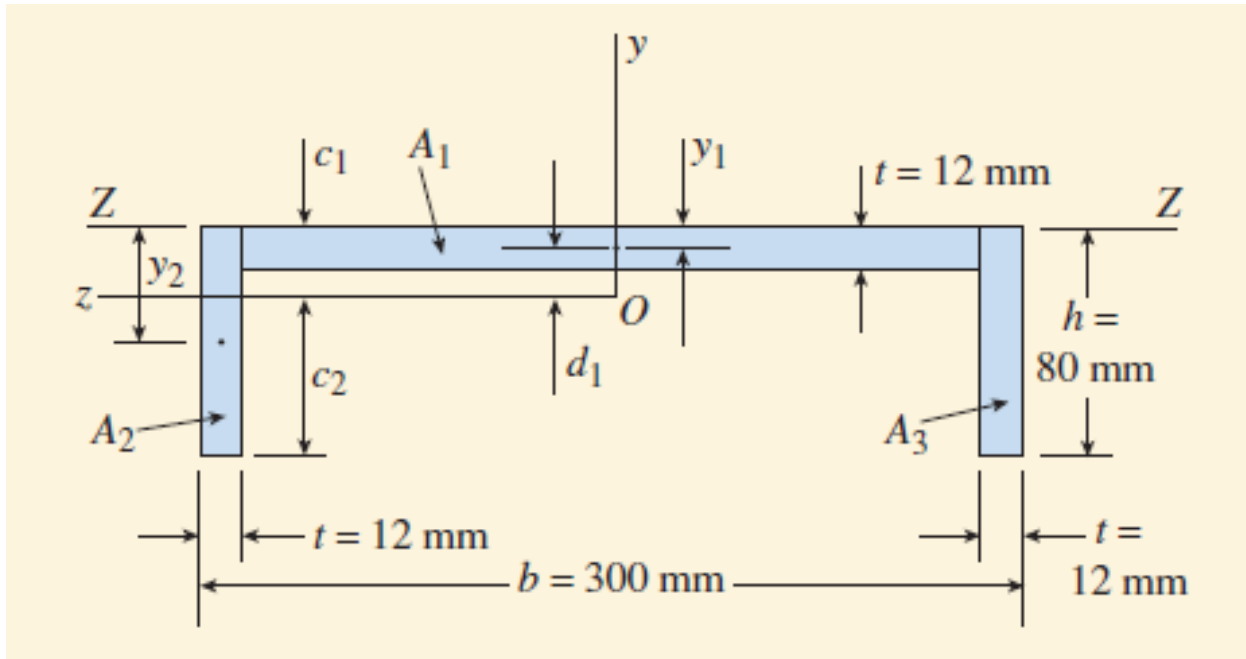
Moment of Inertia

$$(I_z)_1 = (I_c)_1 + A_1 d_1^2$$

(if neutral axis is different from the centroid of each component)



Customized section



$$\sigma_1 = -\frac{Mc_1}{I}$$

$$\sigma_2 = \frac{Mc_2}{I}$$