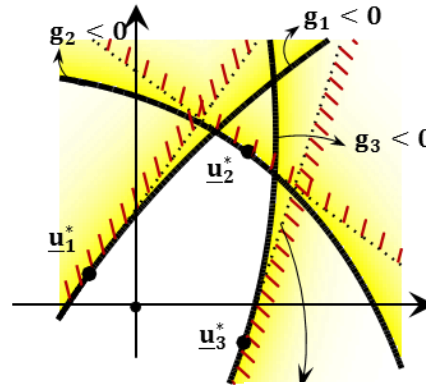


**457.646 Topics in Structural Reliability**  
**In-Class Material: Class 17**

© FORM approximation (Hohenbichler & Rackwitz 1983)

① Series system

$$\begin{aligned}
 P(E_{sys}) &= P\left(\bigcup_{i=1}^m E_i\right) \\
 &= P\left(\bigcup_{i=1}^m g_i(\mathbf{x}) \leq 0\right) \\
 &\stackrel{FORM}{\cong} P\left(\bigcup_{i=1}^m \dots \leq 0\right)
 \end{aligned}$$



$n \rightarrow$  # rv's  
 $m \rightarrow$  # comp's

Let  $Z_i = \hat{\alpha}_i \mathbf{u}$ ,  $i = 1, \dots, m$

$$E[Z_i] =$$

$$Var[Z_i] = \|\dots\|^2 =$$

$$\begin{aligned}
 G_i(\mathbf{u}) &\approx G_i(\mathbf{u}_i^*) + \nabla G_i(\mathbf{u}_i^*)(\mathbf{u} - \mathbf{u}_i^*) \\
 &= \nabla G_i(\mathbf{u}_i^*)(\mathbf{u} - \mathbf{u}_i^*) \leq 0
 \end{aligned}$$

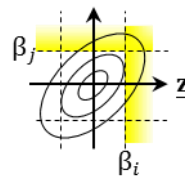
$$\Leftrightarrow \dots \leq 0$$

Therefore,  $Z_i \sim (\dots, \dots)$

$$\begin{aligned}
 \rho_{Z_i, Z_j} &= \frac{[ \dots, \dots ]}{\dots} = \frac{E[ \dots ] - E[ \dots ] \cdot E[ \dots ]}{\dots} \\
 &= E[ \dots \cdot \dots^T ] = E[ \dots ] = \frac{E[\mathbf{u}\mathbf{u}^T]}{\dots} =
 \end{aligned}$$

$\therefore \mathbf{Z} \sim (\dots, \dots)$ ,  $\rho_{Z_i, Z_j} =$

$$\begin{aligned}
 \therefore P(E_{sys}) &\stackrel{FORM}{\cong} P\left(\bigcup_{i=1}^m \dots \leq 0\right) \\
 &= 1 - P\left(\bigcap_{i=1}^m \dots \leq \dots\right)
 \end{aligned}$$



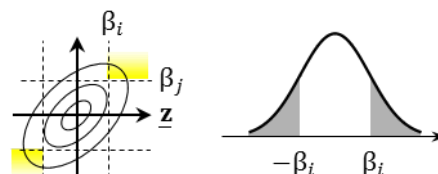
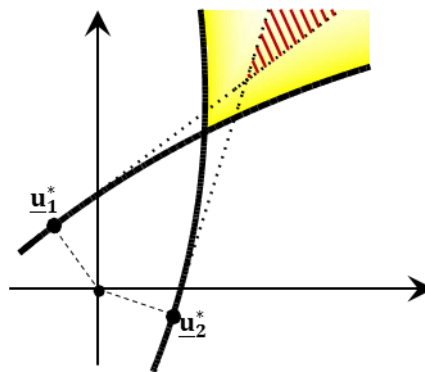
$$= 1 - \Phi_m(\dots, \dots; \mathbf{R})$$

Joint normal CDF of  $\mathbf{Z} \sim N(\mathbf{0}; \mathbf{R})$

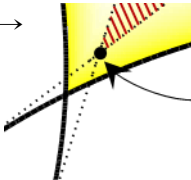
Where  $\Phi_m(\boldsymbol{\beta}; \mathbf{R}) = \int_{-\infty}^{\beta_1} \dots \int_{-\infty}^{\beta_m} \phi_m(\mathbf{Z}; \mathbf{R}) d\mathbf{z}$

② Parallel system

$$\begin{aligned}
 P(E_{sys}) &= P\left(\bigcap_{i=1}^m E_i\right) \\
 &= P\left(\bigcap_{i=1}^m g_i(\mathbf{x}) \leq 0\right) \\
 &\stackrel{FORM}{\cong} P\left(\bigcap_{i=1}^m \left( \frac{g_i(\mathbf{x})}{\sigma_{g_i}} \leq -\beta_i \right)\right) \\
 &= P\left(\bigcap_{i=1}^m \left( \frac{g_i(\mathbf{x})}{\sigma_{g_i}} \geq \beta_i \right)\right) \\
 &\stackrel{sym}{=} P\left(\bigcap_{i=1}^m \left( \frac{g_i(\mathbf{x})}{\sigma_{g_i}} \leq -\beta_i \right)\right) \\
 &= \Phi_m\left( \beta_1, \dots, \beta_m; \mathbf{R} \right)
 \end{aligned}$$

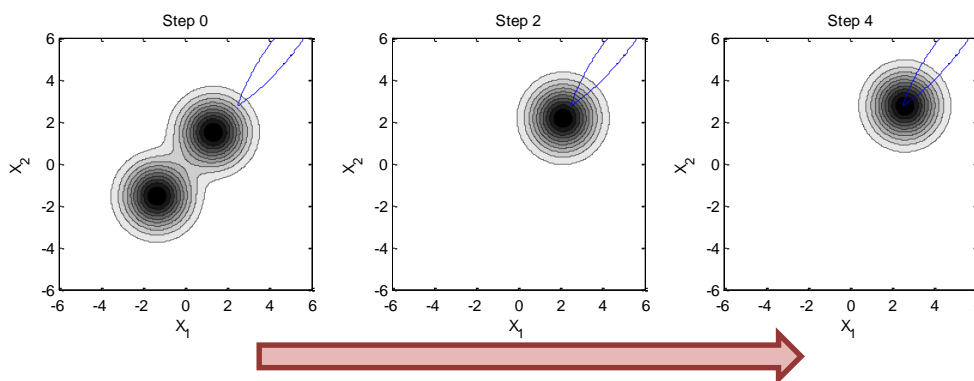


→ may have huge errors due to curvatures

→  better linearization point?  
 “joint design point”  
 Hard to find or may not exist

**Note:** One could find such important domain using an adaptive sampling technique

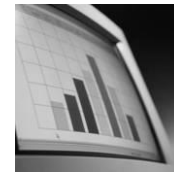
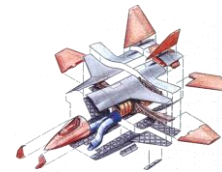
Kurtz, N., and J. Song (2013). Cross-entropy-based adaptive importance sampling using Gaussian mixture. *Structural Safety*. Vol. 42, 35-44.



③ General system?

⇒ No direct FORM approximation

# Risk-quantification of Complex Systems by Matrix-based System Reliability Method



**Junho Song**

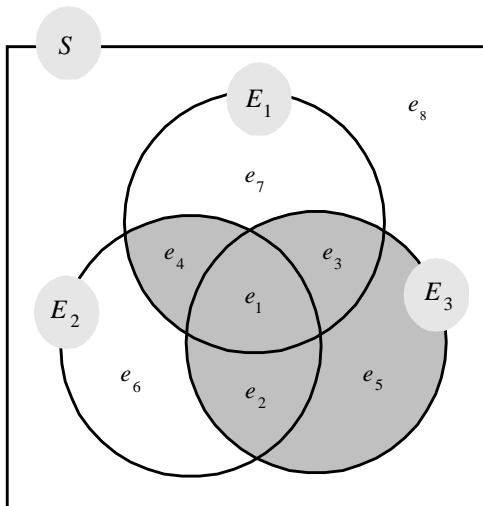
**Department of Civil and Environmental Engineering  
Seoul National University**

# Matrix-based Formulation

- Matrix-based formulation of system failure:

$$P(E_{sys}) = \mathbf{c}^T \mathbf{p}$$

\* Example:  $P(E_1 E_2 \cup E_3) = p_1 + p_2 + p_3 + p_4 + p_5$   
 $= [1 \quad 1 \quad 1 \quad 1 \quad 1 \quad 0 \quad 0 \quad 0] \cdot$   
 $[p_1 \quad p_2 \quad p_3 \quad p_4 \quad p_5 \quad p_6 \quad p_7 \quad p_8]^T$



- c**: "event" vector  
~ describes the system event of interest
- p**: "probability" vector  
~ likelihood of component joint failures

# Identification of event vector, $\mathbf{c}$

- Matrix-based event operations:

$$\mathbf{c}^{\bar{E}} = \mathbf{1} - \mathbf{c}^E$$

$$\mathbf{c}^{E_1 \cdots E_n} = \mathbf{c}^{E_1} .* \mathbf{c}^{E_2} .* \dots .* \mathbf{c}^{E_n}$$

$$\mathbf{c}^{E_1 \cup \dots \cup E_n} = \mathbf{1} - (\mathbf{1} - \mathbf{c}^{E_1}) .* (\mathbf{1} - \mathbf{c}^{E_2}) .* \dots .* (\mathbf{1} - \mathbf{c}^{E_n})$$

- Efficient and easy to implement by matrix-based computing languages, e.g. Matlab®, Octave
- Can construct directly from event vectors of components and other system events
- Can develop/use problem-specific algorithms to identify event vectors

# Identification of event vector, $\mathbf{c}$

- Event vectors for component events:

$$\mathbf{C}_{[1]} = \begin{bmatrix} \mathbf{1} \\ \mathbf{0} \end{bmatrix} \quad \mathbf{C}_{[i]} = \begin{bmatrix} \mathbf{C}_{[i-1]} & \mathbf{1} \\ \mathbf{C}_{[i-1]} & \mathbf{0} \end{bmatrix} \quad \text{for } i = 2, \dots, n$$

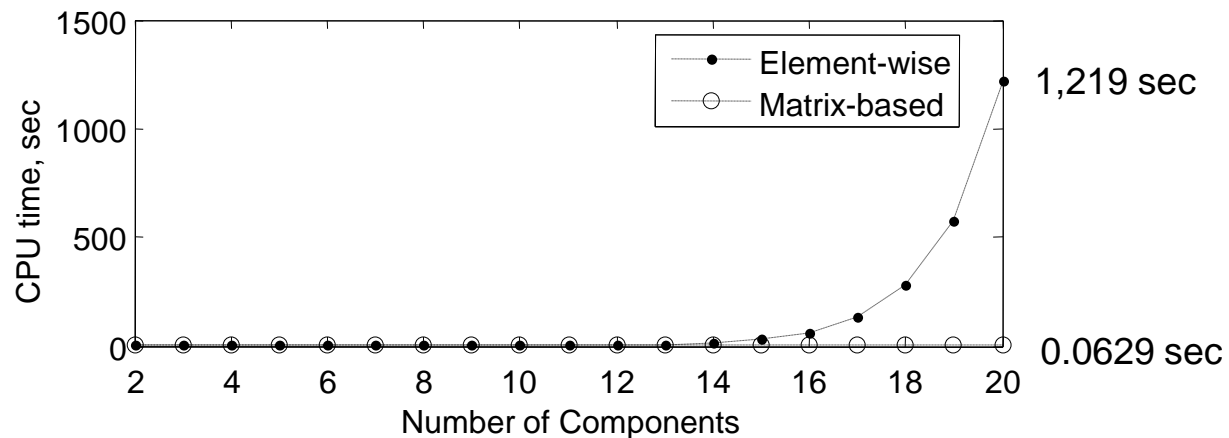
- $\mathbf{0}$  and  $\mathbf{1}$  denote the column vectors of  $2^{(i-1)}$  zeros and ones
- After  $\mathbf{C}_{[n]}$  is constructed, the  $i$ -th column of the matrix is the event vector of the  $i$ -th component event.

# Computation of probability vector, $\mathbf{p}$

- Iterative matrix-based procedure for statistically independent (s.i.) components

$$\mathbf{p}_{[1]} = [P_1 \quad 1 - P_1]^T$$

$$\mathbf{p}_{[i]} = \begin{bmatrix} \mathbf{p}_{[i-1]} \cdot P_i \\ \mathbf{p}_{[i-1]} \cdot (1 - P_i) \end{bmatrix} \quad \text{for } i = 2, \dots, n$$



# Statistical dependence b/w components

- By total probability theorem,

$$\begin{aligned} P(E_{sys}) &= \int_{\mathbf{s}} P(E_{sys} | \mathbf{s}) f_{\mathbf{s}}(\mathbf{s}) d\mathbf{s} \\ &= \int_{\mathbf{s}} \mathbf{c}^T \mathbf{p}(\mathbf{s}) f_{\mathbf{s}}(\mathbf{s}) d\mathbf{s} \\ &= \mathbf{c}^T \tilde{\mathbf{p}} \end{aligned}$$

- Utilize **conditional s.i.** of components given an outcome of random variables **S** causing component dependence e.g. Earthquake magnitude for a bridge system
- Event vector **c** is independent of this consideration ~ no need to construct the probability vector for new system events



# “What if not explicitly identified?”

- Example: approximation by Dunnett-Sobel (DS) correlation matrix (1955)

$$Z_i \sim N(\mathbf{0}, \mathbf{R}), \quad \rho_{ij} = r_i \cdot r_j$$

$$Z_i = \sqrt{1 - r_i^2} \cdot U_i + r_i S,$$

- $Z_i, i=1, \dots, n$  are conditional s.i. given  $S=s$
- Fit the given correlation matrix with a DS correlation matrix with the least square error
- Generalized DS model (Song and Kang, Structural Safety)

$$Z_i \sim N(\mathbf{0}, \mathbf{R}), \quad \rho_{ij} = \sum_{k=1}^m (r_{ik} r_{jk})$$

$$Z_i = \sqrt{1 - \sum_{k=1}^m r_{ik}^2} \cdot U_i + \sum_{k=1}^m (r_{ik} S_k)$$

# Conditional prob./importance measure

- Conditional probability Importance Measure (CIM)

$$CIM_i = P(E_i | E_{sys}) = \frac{P(E_i E_{sys})}{P(E_{sys})}$$

- Fussell-Vesely IM

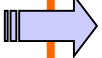
$$FV_i = \frac{P(\bigcup_{k:C_k \supseteq E_i} C_k)}{P(E_{sys})}$$

- $P(E_{sys}')/P(E_{sys}) = (\mathbf{c}'^T \mathbf{p}) / (\mathbf{c}^T \mathbf{p})$
- Once the system reliability is done, only additional task is to find the event vector for a new system event

# Parameter sensitivity of system reliability

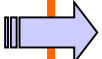
- Statistically independent components

$$P(E_{sys}) = \mathbf{c}^T \mathbf{p}$$


$$\frac{\partial P_{sys}}{\partial \theta} = \mathbf{c}^T \frac{\partial \mathbf{p}}{\partial \theta}$$

- Statistically dependent components

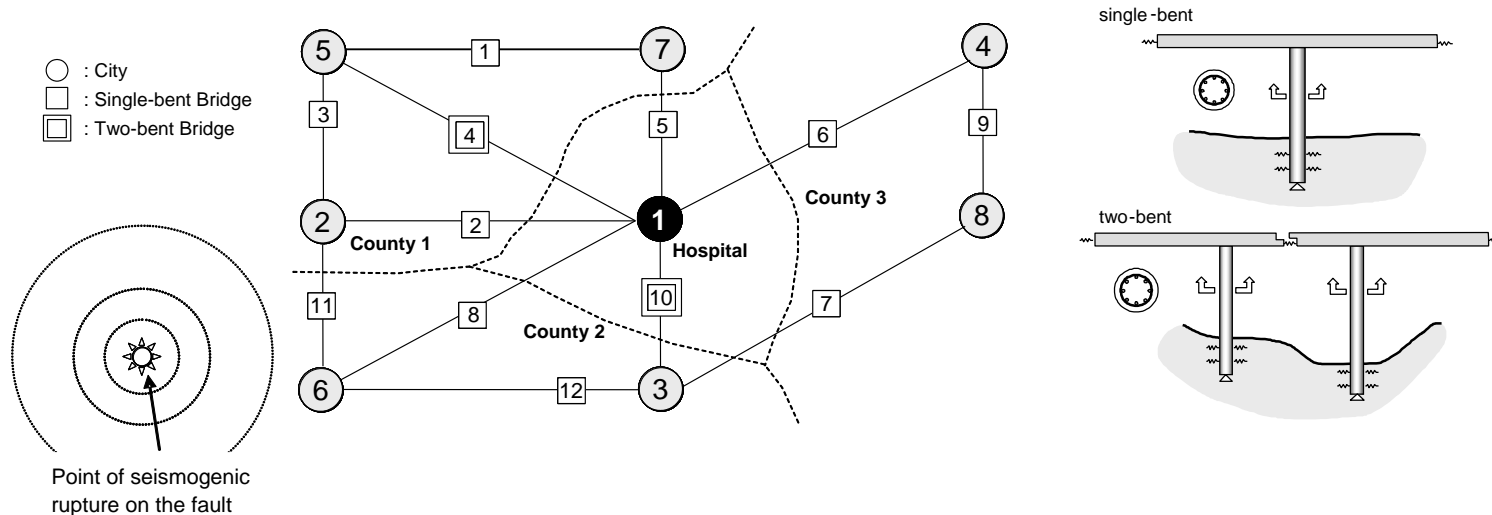
$$P(E_{sys}) = \int_{\mathbf{s}} \mathbf{c}^T \mathbf{p}(\mathbf{s}) f_{\mathbf{s}}(\mathbf{s}) d\mathbf{s}$$


$$\frac{\partial P_{sys}}{\partial \theta} = \int_{\mathbf{s}} \mathbf{c}^T \left[ \frac{\partial \mathbf{p}(\mathbf{s})}{\partial \theta} f_{\mathbf{s}}(\mathbf{s}) + \mathbf{p}(\mathbf{s}) \frac{\partial f_{\mathbf{s}}(\mathbf{s})}{\partial \theta} \right] d\mathbf{s}$$

Zero unless  $\theta$  is a parameter related to common source.

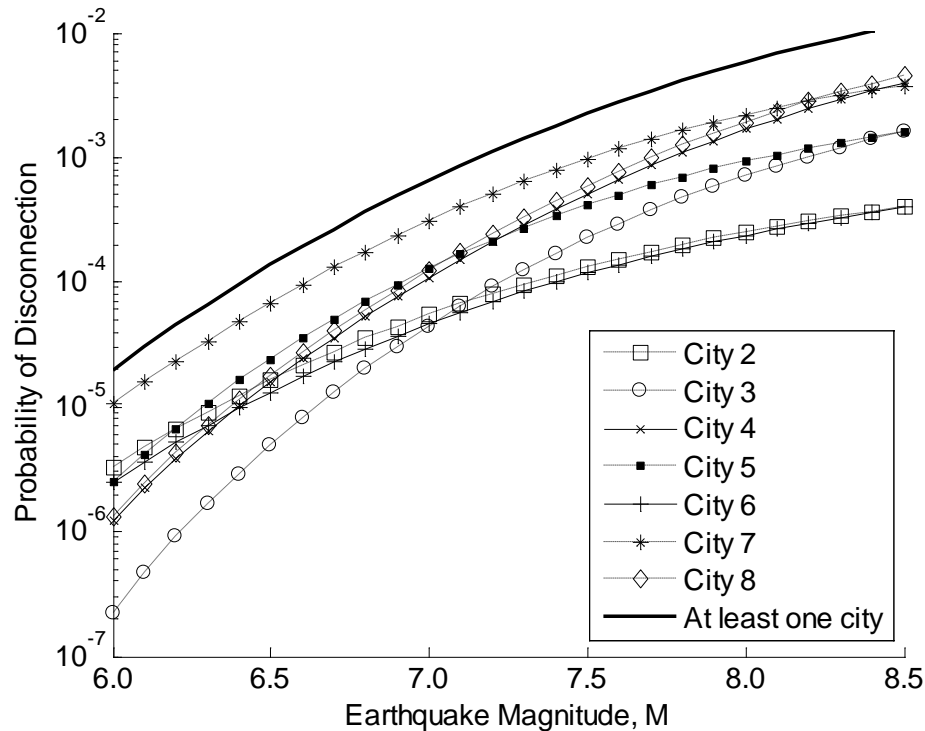
# Appl. I: Connectivity of a transportation network

\* Kang, W.-H., J. Song, and P. Gardoni (2008) "Matrix-based system reliability method and applications to bridge networks," *Reliability Engineering & System Safety*, Vol. 93, 1584-1593.



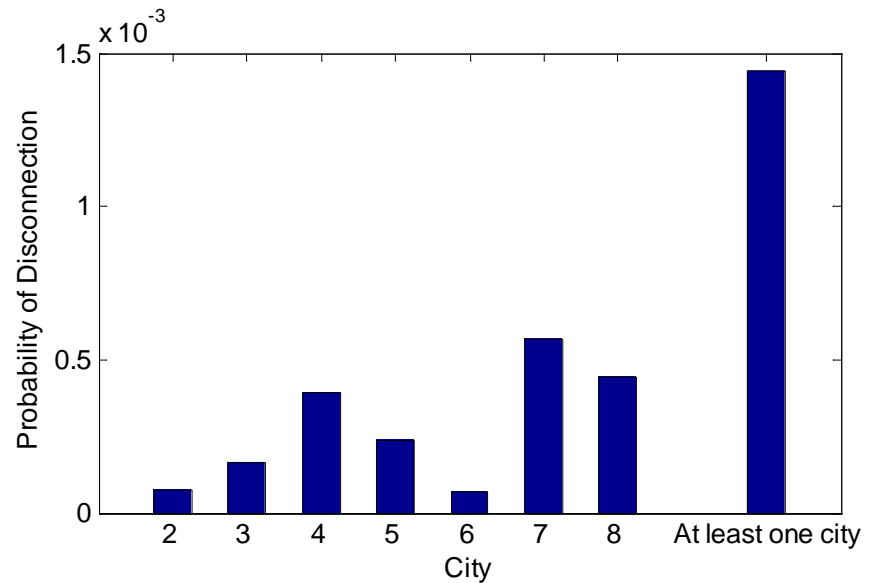
- Post-earthquake disconnection from the critical facility
- Fragilities for bridges (Gardoni et al. 2003)
- Deterministic attenuation relationship used
- For given magnitude, the bridge component failures are conditional s.i.

# Connectivity of a transportation network



$$P(E_{sys} | M = m) = \mathbf{c}^T \mathbf{p}(m)$$

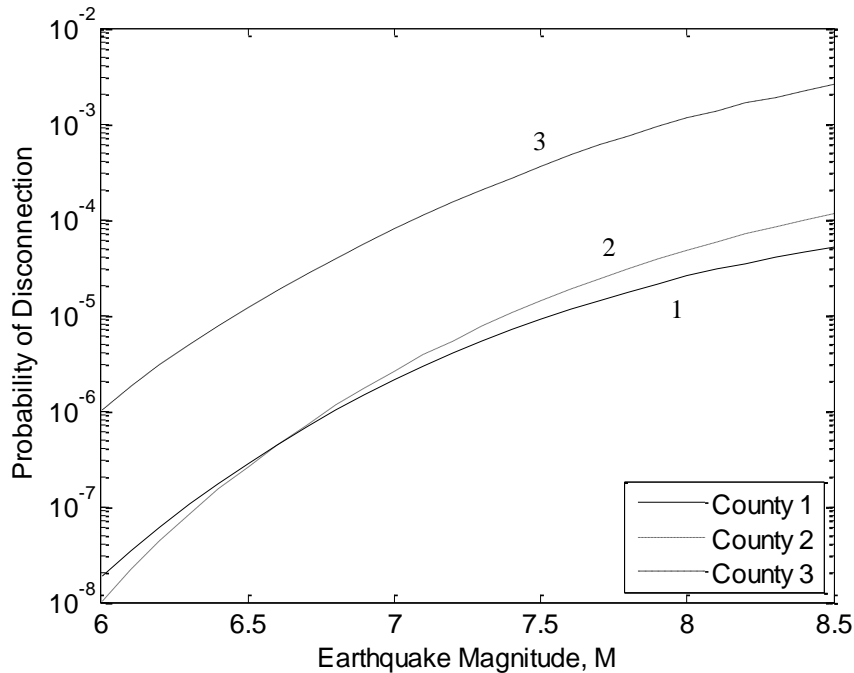
Conditional probability of disconnection of cities



$$P(E_{sys}) = \int_{m_0}^{m_c} \mathbf{c}^T \mathbf{p}(m) f_M(m) dm = \mathbf{c}^T \tilde{\mathbf{p}}$$

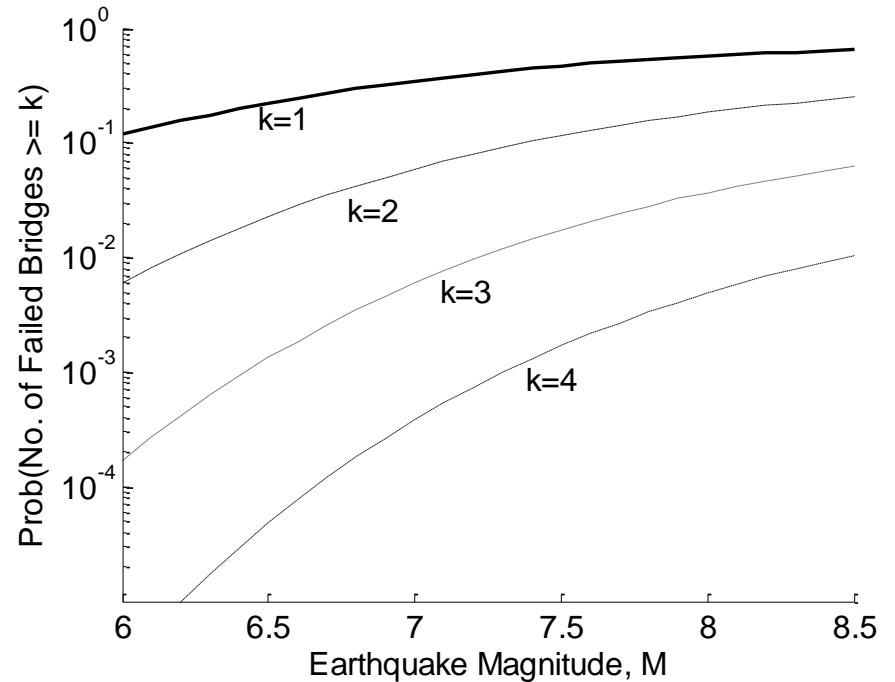
Probability of disconnection of cities

# Connectivity of a transportation network



$$P(E'_{sys}) = \mathbf{c}'^T \mathbf{p}(m)$$

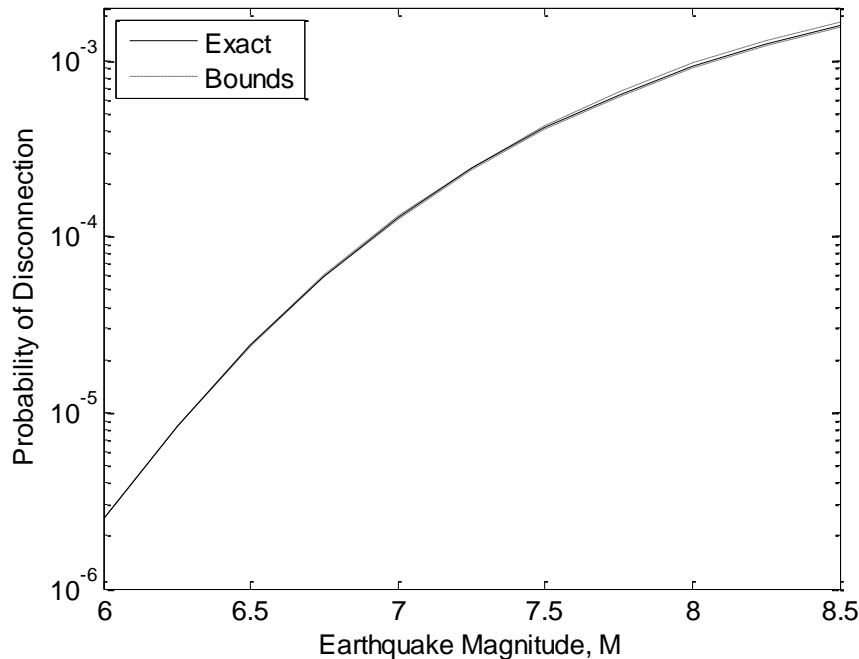
Conditional probability of disconnection of counties



$$P(E'_{sys}) = \mathbf{c}''^T \mathbf{p}(m)$$

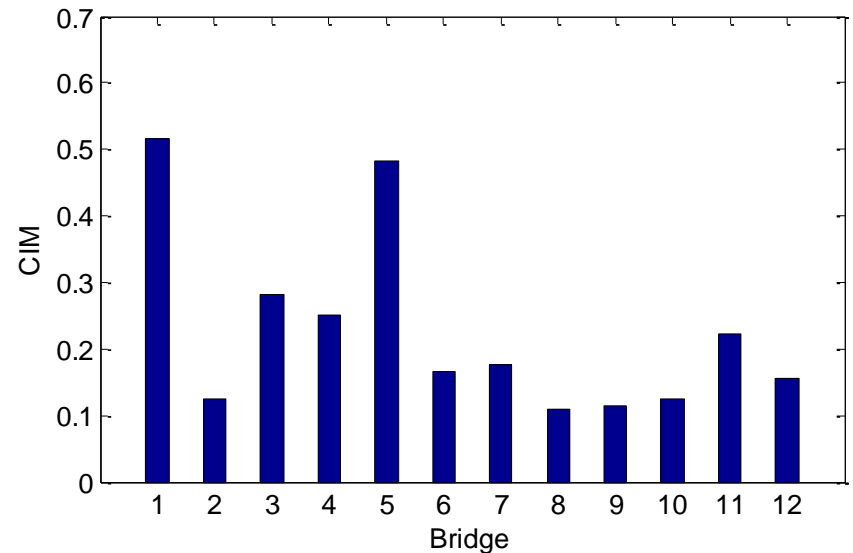
Prob (No. of failed bridges  $\geq k$ )

# Connectivity of a transportation network



$$\min(\max) \quad \mathbf{c}^T \mathbf{p}(m)$$

Bounds on  $P(\text{City 5 disconnected})$   
(No information on Bridge 12)

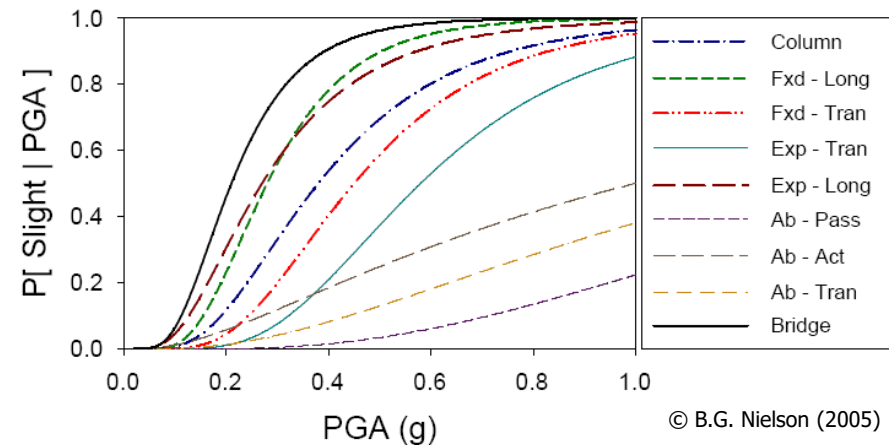
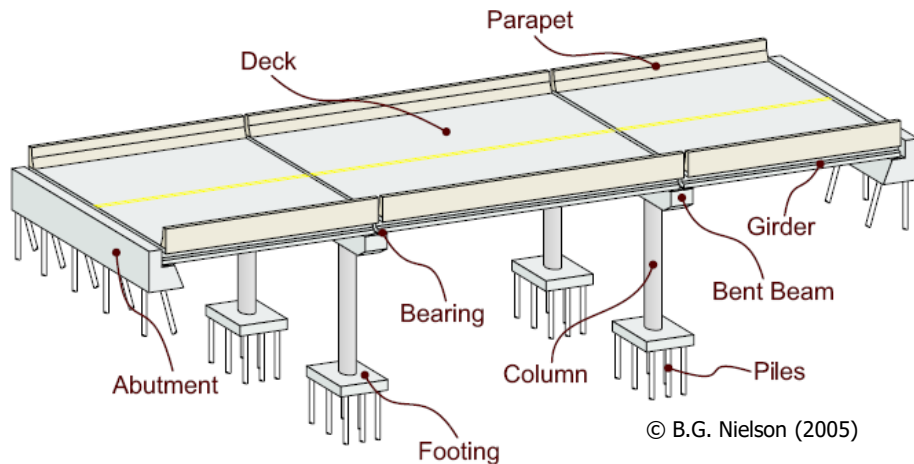


$$P(E_i | E_{sys}) = \frac{P(E_i E_{sys})}{P(E_{sys})} = \frac{\mathbf{c}'^T \tilde{\mathbf{p}}}{\mathbf{c}^T \tilde{\mathbf{p}}}$$

Importance measure of components  
w.r.t. the likelihood of at least a disconnection

# Appl. II: Damage of a bridge structural system

\* Song, J. and W.-H. Kang “System Reliability and Sensitivity under Statistical Dependence by Matrix-based System Reliability Method,” *Structural Safety*, Vol. 31(2), 148-156.



- Nielson (2005) developed analytical fragilities of bridge components such as bearings, abutments and columns
- Identified the statistical dependence between demands
- Probability that at least one component fails (series system)
- Performed MCS to account for component dependence



# Damage of a bridge structural system

\* Safety Factor  $F_i = \ln C_i - \ln D_i$

\* Fragility  $P(LS_i | IM) = P(F_i \leq 0 | IM)$

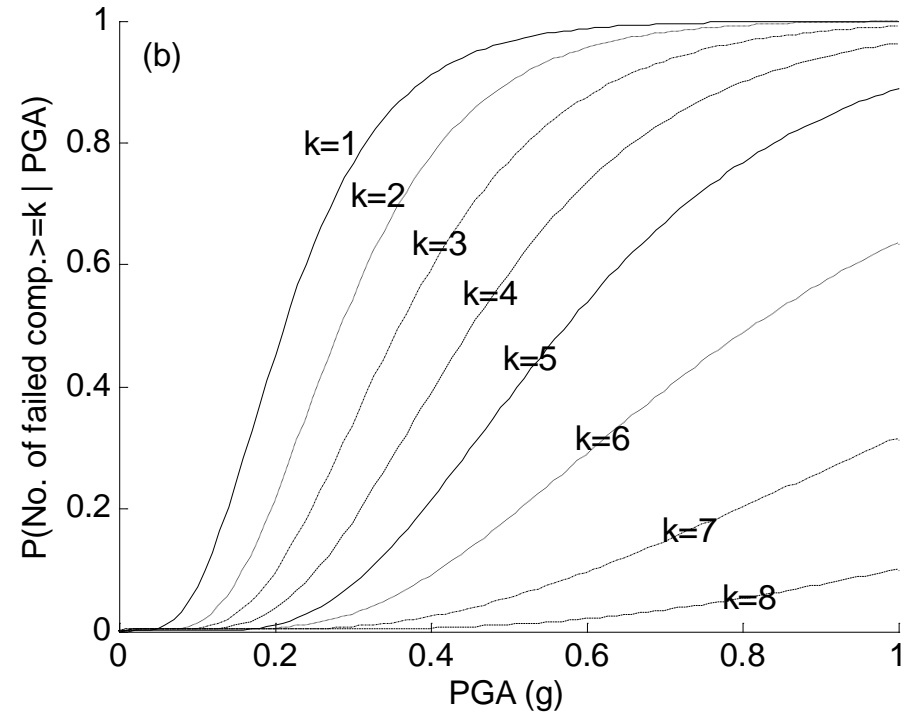
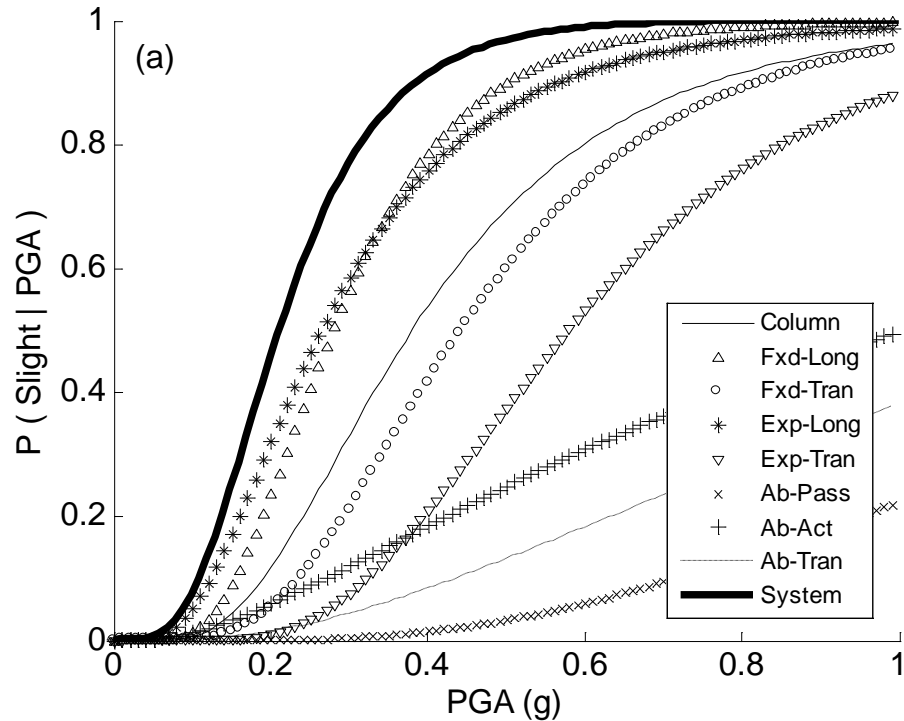
$$= P\left(Z_i \leq -\frac{\mu_{F_i}}{\sigma_{F_i}} \mid IM\right)$$

$$= \Phi\left[-\frac{\mu_{F_i}(IM)}{\sigma_{F_i}(IM)}\right]$$

\* Correlation  $\rho_{Z_i Z_j} = \rho_{F_i, F_j} = \frac{(\zeta_{D_i} \cdot \zeta_{D_j})}{(\zeta_{C_i}^2 + \zeta_{D_i}^2)^{1/2} (\zeta_{C_j}^2 + \zeta_{D_j}^2)^{1/2}} \cdot \underline{\rho_{\ln D_i, \ln D_j}}$

\* Fitting by DS-class corr. matrix: average of percentage error  $\sim 3\%$

# Damage of a bridge structural system



$$P(E_{\text{sys}} | \text{PGA} = pga) = \mathbf{c}^T \mathbf{p}(pga)$$

$$= \int \mathbf{c}^T \mathbf{p}(pga, x) \varphi(x) dx$$

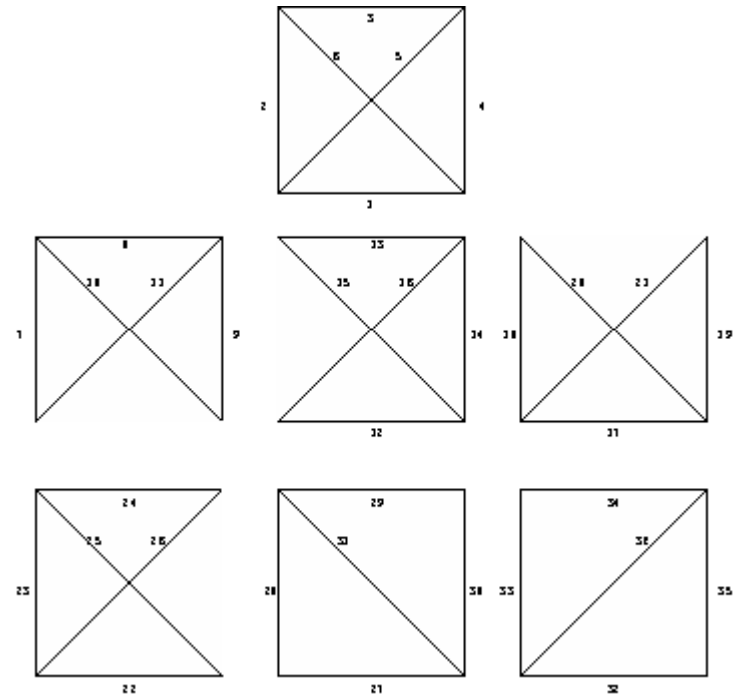
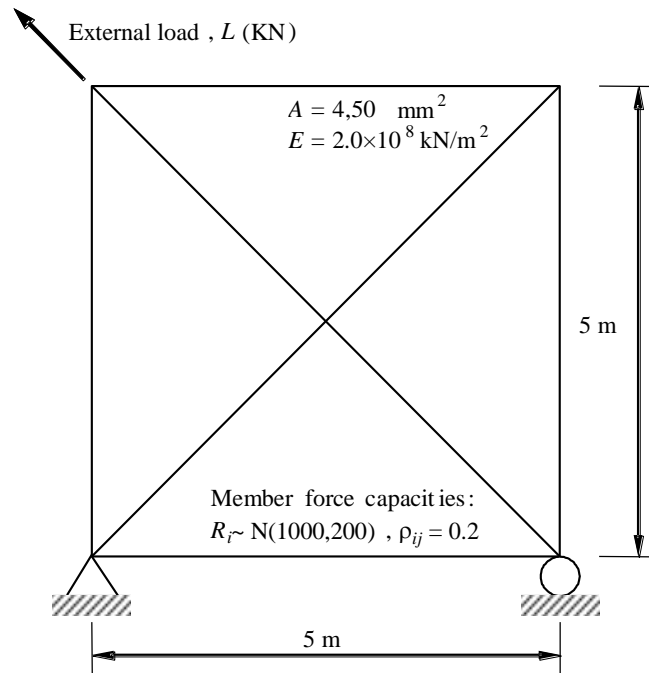
System fragility (at least one)

$$P(E_{\text{sys}} | \text{PGA} = pga) = \mathbf{c}'^T \mathbf{p}(pga)$$

$P(\text{No. of failed components} \geq k)$

# Appl. III: Progressive failure of a truss structure

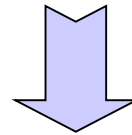
\* Song, J. and W.-H. Kang “System Reliability and Sensitivity under Statistical Dependence by Matrix-based System Reliability Method,” *Structural Safety*, Vol. 31(2), 148-156.



$$\begin{aligned}
 P(\bar{E}_{sys}) = & P[\bar{E}_1 \bar{E}_2 \bar{E}_3 \bar{E}_4 \bar{E}_5 \bar{E}_6 \cup (E_1 \bar{E}_2 \bar{E}_3 \bar{E}_4 \bar{E}_5 \bar{E}_6)(\bar{E}_7 \bar{E}_8 \bar{E}_9 \bar{E}_{10} \bar{E}_{11}) \\
 & \cup (\bar{E}_1 E_2 \bar{E}_3 \bar{E}_4 \bar{E}_5 \bar{E}_6)(\bar{E}_{12} \bar{E}_{13} \bar{E}_{14} \bar{E}_{15} \bar{E}_{16}) \cup \dots \\
 & \cup (\bar{E}_1 \bar{E}_2 \bar{E}_3 \bar{E}_4 \bar{E}_5 E_6)(\bar{E}_{32} \bar{E}_{33} \bar{E}_{34} \bar{E}_{35} \bar{E}_{36})]
 \end{aligned}$$

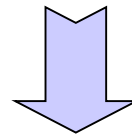
# Progressive failure of a truss structure

$$\begin{aligned}
 P(\bar{E}_{sys}) = & P[\bar{E}_1\bar{E}_2\bar{E}_3\bar{E}_4\bar{E}_5\bar{E}_6 \cup (E_1\bar{E}_2\bar{E}_3\bar{E}_4\bar{E}_5\bar{E}_6)(\bar{E}_7\bar{E}_8\bar{E}_9\bar{E}_{10}\bar{E}_{11}) \\
 & \cup (\bar{E}_1E_2\bar{E}_3\bar{E}_4\bar{E}_5\bar{E}_6)(\bar{E}_{12}\bar{E}_{13}\bar{E}_{14}\bar{E}_{15}\bar{E}_{16}) \cup \dots \\
 & \cup (\bar{E}_1\bar{E}_2\bar{E}_3\bar{E}_4\bar{E}_5E_6)(\bar{E}_{32}\bar{E}_{33}\bar{E}_{34}\bar{E}_{35}\bar{E}_{36})]
 \end{aligned}$$



Disjoint link sets (36→11)

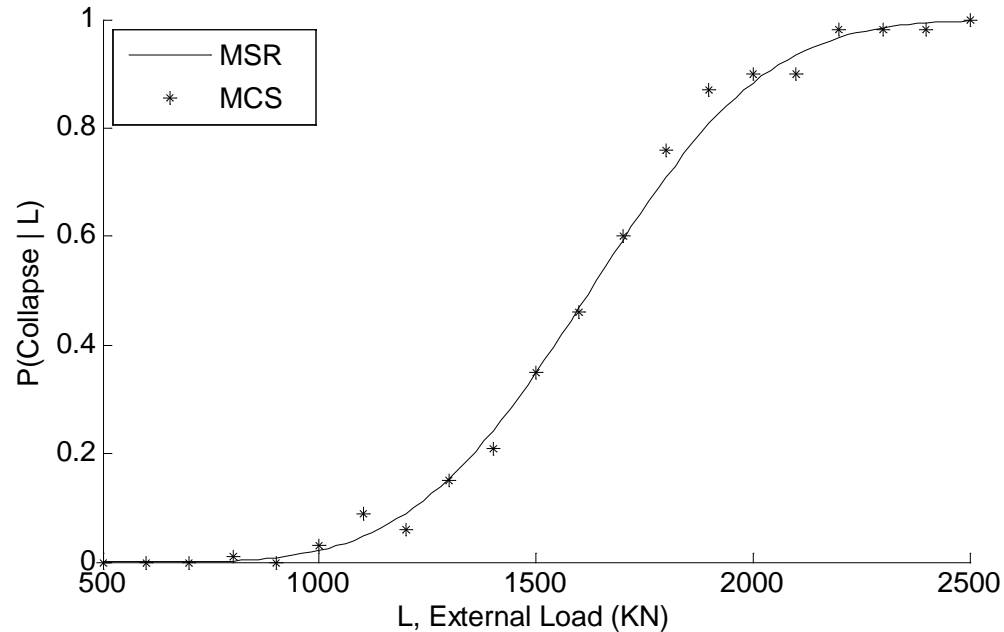
$$\begin{aligned}
 P(\bar{E}_{sys}) = & P(\bar{E}_1\bar{E}_2\bar{E}_3\bar{E}_4\bar{E}_5\bar{E}_6) + P(E_1\boxed{\bar{E}_2}\bar{E}_3\bar{E}_4\bar{E}_5\bar{E}_6\boxed{\bar{E}_7}\bar{E}_8\bar{E}_9\bar{E}_{10}\bar{E}_{11}) \\
 & \dots + P(\bar{E}_1\bar{E}_2\bar{E}_3\bar{E}_4\bar{E}_5E_6\bar{E}_{32}\bar{E}_{33}\bar{E}_{34}\bar{E}_{35}\bar{E}_{36})
 \end{aligned}$$



Perfect correlation

7 systems with 6 components

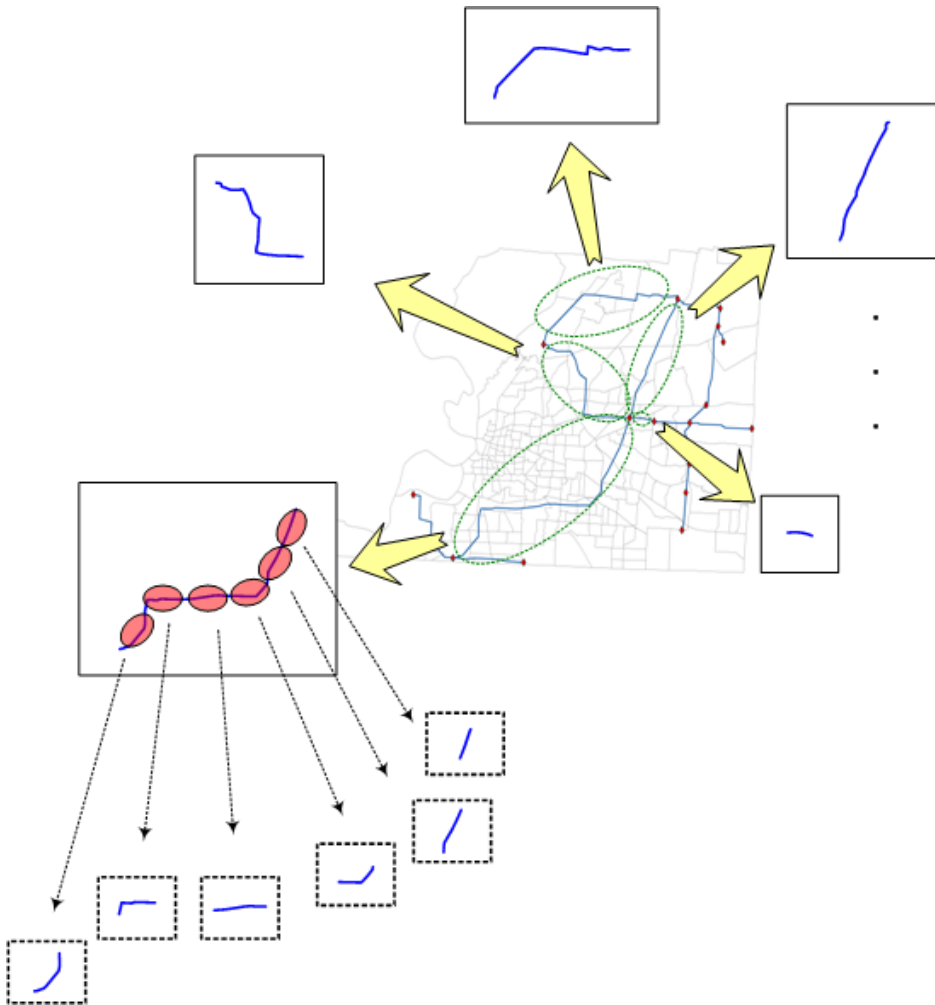
# Progressive failure of a truss structure



- System collapse fragility curve given abnormal load
- Verified through MCS
- Importance of members (components)
- Sensitivity of fragility w.r.t. design parameters

# Appl. IV: Multi-scale SRA of lifeline networks

\* Song, J., and S.-Y. Ok (2010). Multi-scale system reliability analysis of lifeline networks under earthquake hazards. *Earthquake Engineering and Structural Dynamics*, Vol. 39(3), 259-279.



## ▪ “Divide and Conquer” approach

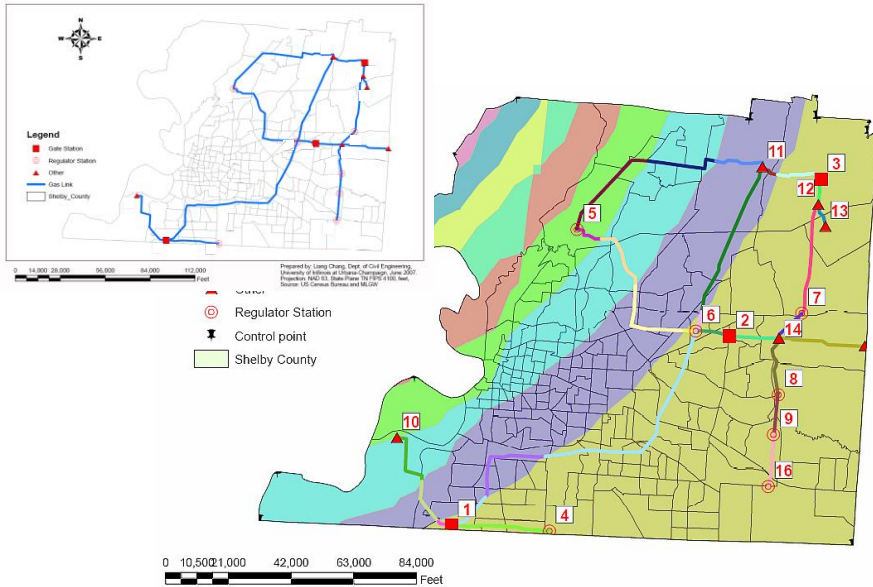
- Lower-scale system reliability analyses are performed for “supercomponents” and followed by higher-scale system reliability analyses
- Proposed to facilitate the use of **LP bounds method** (Song and Der Kiureghian, 2003) for large-size systems
- **MSR method** is a good tool for SRA at multiple scales

## ▪ Advantages

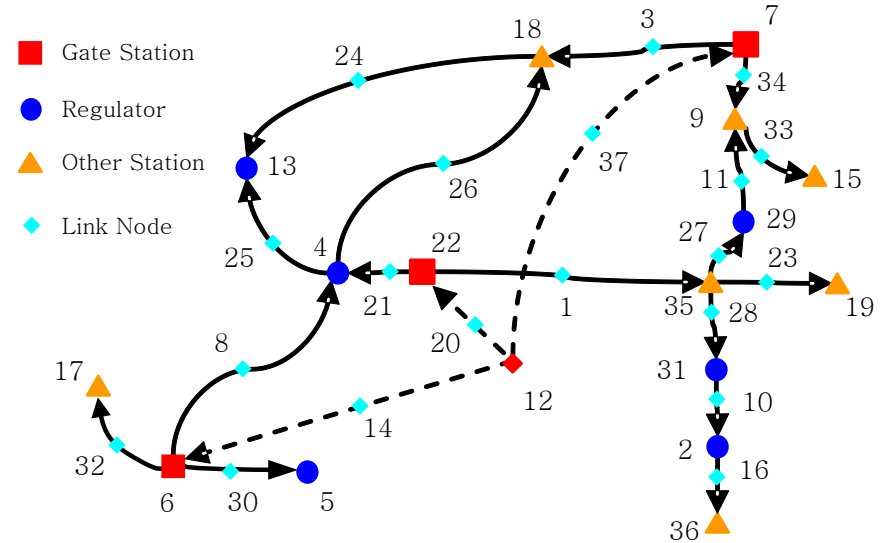
- **Multi-scale modeling** of a system – seeing big picture without disregarding the details
- Helps identify **important** components and parameters at **multiple scales**
- **Collaborative** risk management
- Facilitates parallel computing

# Example: MLGW gas network

MLGW Gas Transmission System in Memphis and Shelby County, TN

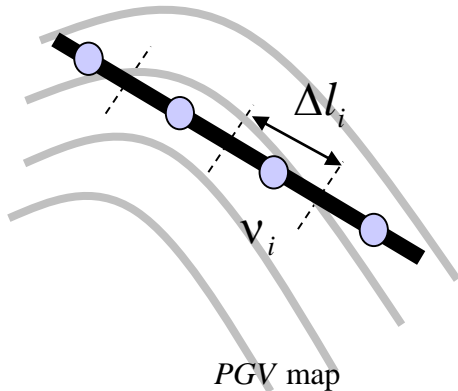


Simplified MLGW Gas Network (37-node)



- Gas pipeline network of Memphis Light, Gas, and Water (MLGW), Shelby County, TN
- A simplified network in Chang et al. (1996) was modified based on comments from R. Bowker (MLGW)
- 37-node and 40-arc network: nodes representing pipelines and stations
- Earthquake hazard scenarios: Epicenter at N35.54°-W90.43° at Blytheville, AR
- Fragilities of pipelines and stations – *HAZUS-MH*
- PGV and PGA maps from *MAEviz*

# Failure prob. of pipeline segments



- Failure probability of the  $i$ -th segment of a pipeline

$$P_i = 1 - \exp(-v_i \cdot \Delta L_i)$$

- Failure occurrence rate of a pipeline (HAZUS-MH: FEMA 2003)

$$v_i = k \cdot (PGV_i)^\gamma$$

- Uncertainty in PGV (Adachi & Ellingwood, 2007)

$$PGV_i = \overline{PGV_i} \times \varepsilon_i$$

Lognormal r.v. (median = 1, c.o.v. = 0.6)

Attenuated PGV (Fernandez and Rix 2006)

- Spatial Correlation (Wang & Takada, 2005)

$$\rho_{\ln PGV_i, \ln PGV_j} = \rho_{\ln \varepsilon_i, \ln \varepsilon_j} = \exp(-\| \mathbf{x}_i - \mathbf{x}_j \| / L_{corr})$$

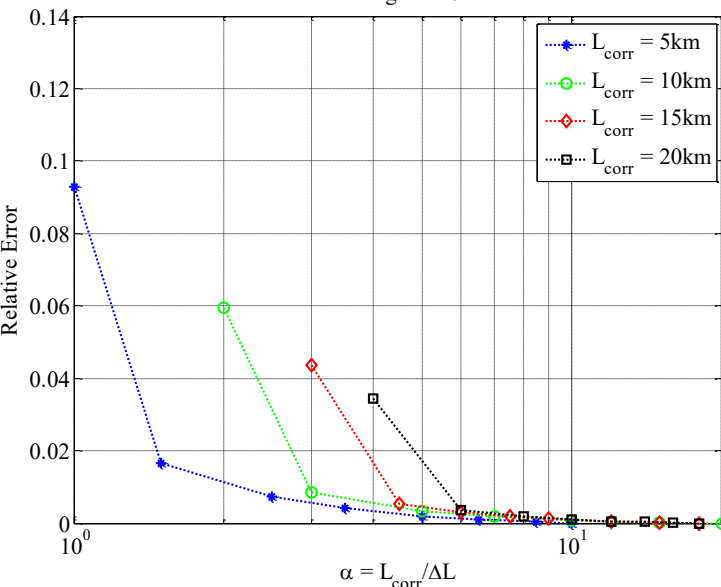
- Generalized Dunnett-Sobel (Song and Kang, 2008)

$$Z_i = \ln \varepsilon_i / \zeta_i \sim N(\mathbf{0}, \mathbf{R}) \rightarrow \text{Find gDS that fits best}$$

- ( $\leftarrow$ ) Discretization error

choose number of segments considering corr. length

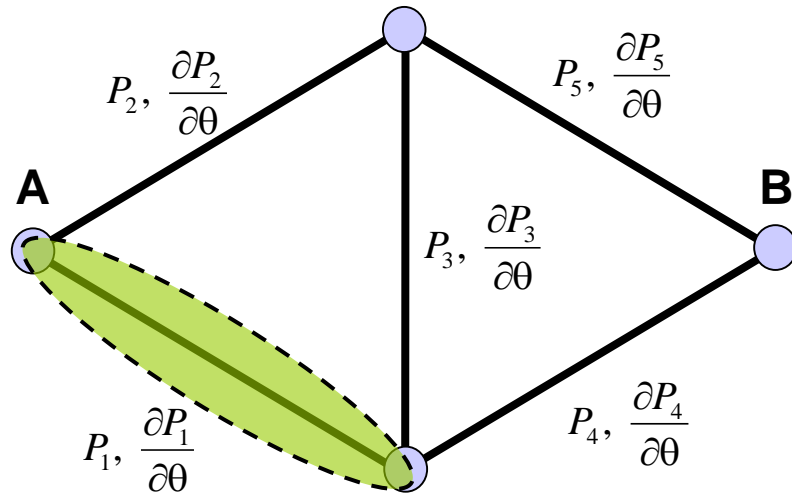
Link Length = 10km





# Multi-scale SRA using MSR Method

## Higher-scale

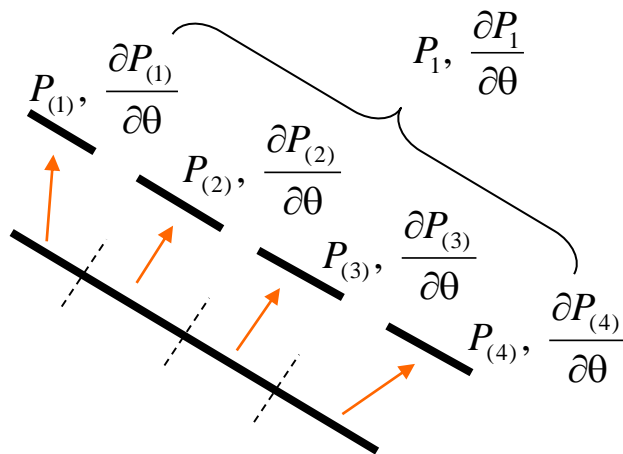


$$P(E_{sys}) = \mathbf{c}^T \mathbf{p}$$

$$\frac{\partial P(E_{sys})}{\partial \theta} = \mathbf{c}^T \frac{\partial \mathbf{p}}{\partial \theta} = \mathbf{c}^T \hat{\mathbf{P}} \frac{\partial \mathbf{P}}{\partial \theta}$$

→ MSR analysis using failure probability and sensitivity of links  $P_i, \frac{\partial P_i}{\partial \theta} \quad i=1, \dots, n_{link}$

## Lower-scale

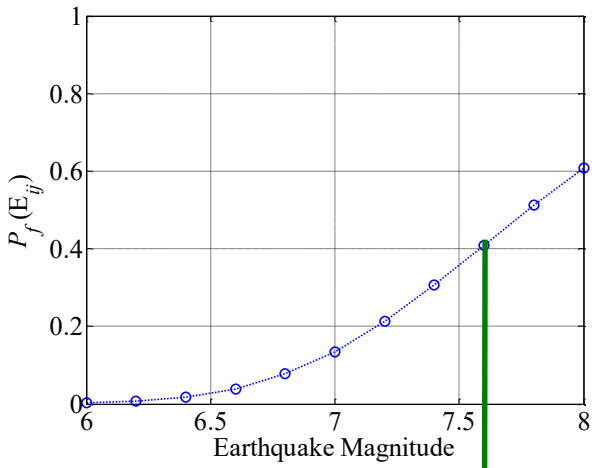


$$P_1 = \mathbf{c}_1^T \mathbf{p}_1$$

$$\frac{\partial P_1}{\partial \theta} = \mathbf{c}_1^T \frac{\partial \mathbf{p}_1}{\partial \theta} = \mathbf{c}_1^T \hat{\mathbf{P}}_1 \frac{\partial \mathbf{P}_1}{\partial \theta}$$

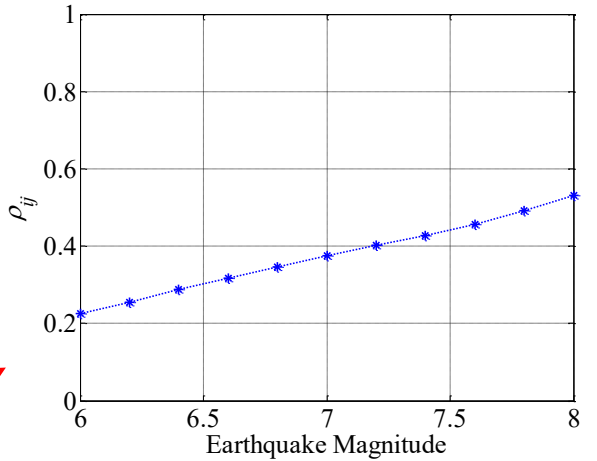
→ MSR analysis using failure probability and sensitivity of segments  $P_{(i)}, \frac{\partial P_{(i)}}{\partial \theta} \quad i=1, \dots, n_{seg}$

# Correlation between pipelines



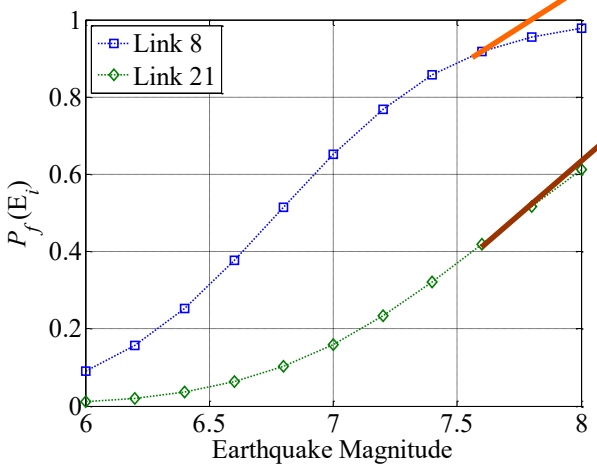
**MSR for joint failure of pipelines**

$$(E_{i,1} \cup \dots \cup E_{i,N_i}) \cap (E_{j,1} \cup \dots \cup E_{j,N_j})$$



$$\Phi_2(-\beta_1, -\beta_2, \rho) = \Phi(-\beta_1)\Phi(-\beta_2) + \int_0^\rho \varphi(-\beta_1, -\beta_2, \rho') d\rho'$$

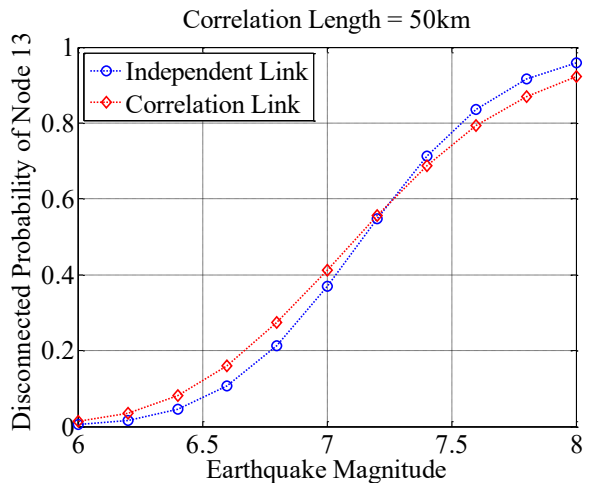
**MSR w/ correlation**



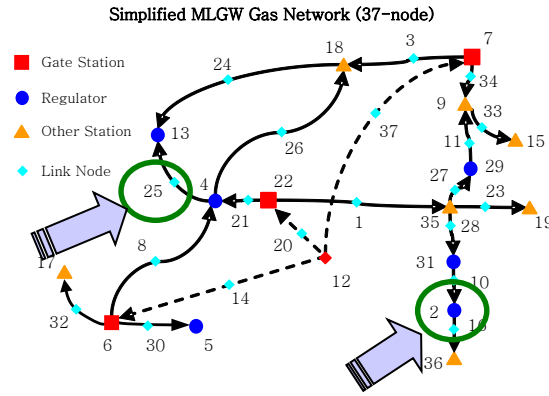
**Lower-scale MSR**

$$(E_{i,1} \cup \dots \cup E_{i,N_i})$$

$$(E_{j,1} \cup \dots \cup E_{j,N_j})$$

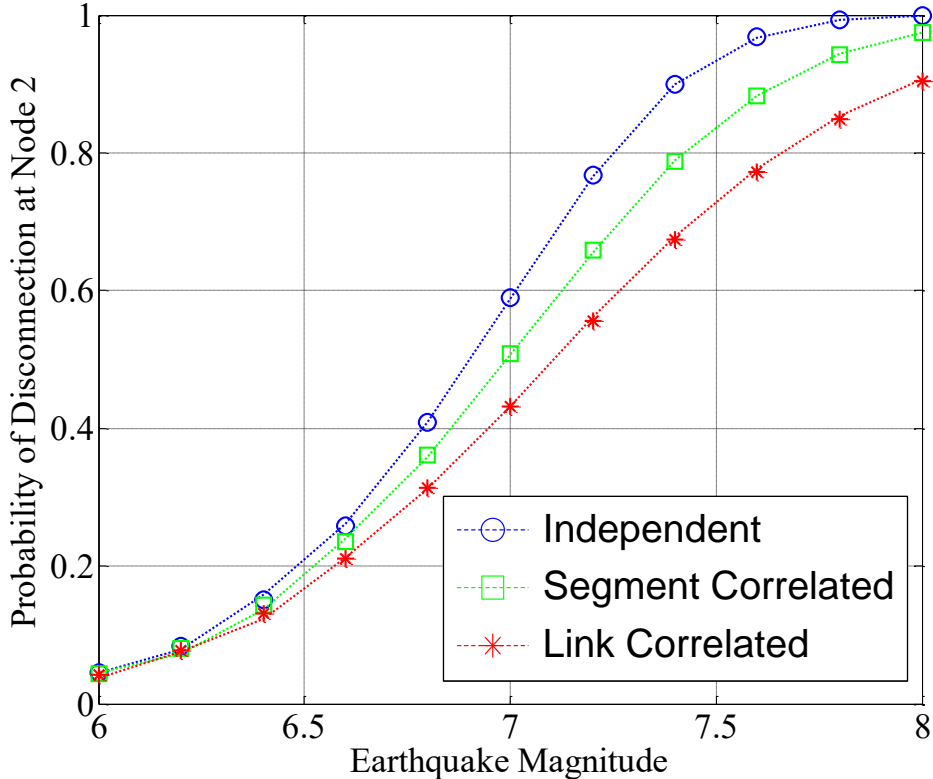
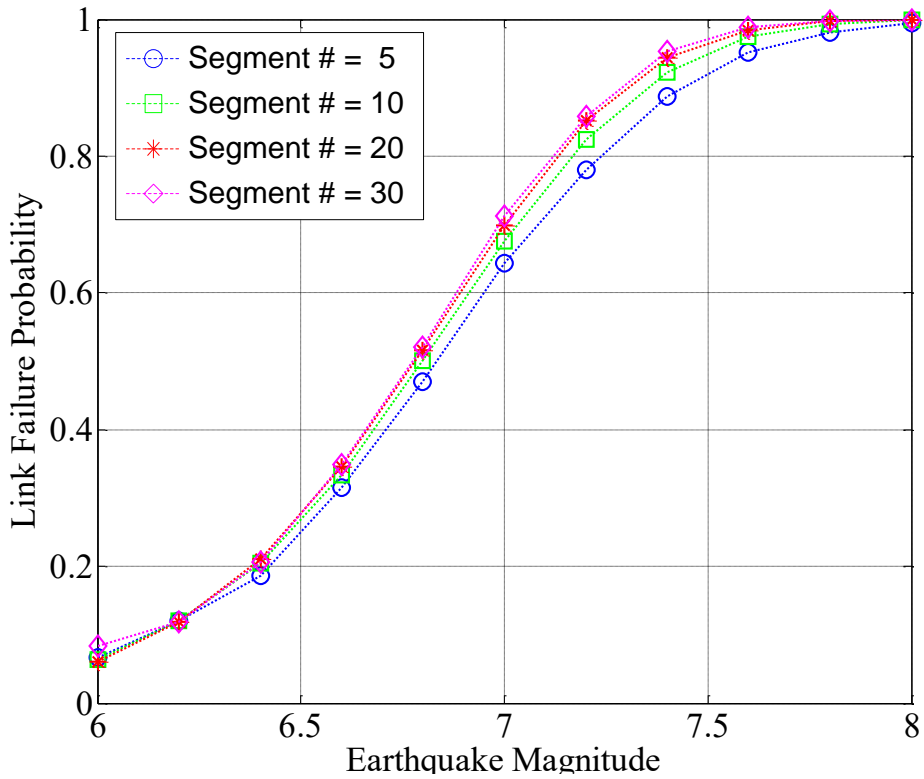


# Risk at multiple scales



Lower-scale: pipelines

Failure probability of **Link 25**

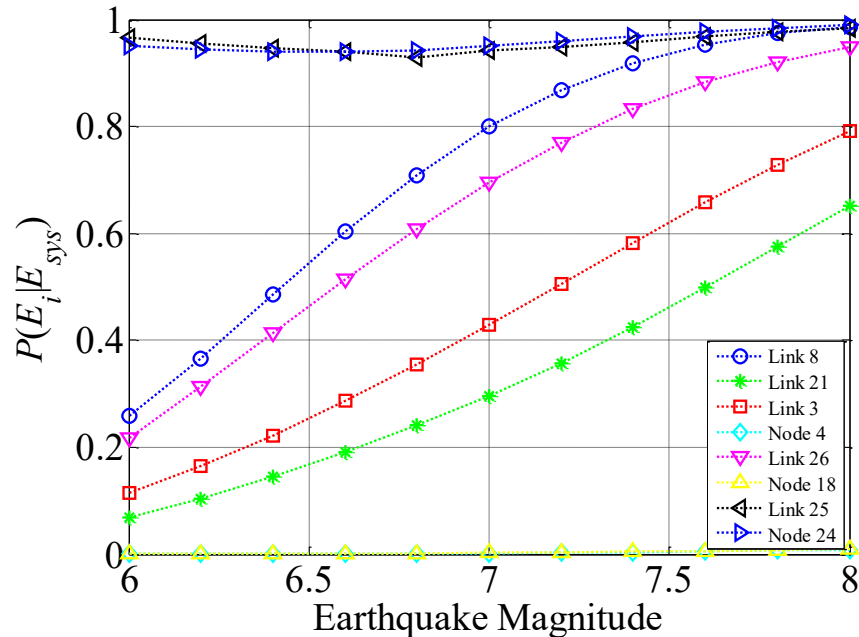


Higher-scale: service nodes

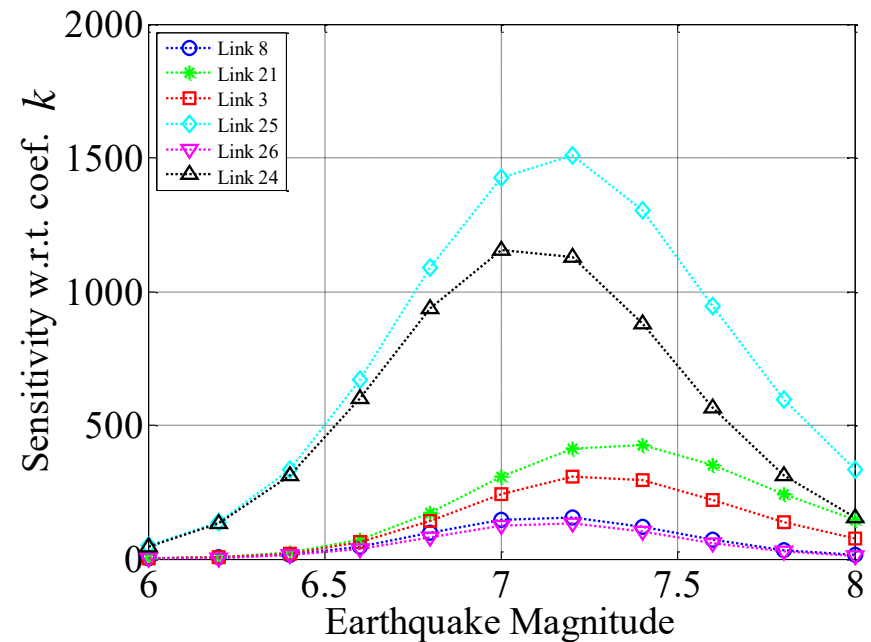
Prob. of Disconnection at **Node 2**

# Probabilistic inference and sensitivity

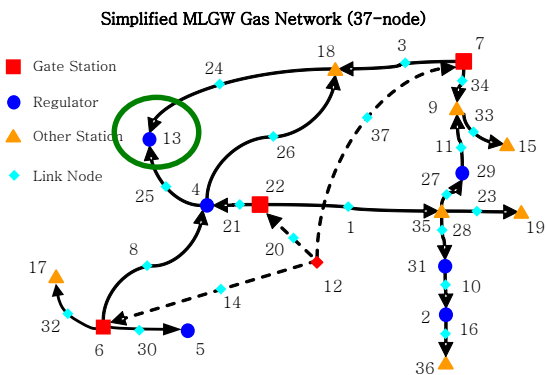
## Conditional Probabilities



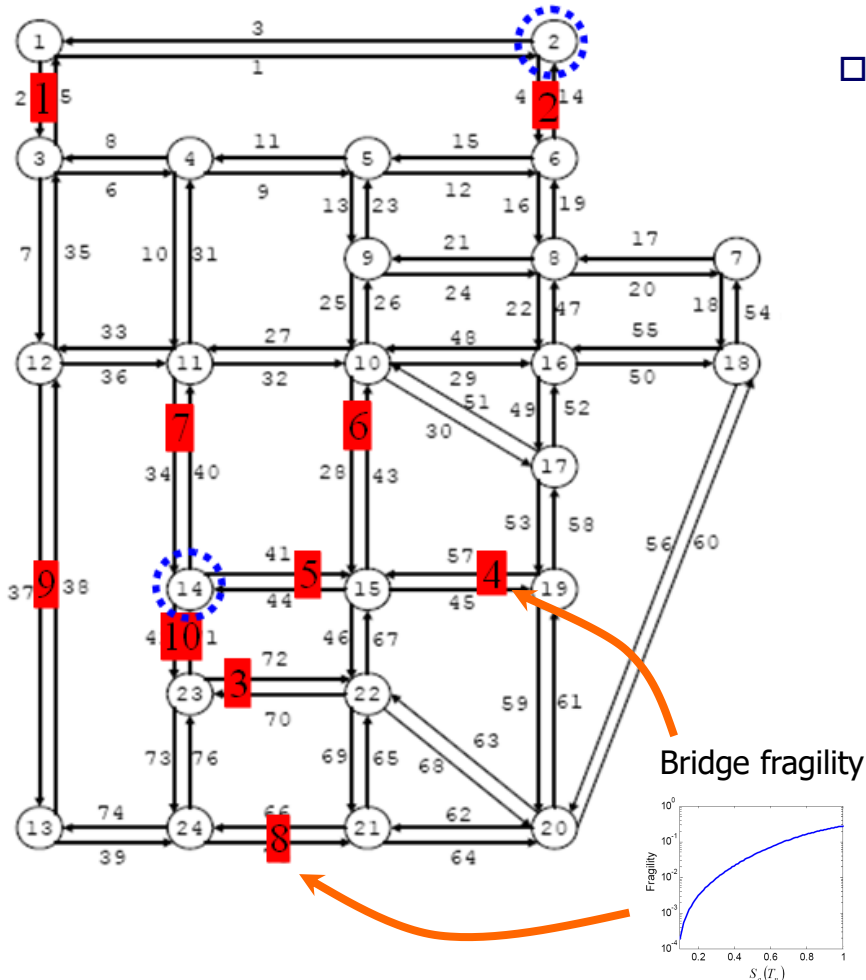
## Parameter Sensitivity



- Conditional probability of link failure probability given observed system event (e.g. disconnection)
- Sensitivity of system failure probability with respect to parameters in PGV-based model for failure occurrence rate:  $v_i = k \cdot (PGV_i)^\gamma$



# Appl. V: Post-hazard **flow** capacity of a network



- Traffic flow **capacity** between two points in a network → determined by combinations of bridge damage

**q** : a vector of network flow capacity for bridge failure combinations (obtained by maximum flow capacity analysis)

$$\mu_Q = \mathbf{q}^T \mathbf{p} \quad \text{: average post-hazard flow capacity}$$

$$\sigma_Q^2 = (\mathbf{q} \cdot * \mathbf{q})^T \mathbf{p} - (\mathbf{q}^T \mathbf{p})^2$$

: variance of post-hazard flow capacity

$$P(Q < a) = \sum_{\forall i: q_i < a} p_i$$

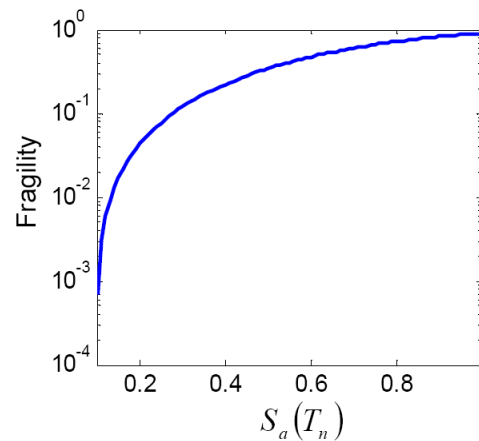
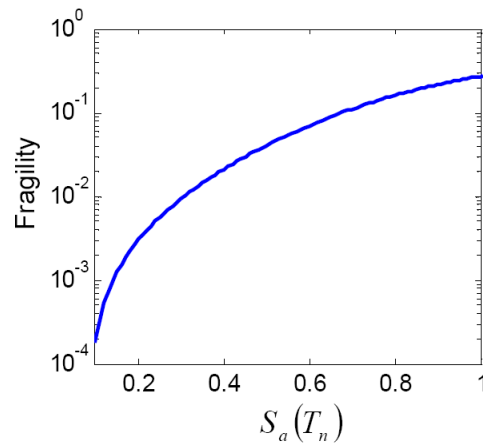
: probability that flow capacity is lower than  $a$

Example: Modified Sioux-Falls network

Red: bridges; Circles: Starting & Ending points

# Multi-state Fragility

- Fragility curves (Gardoni *et al.* 2002, 2003)



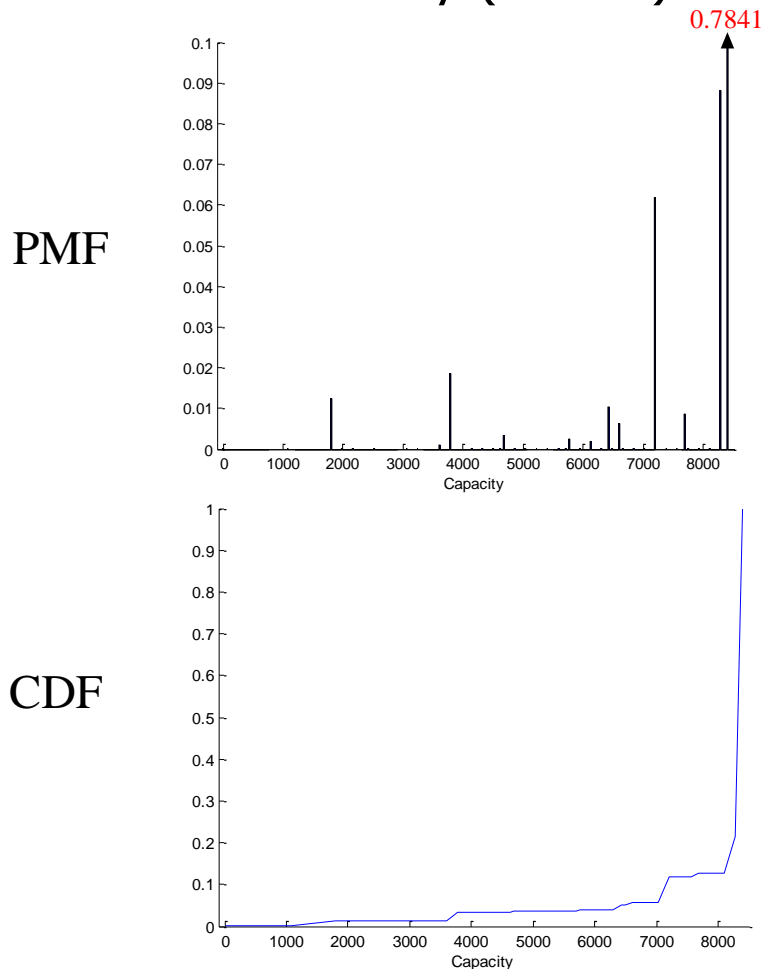
⇒ Only two states, “connected” or “disconnected”

$$\begin{aligned} P(\text{Complete failure}) &= 0.3 \times P_f \\ P(\text{Heavy damage}) &= 0.45 \times P_f \\ P(\text{Moderate damage}) &= 0.25 \times P_f \\ P(\text{No damage}) &= 1 - P_f \end{aligned}$$

$$\begin{aligned} F(\text{Complete failure}) &= 0 \\ F(\text{Heavy damage}) &= 0.3 \times \text{Full capacity} \\ F(\text{Moderate damage}) &= 0.7 \times \text{Full capacity} \\ F(\text{No damage}) &= 1.0 \times \text{Full capacity} \end{aligned}$$

# Uncertainty quantification of flow capacity

- Capacity distribution for a given seismic intensity (M=7.0)

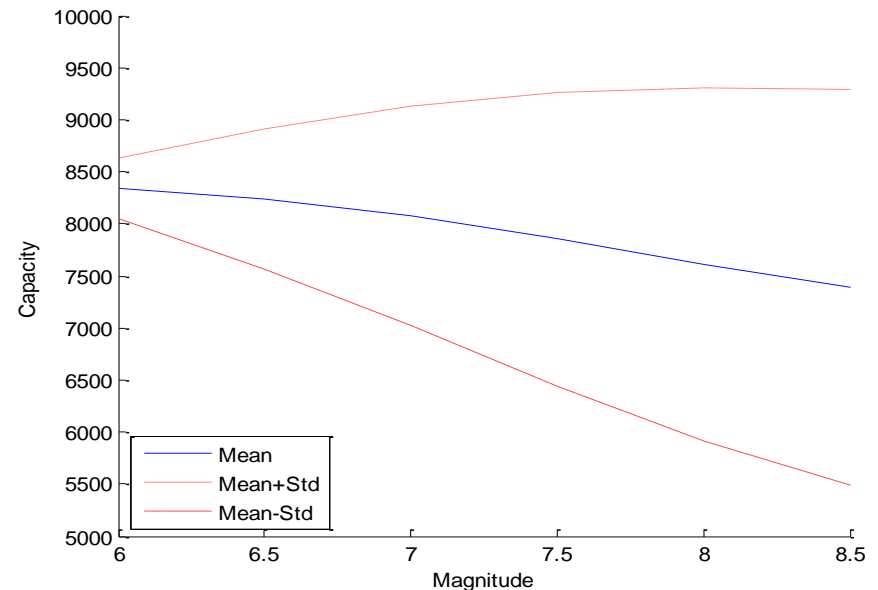


- Statistical parameters of flow capacity (M=6.0~8.5)

$$\mu_Q = \mathbf{p}^T \mathbf{f}$$

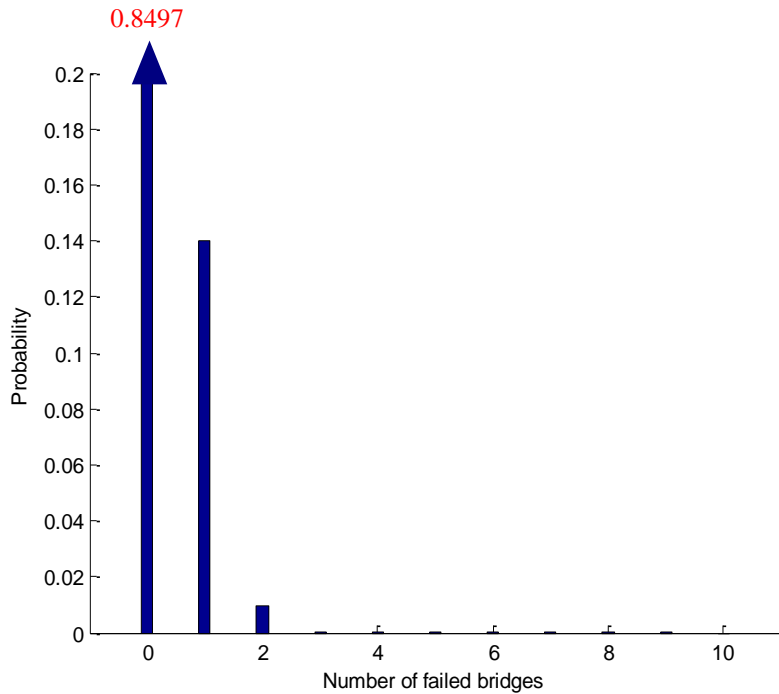
$$\sigma_Q = (\mathbf{p}^T (\mathbf{f} \cdot \mathbf{f}) - \mu_Q^2)^{1/2}$$

$$\delta_Q = \sigma_Q / \mu_Q$$

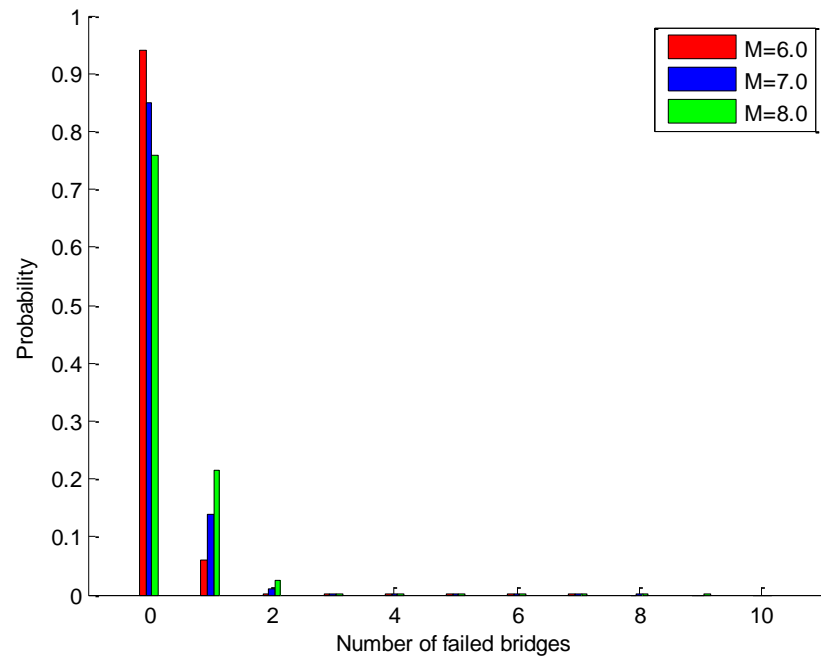


# Analysis Results

- Probability with number of failed bridges



**M=7.0**



**M=6.0~8.0**

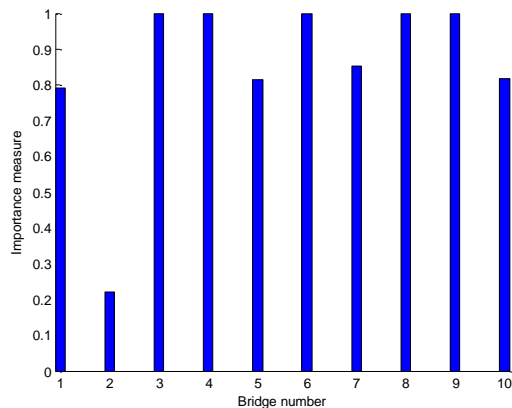


# Analysis Results

- Conditional flow capacity (For 10<sup>th</sup> bridge, M=7.0)

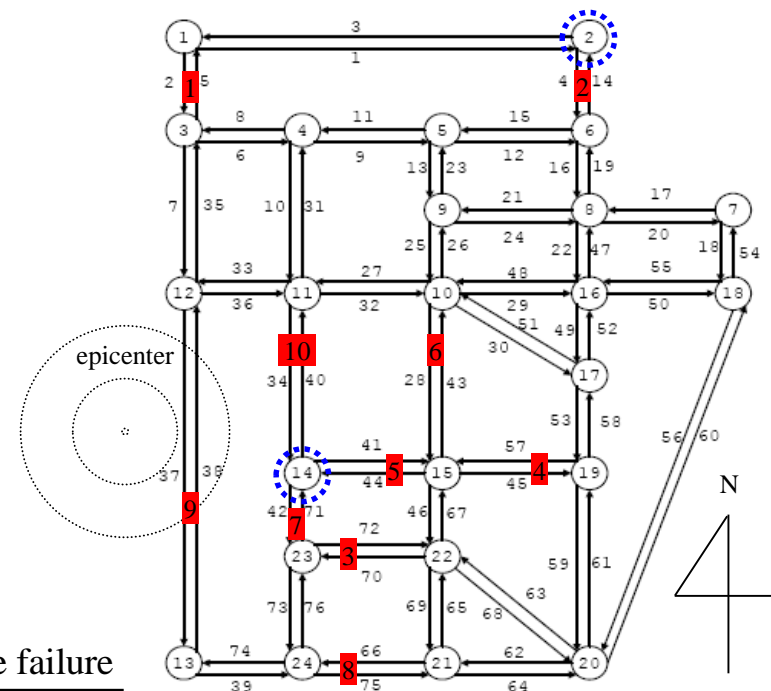
Parameter		Value
Mean	$\mu_{Q 10th}$	6591.9 ( <b>8076.3</b> )
Standard deviation	$\sigma_{Q 10th}$	1268.9 ( <b>1056.6</b> )
C.O.V.	$\delta_{Q 10th}$	0.1925 ( <b>0.1308</b> )

- Importance measure for all bridges (M=7.0)



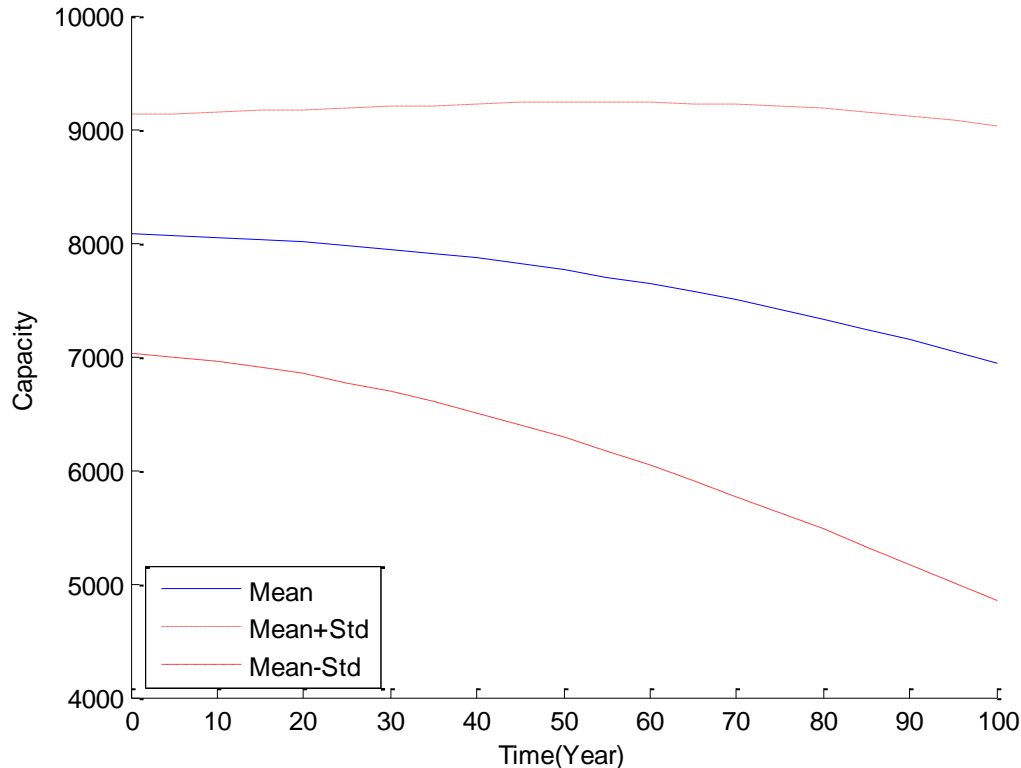
$$RF = 1 - \frac{\mu_{Q|bridge\ failure}}{\mu_Q}$$

1<sup>st</sup>, 2<sup>nd</sup>, 5<sup>th</sup>, 7<sup>th</sup>, and 10<sup>th</sup> bridges are most important



# Analysis Results

## Flow capacity with deterioration



## Assumptions

$$P(T, \text{Complete failure})$$

$$= P(\text{Complete failure}) \times (1.0 + 0.0005 \times T^2)$$

$$P(T, \text{Heavy damage})$$

$$= P(\text{Heavy damage}) \times (1.0 + 0.015 \times T)$$

$$P(T, \text{Moderate damage})$$

$$= P(\text{Moderate damage}) \times (1.0 - 0.015 \times T)$$

$$P(T, \text{No damage}) = 1 - P(T, \text{Complete failure})$$

$$- P(T, \text{Heavy damage})$$

$$- P(T, \text{Moderate damage})$$

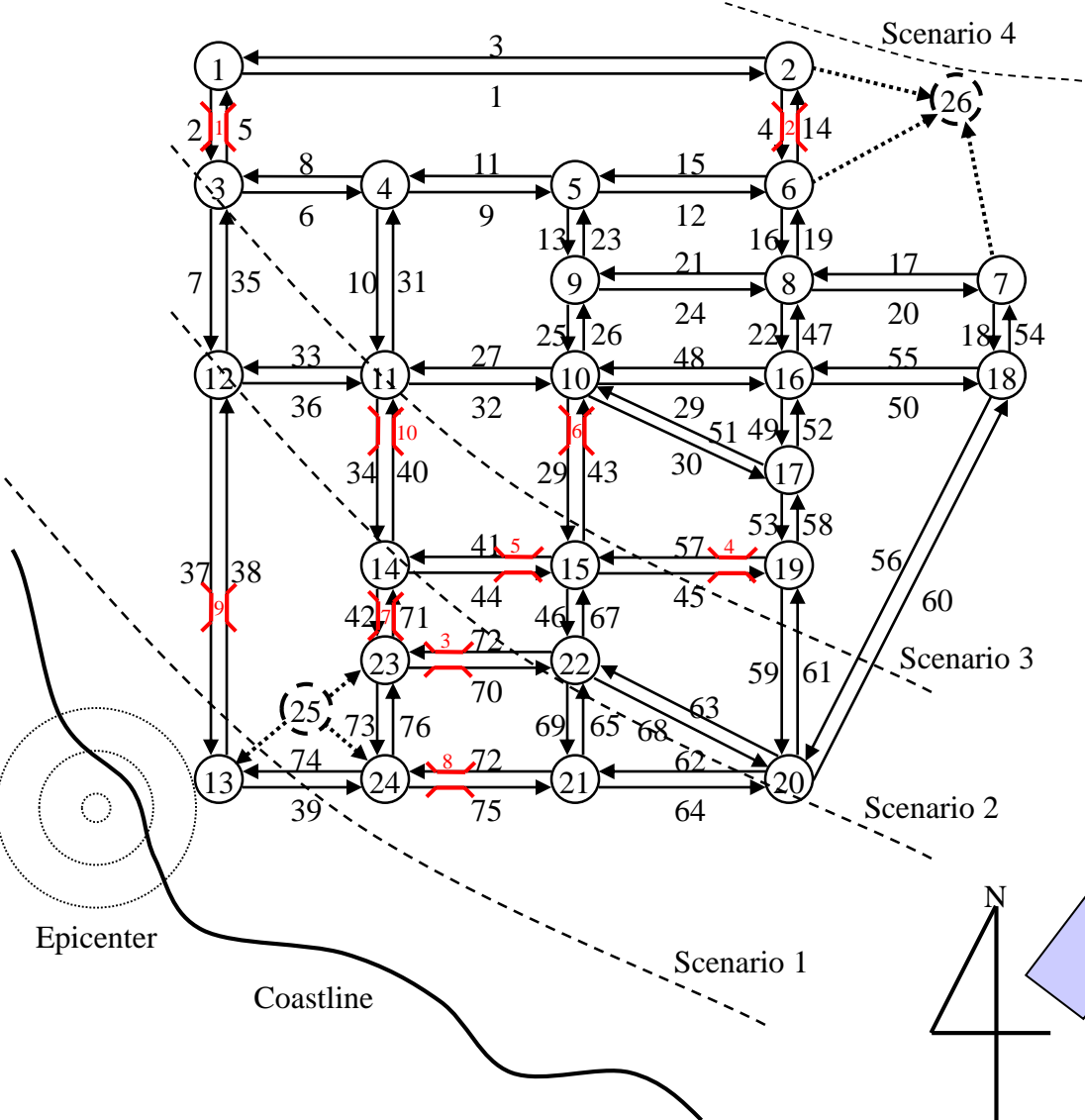
, where  $T$ : [Years]

$$\mu_Q(t) = \mathbf{q}^T \mathbf{p}(t)$$

$$\sigma_Q(t) = \sqrt{(\mathbf{q} \cdot \mathbf{q})^T \mathbf{p}(t) - \mu_Q^2(t)}$$

# Extension to multi-hazard environment

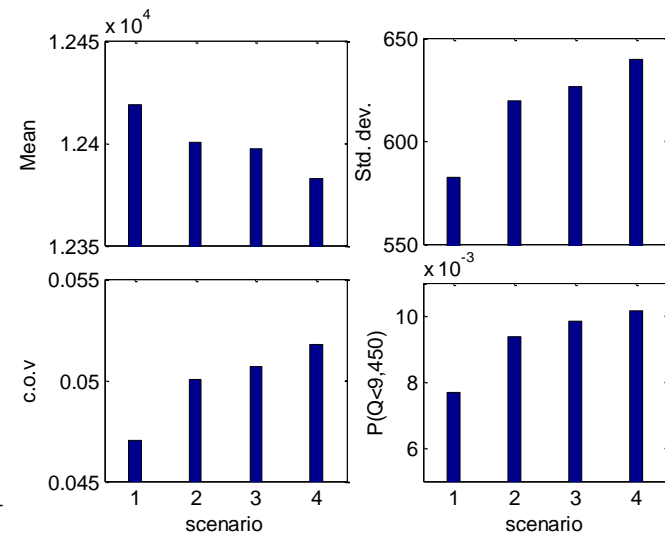
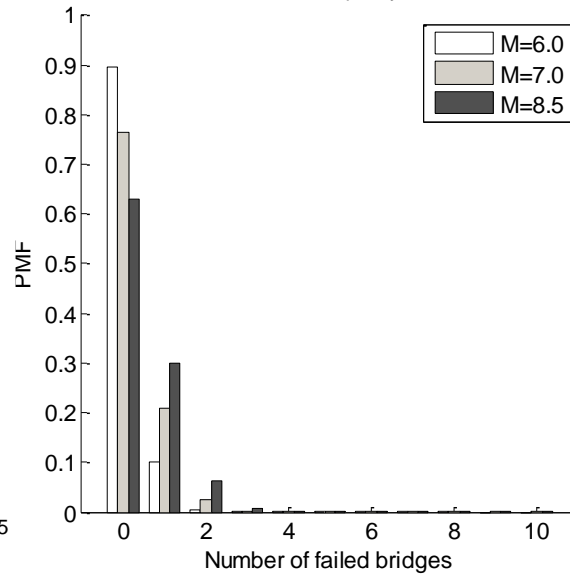
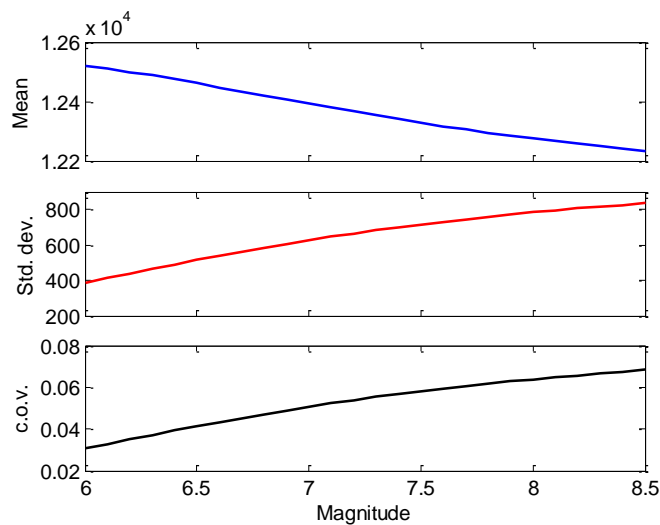
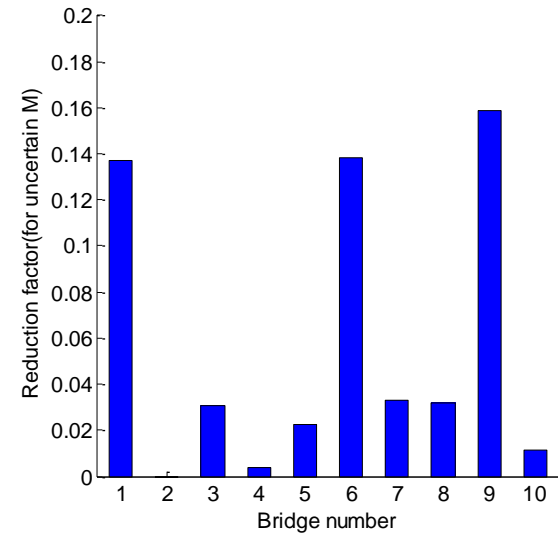
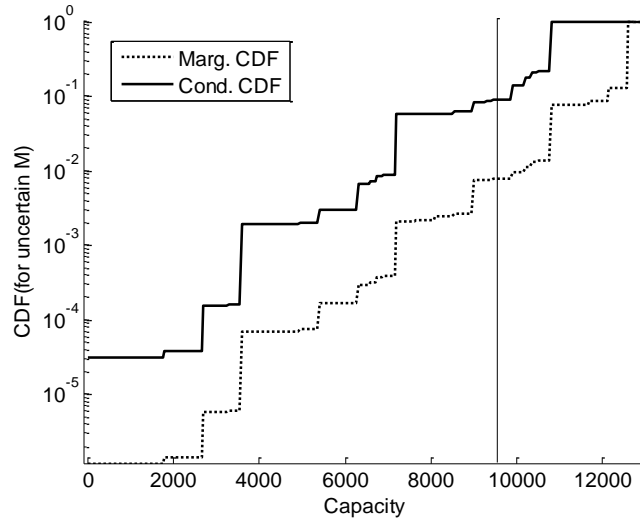
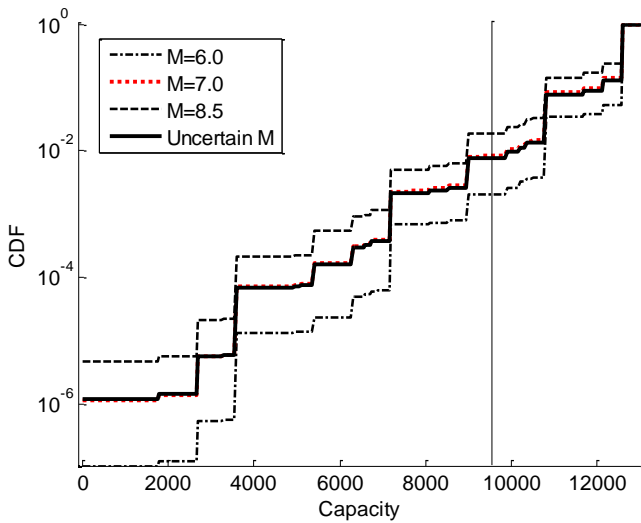
\* Lee, Y.-J., J. Song, P. Gardoni, and H.-W. Lim. (2010). Post-hazard flow capacity of bridge transportation network considering structural deterioration of bridges, *Structure and Infrastructure Engineering*, Accepted for Publication.



- More realistic assumptions
  - Multi-state fragility estimates w.r.t. drift capacity levels
  - Attenuation relationship (PSA & PGV)
  - Deterioration fragility estimates (Choe *et al.* 2007)
  - Multi-state flow capacity level proportional to number of open lanes
  - Deterioration scenarios
- Area-to-area flow capacity
- Further analysis for uncertain earthquake magnitude

**Progress of Structural Deterioration (Corrosion) by Sea Air**

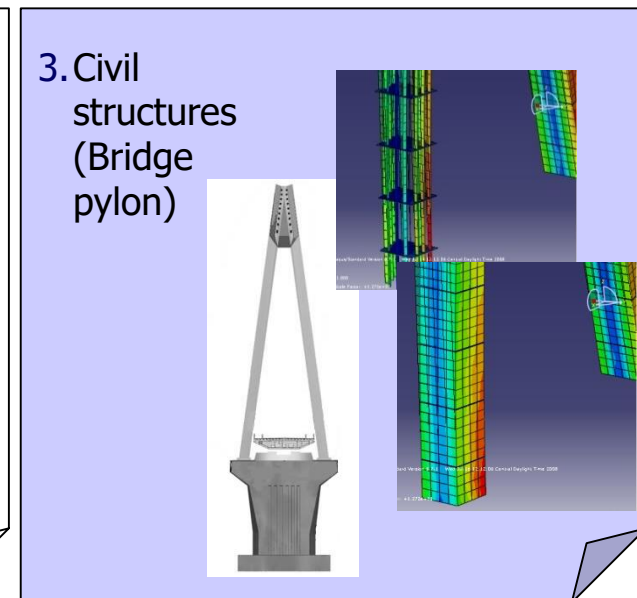
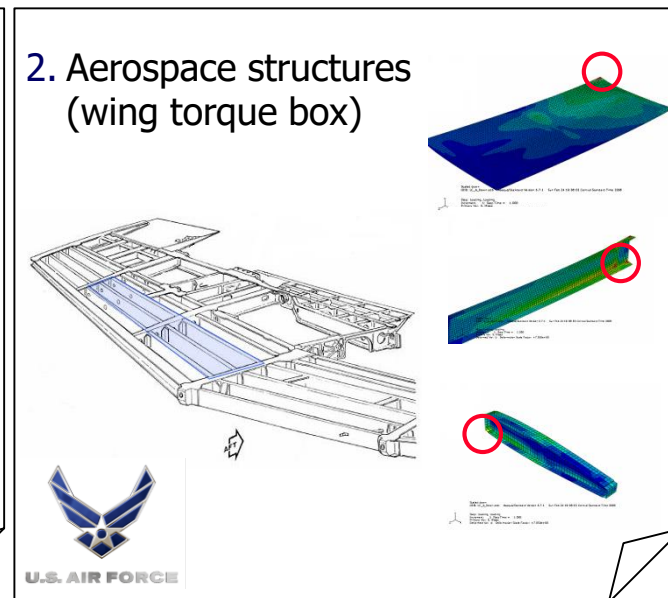
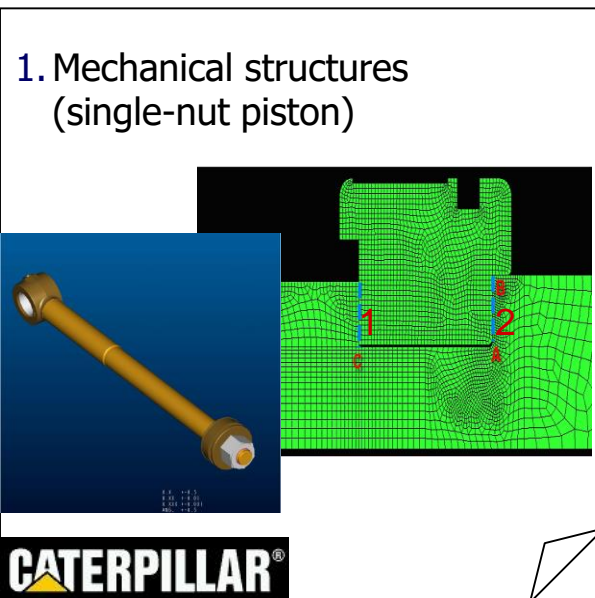
# Analysis Results



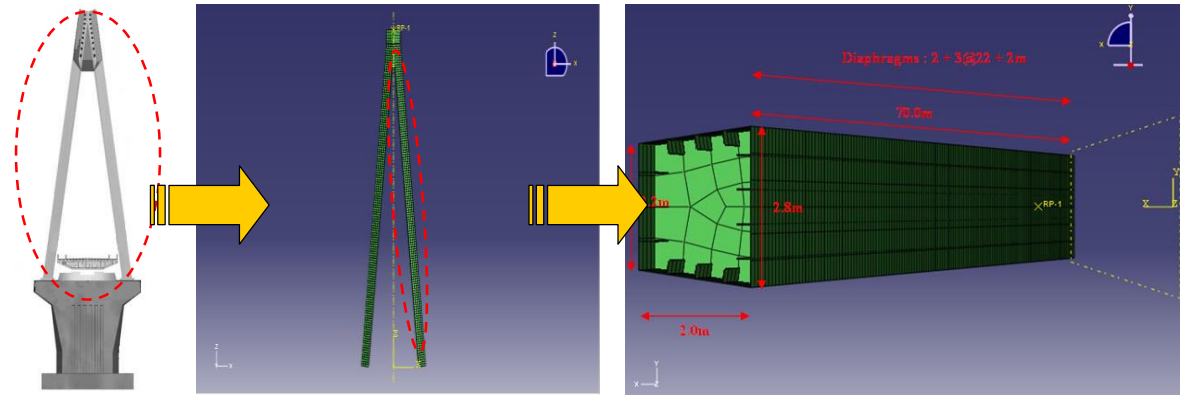
# Application VI: FE system reliability analysis

\* Lee, Y.-J., J. Song, and E.J. Tuegel (2008). Finite element system reliability analysis of a wing torque box. *Proc. 10<sup>th</sup> AIAA NDA*, April 7-10, Schaumburg, IL.

- FE reliability analysis: component vs. system
  - System-level risk is a logical function of multiple component events characterized by **failure modes, locations** and **load cases**
  - Using MSR methods, the **system-level risk and parameter sensitivities** are estimated based on the results of FE “component” reliability analysis.

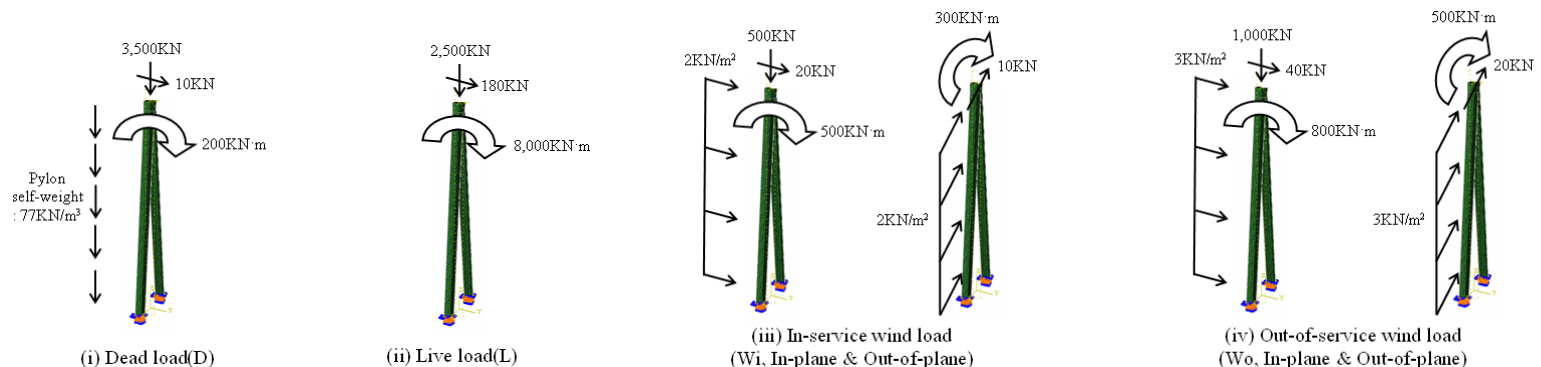


# Example: FE-SRA of bridge pylon system



## ■ Bridge pylon system

- Consists of 2 arms – each has 13 stiffeners and 23 diaphragms
- Yielding failure considered in this example
- Uncertainties in Young's modulus, yield strength and scale factors of load cases (dead, live, in-service wind and out-of-service wind loads) considered
- Two load combinations considered:  $LC1 = D+L+W_i$ ,  $LC2 = D+W_o$



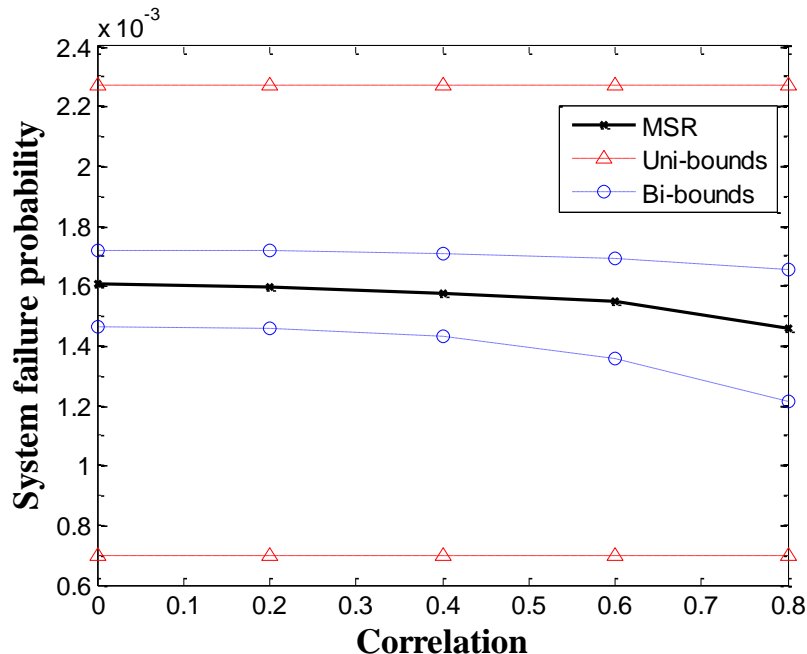


# FE system reliability analysis by MSR

## FE-SRA by MSR

- Probability of most dominant component:  $6.996 \times 10^{-4}$  vs. system failure probability  $1.550 \times 10^{-3}$   
→ component reliability analysis may underestimate the risk significantly
- Using component failure probability and sensitivity, the MSR method computes the system level parameter sensitivity
- Can analyze other system events just by replacing event vector  $\mathbf{c}$

$$\begin{aligned}
 P(E_{sys}) &= P\left(\bigcup_{i=1}^8 E_i\right) \cong P\left[\bigcup_{i=1}^8 \beta_i - Z_i \leq 0\right] \\
 &= \int_{\Omega} \varphi_N(\mathbf{z}; \mathbf{R}) d\mathbf{z} \\
 &= \int_{\mathbf{s}} \mathbf{c}^T \mathbf{p}(\mathbf{s}) f_s(\mathbf{s}) d\mathbf{s}
 \end{aligned}$$



Random variables	$\delta_i = \frac{\partial P_1}{\partial \mu_i} \sigma_i$	$\eta_i = \frac{\partial P_1}{\partial \sigma_i} \sigma_i$
Diaphragm (Left)	-0.0004	0
Diaphragm (Right)	-0.0003	0
Young's modulus		
Body (Left)	-0.6480	1.8018
Body (Right)	-0.6624	1.8159
Stiffener (Left)	0.3463	1.3114
Stiffener (Right)	0.3558	1.3198
Load scale factor		
Dead load	0.5130	0.0171
Live load	2.1175	1.8348
In-service wind load (In-plane)	2.9923	14.873
In-service wind load (Out-of-plane)	0.4900	1.9121
Out-of-service wind load (In-plane)	13.989	66.648
Out-of-service wind load (Out-of-plane)	2.3301	8.599
Yield strength		
Body (Left)	-8.0319	8.8381
Stiffener (Left)	-2.5299	2.925
Body (Right)	-8.0583	8.8729
Stiffener (Right)	-2.5132	2.9001



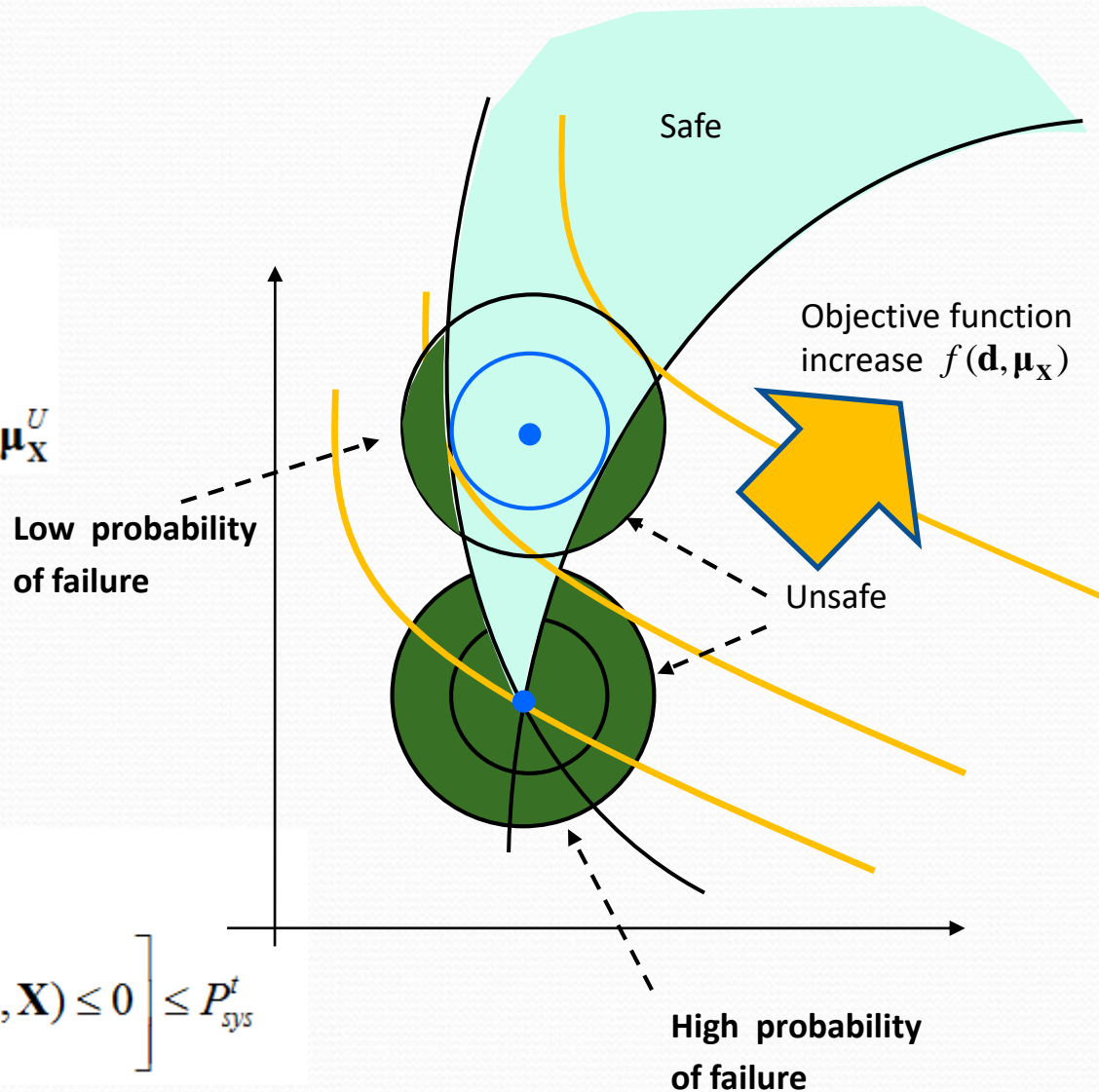
# App. VII: Reliability-Based Design Optimization

## >> Deterministic Optimization

$$\begin{aligned} \min_{\mathbf{d}, \boldsymbol{\mu}_X} \quad & f(\mathbf{d}, \boldsymbol{\mu}_X) \\ \text{s.t.} \quad & g_i(\mathbf{d}, \mathbf{X}) > 0 \\ & \mathbf{d}^L \leq \mathbf{d} \leq \mathbf{d}^U, \quad \boldsymbol{\mu}_X^L \leq \boldsymbol{\mu}_X \leq \boldsymbol{\mu}_X^U \end{aligned}$$

## >> Reliability-Based Design Optimization (RBDO)

$$\begin{aligned} \min_{\mathbf{d}, \boldsymbol{\mu}_X} \quad & f(\mathbf{d}, \boldsymbol{\mu}_X) \\ \text{s.t.} \quad & P_{\text{sys}} = P(E_{\text{sys}}) = P \left[ \bigcup_k \bigcap_{i \in C_k} g_i(\mathbf{d}, \mathbf{X}) \leq 0 \right] \leq P_{\text{sys}}^t \end{aligned}$$

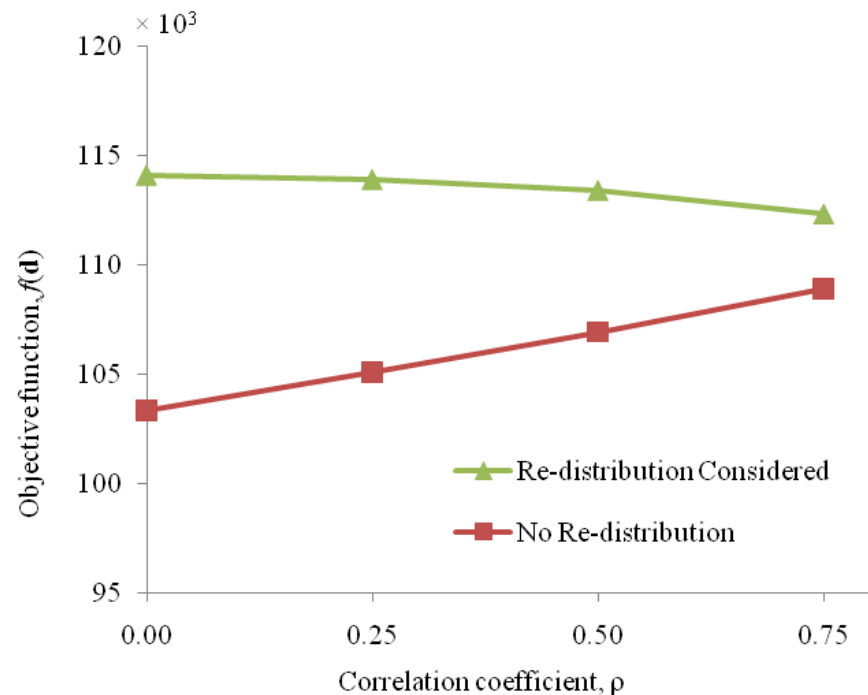
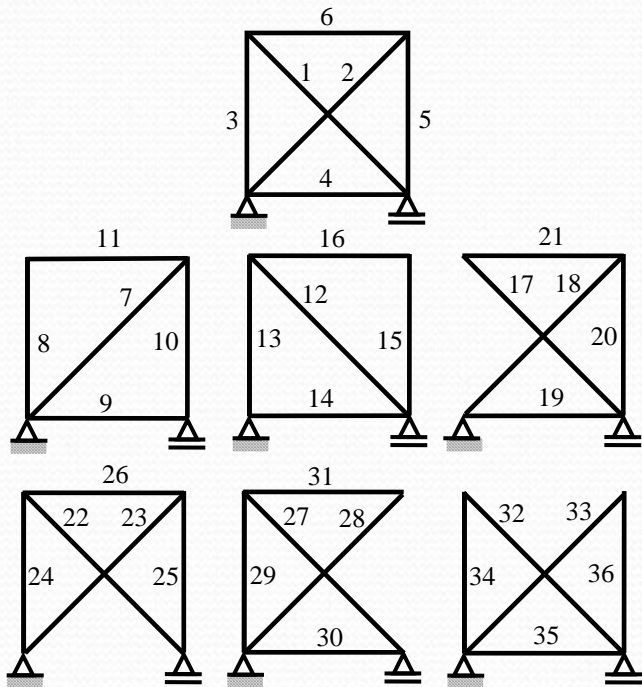


# System RBDO by MSR method

**RBDO of Truss system:** Minimize the cross section areas under target failure probability of system collapse

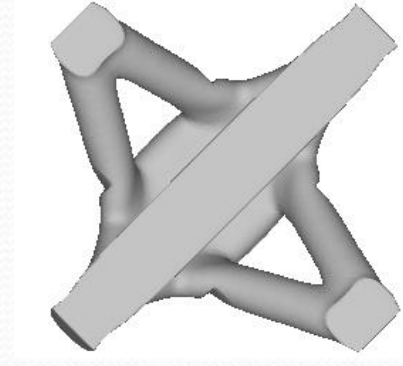
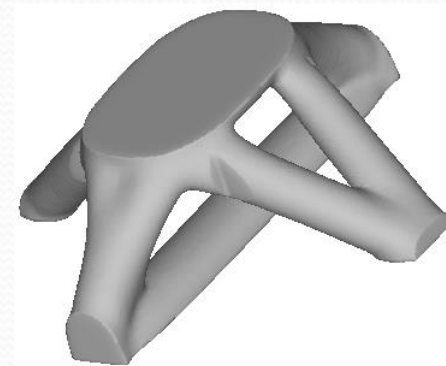
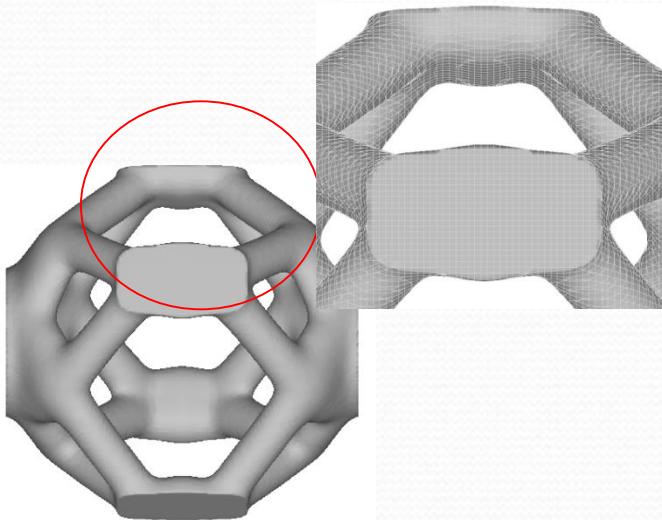
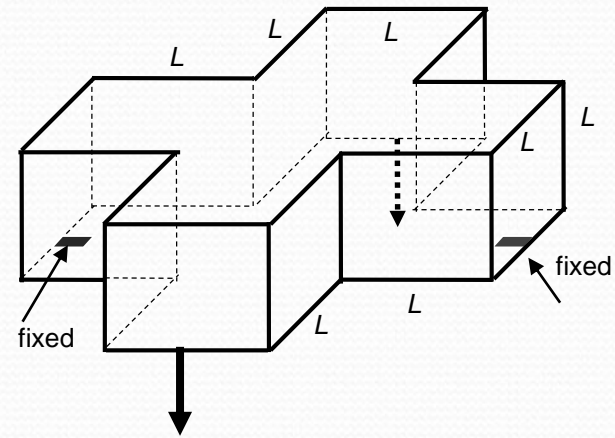
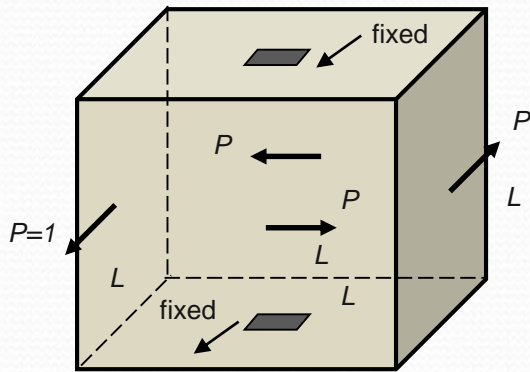
Using MSR method, we can consider

- Effects of **load re-distributions** (sequential failures)
- Effects of **correlation between components**



# System RBTO by MSR method

**RBTO of 2D or 3D continuum:** Minimize the volume or compliance under target failure probability of system failure

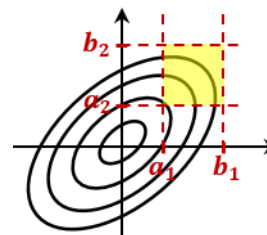


**457.646 Topics in Structural Reliability**  
**In-Class Material: Class 18**

⊙ **Multivariate normal integrals**

$$\mathbf{Z} \sim N(\mathbf{0}; \mathbf{R})$$

$$F(\mathbf{a}, \mathbf{b}; \mathbf{R}) = \int_{a_1}^{b_1} \cdots \int_{a_m}^{b_m} dz$$



If  $a_i = -\infty, i = 1, \dots, m$ , it becomes Joint of  $\mathbf{Z} \sim N(\mathbf{0}; \mathbf{R})$

$$\Phi_m(b_1, \dots, b_m; \mathbf{R}) = \int_{-\infty}^{b_1} \cdots \int_{-\infty}^{b_m} dz$$

I) **Ditlevsen & Madsen (1996)**

$$m = 2: \Phi_2(b_1, b_2; \rho_{12}) = \int_0^{\rho_{12}} \phi_2(b_1, b_2; \rho) d\rho$$

\_\_\_\_\_ assumption error by \_\_\_\_\_ assumption

Note: double-fold integral involving  $(-\infty, b_i) \Rightarrow$  single-fold integral in  $(0, \rho_{12})$

Note:  $\rho_{12} > 0$ : s.i assumption under/overestimate

$\rho_{12} < 0$ : s.i assumption under/overestimate

※  $m = 3$  Song & ADK (2005) double-fold integral

II) **Sequentially Conditioned Importance Sampling (SCIS)**

**(Ambartzumian et al. 1998)**

~sequentially sampling based on conditional PDF

given sampled value

~"scis.m" (developed by Prof. Young Joo Lee at UNIST

available at <http://systemreliability.wordpress.com/software/>

III) **Product of Conditional Marginals (Pandey & Sarkar 2002)**

$$\Phi_m(\mathbf{b}; \mathbf{R}) \cong \prod_{k=1}^m \Phi \left( \frac{b_k - \mu_{k|k-1}}{\sigma_{k|k-1}} \right)$$

- reasonable accuracy & very efficient
- parallel or series
- error ↑ as  $m$  ↑
- Improved PCM (Yuan & Pandey 2006)

IV) **Sequential Compounding Method (Kang & Song 2010)**

$$\{(Z_1 < -\beta_1) \cup (Z_2 < -\beta_2)\} \cap (Z_3 < -\beta_3)$$

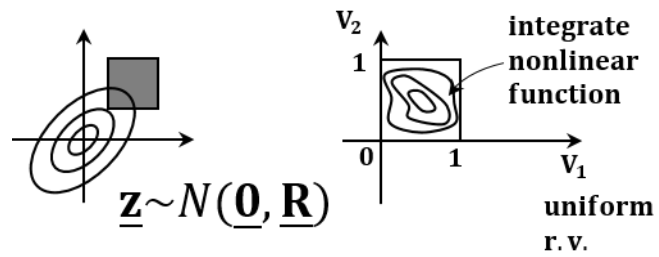
$Z_A < -\beta_A \rho_A$        $Z_B < -\beta_B$

- applicable to general system
- efficient and accurate
- handle large  $m$
- when the same component event appears multiple times → difficult
- parameter sensitivity of system reliability using SCM (Chun, Song, and Paulino, 2015, *Structural Safety*)

V) **Matrix-based System Reliability (MSR) Method (Kang & Song 2008) (Kang et al. 2012)**

VI) **Method by Genz (1992)** <http://www.math.wsu.edu/faculty/genz/homepage>

Transformations to uniform hypercube



→ Parallel system

→ Very accurate & efficient even for large-size system

→ Integration by quasi-MCS

→ `mvncdf.m` in Matlab

Genz, A., and Bretz, F. (2009) *Computation of Multivariate Normal and t Probabilities, Lecture Notes in Statistics*, Springer-Verlag, NY.